

Structural Dynamics Toolbox & FEMLink

For Use with MATLAB®

User's Guide

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1.1 Getting started

This section is intended for people who don't want to read the manual. It summarizes what you should know before going through the *SDT* demos to really get started.

You can find a primer for beginners at <http://www.sdtools.com/help/primer.pdf>.

Self contained code examples are distributed throughout the manual. Additional demonstration scripts can be found in the `sdt/sdtdemos` directory which for a proper installation should be in your MATLAB path. If not, use `sdtcheck('path')` to fix your path.

The MATLAB `doc` command no longer supports non MathWorks toolboxes, documentation access is thus now obtained with `sdtweb FunctionName`.

The *SDT* provides tools covering the following areas.

Area 1: Experimental modal analysis

Experimental modal analysis combines techniques related to system identification (data acquisition and signal processing, followed parametric identification) with information about the spatial position of multiple sensors and actuators.

An experimental modal analysis project can be decomposed in following steps

- before the test, preparation and design (see section 2.2)
- acquisition of test data, import into the SDT, direct exploitation of measurements (visualization, operational deflection shapes, ...) (see section 2.1)
- identification of modal properties from test data (see section 2.3)
- handling of MIMO tests and other model transformations (output of identified models to state-space, normal mode, ... formats, taking reciprocity into account, ...) (see section 2.4)

The series of `gart..` demos cover a great part of the typical uses of the *SDT*. These demos are based on the test article used by the GARTEUR Structures & Materials Action Group 19 which organized a Round Robin exercise where 12 European laboratories tested a single structure between 1995 and 1997.

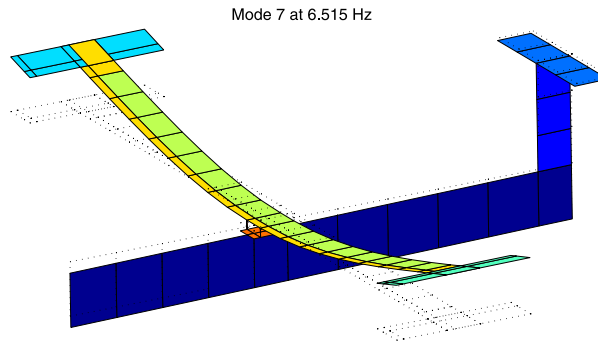


Figure 1.1: GARTEUR structure.

`gartfe` builds the finite element model using the `femesh` pre-processor

`gartte` shows how to prepare the visualization of test results and perform basic correlation

`gartid` does the identification on a real data set

`gartsens` discusses sensor/shaker placement

Area 2: Test/analysis correlation

Correlation between test results and finite element predictions is a usual motivation for modal tests. Chapter 3 addresses topology correlation, test preparation, correlation criteria, modeshape expansion, and structural dynamic modification. Details on the complete range of sensor definitions supported by SDT can be found in 4.3. Indications on how to use *SDT* for model updating are given in section 6.5.

`gartco` shows how to use `fe_sens` and `fe_exp` to perform modeshape expansion and more advanced correlation

`gartup` shows how the `upcom` interface can be used to further correlate/update the model

Area 3: Basic finite element analysis

Chapter 4 gives a tutorial on FEM modeling in *SDT*. Developer information is given in chapter 7. Available elements are listed in chapter 8.

A good part of the finite element analysis capabilities of the *SDT* are developed as part of the OpenFEM project. OpenFEM is typically meant for developers willing to invest in a stiff learning curve but needing an Open Source environment. *SDT* provides an integrated and optimized access to OpenFEM and extends the library with

- solvers for structural dynamics problems (eigenvalue ([fe_eig](#)), component mode synthesis (section 6.3), state-space model building ([fe2ss](#)), ... (see [fe_simul](#));
- solvers capable of handling large problems more efficiently than MATLAB;
- a complete set of tools for graphical pre/post-processing in an object oriented environment (see section 4.1);
- high level handling of FEM solutions using cases;
- interface with other finite element codes through the FEMLink extension to *SDT*.

Area 4: Advanced FE analysis (model reduction, component mode synthesis, families of models)

Advanced model reduction methods are one of the key applications of *SDT*. To learn more about model reduction in structural dynamics read section 6.2. Typical applications are treated in section 6.3.

Finally, as shown in section 6.4, the *SDT* supports many tools necessary for finite element model updating.

1.2 Understanding the Toolbox architecture

1.2.1 Layers of code

The *SDT* has three layers of code.

- **Graphical user interfaces** (`fepplot`, `iipplot`, `ii_mac`) provide a layer of pre-defined operations for Frequency Response Function (FRF) visualization and analysis, identification, 3-D deformation animation, and test/analysis correlation. Graphically supported operations (interactions between the user and plots/menus/mouse movements/key pressed) form a subset of commands provided by user interface functions.

The policy of the GUI layer is to let the user free to perform his own operations at any point. Significant efforts are made to ensure that this does not conflict with the continued use of GUI functions. But it is accepted that it may exceptionally do so, since command line and script access is a key to the flexibility of *SDT*. In most such cases, clearing the figure (using `clf`) or in the worst case closing it (use `close` or `delete`) and replotting will solve the problem.

- **User interface** (UI) functions provide high level solutions to problems in identification, finite element mesh handling, model reduction, sensor placement, superelement handling or parameterized models for FE model update. The first argument to these functions is a string command which is parsed to know what operations to perform. See `commode` for conventions linked to parsed commands.
- **Scientific functions** implement standard and state of the art methods in *experimental modal analysis*, *Finite Element analysis*, and to some extent in *structural design and FE model update*. These functions are open and can be easily extended to suit particular needs using the scientific environment provided by MATLAB.

1.2.2 Infos in Stack

When extensible and possibly large lists of mixed data are needed, *SDT* uses `.Stack` fields which are N by 3 cell arrays with each row of the form `{'type', 'name', val}`. The purpose of these cell arrays is to deal with unordered sets of data entries which can be classified by type and name.

`stack_get`, `stack_set` and `stack_rm` commands are low level commands used to get/set/remove single or multiple entries from stacks. Higher level pointer access to stacks stored in `iipplot` (curve stacks) and `fepplot` (model and case stacks) are described in section 2.1.2 and section 4.2.3.

1.2.3 Global variables

Pointers to variables stored in graphical objects are now preferred to global variables. The user interface for data visualization and identification (`iicom`, `idcom`, `iiplot`) no longer uses global variables, see section 2.1.6 for compatibility information. `femesh` and `upcom` are the only functions that use global variables.

User interfaces require knowledge of the current state of the interface and appropriate data. The policy of the *Toolbox* is to let the user free to perform his own operations at any point. Significant efforts are made to ensure that this does not conflict with the continued use of GUI functions, but it is accepted that it may exceptionally do so. This flexibility resulted in the use of both global variables (for information that the user is likely to modify) and graphical objects (for other information).

The `femesh` user interface for finite element mesh handling uses a number of standard global variables shown below

<code>FEnode</code>	main set of nodes (also used by <code>feplot</code>)
<code>FEn0</code>	selected set of nodes
<code>FEn1</code>	alternate set of nodes
<code>FEelt</code>	main finite element model description matrix
<code>FEe10</code>	selected finite element model description matrix
<code>FEe11</code>	alternate finite element model description matrix

By default, `femesh` automatically use base workspace definitions of the standard global variables: base workspace variables with the correct name are transformed to `global` variables even if you did not dot it initially. When using the standard global variables within functions, you should always declare them as global at the beginning of your function. If you don't declare them as global modifications that you perform will not be taken into account, unless you call `femesh`, ... from your function which will declare the variables as global there too. The only thing that you should avoid is to use `clear` and not `clear global` within a function and then reinitialize the variable to something non-zero. In such cases the global variable is used and a warning is passed.

1.3 Typesetting conventions and scientific notations

The following typesetting conventions are used in this manual

<code>courier</code>	blue monospace font : Matlab function names, variables
<code>feplot</code>	light blue monospace font: SDT function names
<code>command</code>	pink : strings and SDT commands
<code>var</code>	italic pink: part of command strings that have to be replaced by their value
<code>% comment</code>	green: comments in script examples
<i>Italics</i>	MATLAB Toolbox names, mathematical notations, and new terms when they are defined
Bold	key names, menu names and items
Small print	comments

Conventions used to specify string commands used by user interface functions are detailed under `commode`.

The following conventions are used to indicate elements of a matrix

<code>(1,2)</code>	the element of indices <code>1, 2</code> of a matrix
<code>(1,:)</code>	the first row of a matrix
<code>(1,3:)</code>	elements <code>3</code> to whatever is consistent of the first row of a matrix

Usual abbreviations are

CMS	Component Mode Synthesis (see section 6.3.3)
COMAC	Coordinate Modal Assurance Criterion (see <code>ii_mac</code>)
DOF,DOFs	degree(s) of freedom (see section 7.5)
FE	finite element
MAC	Modal Assurance Criterion (see <code>ii_mac</code>)
MMIF	Multivariate Mode Indicator Function (see <code>ii_mmif</code>)
POC	Pseudo-orthogonality check (see <code>ii_mac</code>)

For mathematical notations, an effort was made to comply with the notations of the International Modal Analysis Conference (IMAC) which can be found in Ref. [1]. In particular one has

$[], \{ \}$	matrix, vector
$\bar{\quad}$	conjugate
$[b]$	input shape matrix for model with N DOFs and NA inputs (see section 5.1). $\{\phi_j^T b\}, \{\psi_j^T b\}$ modal input matrix of the j^{th} normal / complex mode
$[c]$	sensor output shape matrix, model with N DOFs and NS outputs (see section 5.1). $\{c\phi_j\}, \{c\psi_j\}$ modal output matrix of the j^{th} normal / complex mode
$[E]_{NS \times NA}$	correction matrix for high frequency modes (see section 5.6)
$[F]_{NS \times NA}$	correction matrix for low frequency modes (see section 5.6)
M, C, K	mass, damping and stiffness matrices
N, NM	numbers of degrees of freedom, modes
NS, NA	numbers of sensors, actuators
$\{p\}_{NM \times 1}$	principal coordinate (degree of freedom of a normal mode model) (see section 5.2)
$\{q\}_{N \times 1}$	degree of freedom of a finite element model
s	Laplace variable ($s = i\omega$ for the Fourier transform)
$[R_j]$	$= \{c\psi_j\} \{\psi_j^T b\}$ residue matrix of the j^{th} complex mode (see section 5.6)
$[T_j]$	$= \{c\phi_j\} \{\phi_j^T b\}$ residue matrix of the j^{th} normal mode (used for proportionally damped models) (see section 5.6)
$\{u(s)\}_{NA \times 1}$	inputs (coefficients describing the time/frequency content of applied forces)
$\{y(s)\}_{NS \times 1}$	outputs (measurements, displacements, strains, stresses, etc.)
$[Z(s)]$	dynamic stiffness matrix (equal to $[Ms^2 + Cs + K]$)
$[\alpha(s)]$	dynamic compliance matrix (force to displacement transfer function)
p, α	design parameters of a FE model (see section 6.4.2)
$\Delta M, \Delta C, \Delta K$	additive modifications of the mass, damping and stiffness matrices (see section 6.4.2)
$[\Gamma]$	non-diagonal modal damping matrix (see section 5.3)
λ_j	complex pole (see section 5.5)
$[\phi]_{N \times NM}$	real or normal modes of the undamped system ($NM \leq N$)
$[\backslash \Omega^2 \backslash]$	modal stiffness (diagonal matrix of modal frequencies squared) matrices (see section 5.2)
$[\theta]_{N \times NM}$	NM complex modes of a first order symmetric structural model (see section 5.5)
$[\psi]_{N \times NM}$	NM complex modes of damped structural model (see section 5.5)

1.4 Other toolboxes from SDTools

SDTools also develops other modules that are distributed under different licensing schemes. These modules are often much less documented and address specialized themes, so that only a technical discussion of what you are trying to achieve will let us answer the question of whether the module is useful for you.

- Viscoelastic tools : an SDT extension for the analysis and design of viscoelastic damping. Beta documentation at <http://www.sdtools.com/help/visc.pdf>.
- Rotor tools : an SDT extension for rotor dynamics and cyclic symmetry. Beta documentation at <http://www.sdtools.com/help/rotor.pdf>.
- non linear vibration tools : an SDT extension for non-linear vibration and in particular time and frequency domain simulation of problems with contact and friction.
- OSCAR : a module for the study of pantograph/catenary interaction developed with SNCF.

Selected cross references to these other modules are listed here.

- [fevisco Range](#) this command is part of the viscoelastic tools.
- [fe2xf](#) this function is part of the viscoelastic tools.
- [fe_cyclicb ShaftEig](#) this command is part of the rotor tools.

1.5 Release notes for SDT 6.5 and FEMLink 3.8

1.5.1 Key features

SDT 6.5 is the first version compatible with MATLAB 8.0 (2012b). Key features of this release are

- Major update of `iipLOT`, `iicom` to support more general plots (contour, surface, ...) and documented control procedures for automated inits, legend, marker lines, text based tick, ...
- Major revision of `feplot`. Rewrite of the color field handling : support for colored vector field display, reuse of pre-computed energy at elements data structures, ... Introduction of a complete `iicom ImWrite` command for generation of image sequences to be included in automated reports.
- Significant improvement of utilities for piezo modeling : visualization of charges and electrical fields, ...
- Improved compatibility with MATLAB figure toolbar callbacks and japanese version of MATLAB.

Key features of FEMLink 3.8 are

- `abaqus` improved reading of `.fil` (velocity, acceleration and resultant fields), read/write `.inp` (`*nset`, `*spring`, `*orientation`, composite and orthotropic materials, element conversion table, contact)
- `ans2sdt` improved reading of beam sections, velocity and resultant fields in `.rst` files.
- `nasread` corrected support of MAT9 and MATT entries to reflect improved implementation in SDT. Bugs were corrected with writing of rigid case entries.
- `samcef` improved u18 reading speed and support for 64 bit format files.

For MATLAB compatibility see section 1.5.3.

1.5.2 Detail by function

The following functions have been modified.

beam1 robustness enhancements for compatibility with **fe_caseg StressCut**.
elem0 enhanced **VectFromDir** (see section 7.13)
comgui Major rewrite of **comgui ImWrite** and **iicom ImWrite** for automated figure generation.
fe_cyclic **Build-ByMat** allows interfaces with coincident nodes.
fe_case improved handling of parameters, see **upcom Par** and **fe_def Range**
fe_caseg Robustness enhancements in **Assemble** (see section 4.5.8), major extension of **StressCut**, **ZoomClip**.
fe_curve corrected errors on noise generation for odd numbers of samples.
fe_gmsh significant extension of CAD definition commands.
fe_def **Range** commands for the description of design maps are now documented. Many detail robustness enhancements.
fe_mat robustness enhancements for unit conversion (**convert** command) and for support of interpolated properties, see section 7.3.
fe_mpc **model=fe_mpc('Rbe3Id',model)** generated unique identifiers for RBE3 constraints.
fe_quality improved integration in feplot and robustness.
fe_reduc **Free** (reduction on free modes) now supports **DofSet** (enforced displacement) commands. A **.UseLoad** option has been added to **CraigBampton** to allow computation of load residuals for a Craig Bampton reduction.
fe_sens rewriting of the **gartte** and **gartsens** demos associated with robustness enhancements.
fe_stress output of the **Ener** provides newer options and output in the newer curve format. **feplot** now allows dynamic switching between energy value, density or group value.
fe_time improved implementation of Theta method integration.
feplot complete rewrite of **ColorData** for energy computations, **ColorScale** for more accurate display, field display for **StressCut**.
improved support of HDF file delayed reading.
feutil Major extension of the **feutil SetPro** command. Improved handling of surface sets. **GetDof** corrections for master DOFs in rigid elements.
feutilb **Match** implements a new node matching strategy as **MatchSurf** and has undergone significant speed enhancements. **CombineModel** supports extended renumbering.
idcom improved integration of Error computations. GUI robustness improvements.
ii_plp significant rewrite and documentation of **iiplot PlotInfo** utilities. Significant extensions of the **Legend** command. Documentation of the **TickFcn** capabilities.
iiplot documentation was revised to include **PlotInfo** in particular.

1.5.3 Notes by MATLAB release

- MATLAB 7.6 to 8.0 (2012b). *SDT 6.5* and *FEMLink 3.8* are developed for these versions of MATLAB and are fully compatible with them.
- MATLAB 7.5 is fully compatible with the exception of the new [curvemodel](#) object which requires the newer MATLAB object.
- Earlier MATLAB releases are no longer supported.

1.6 Release notes for SDT 6.4 and FEMLink 3.7

1.6.1 Key features

SDT 6.4 is a relatively minor release due to significant architectural work on GUI and implicit curve models that is not yet mature enough for general release. Key features are

- significant enhancement of sensor support in terms of speed and functionality with the new stress cut (see section 4.4). The objective is to allow detailed stress analysis on arbitrary viewing meshes. This strategy is particularly interesting for the analysis of stress responses in long transients where the volume of data can become very large. Detail extensions of stress processing were also introduced with this functionality.
- proper documentation and introduction of an Euler solver was made for the study of transient heat equation problems, see section 6.1.13 and [p_heat](#).
- [feplot](#) handling of field colors has undergone a major revision for more consistent handling and improved on the fly generation of color maps for the animation of long transients.
- rewriting of documentation and improvement of correlation criteria.

Key features of FEMLink 3.7 are

- [abaqus](#) significant robustness enhancements, in particular for parts and motion commands.
- [ans2sdt](#) improved CP reading in multi-physics configurations. Added some elements missing from the supported list. Fixed compatibility issues with newer ANSYS binaries.
- [nasread](#) introduced partial support for OUTPUT4 in text format. Detail corrections and performance enhancements.
- [samcef](#) now supports an [imp2](#) command to build explicit second order models from implicit ones. A number of detail enhancements are also introduced.

For MATLAB compatibility see section 1.6.3.

1.6.2 Detail by function

The following functions have been modified.

<code>feplot</code>	<code>fecom ColorScale</code> commands were fully revised for improved stability and performance. <code>ColorDataEner</code> was fully revised to allow reuse of existing energy computations. A new <code>InfoMass</code> command is available to summarize component masses. <code>ColorDataEval</code> was significantly extended in particular for the support of <code>StressCut</code> selections. A new <code>-ColorBarTitle</code> option was added. Minor corrections to the <code>AnimAvi</code> command were introduced.
<code>feutil</code>	Major extension of the <code>feutil SetPro</code> command. Improved handling of surface sets.
<code>feutilb</code>	major improvement in matching speed (used for <code>ConnectionSurface</code> or <code>StressCut</code>). Robustness enhancement for out-of-core operations.
<code>fe2ss</code>	Minor bug corrections on damping handling.
<code>fe_case</code>	<code>SensDof</code> entry has been extended for better rotation sensor support. Incompatible matching speed has been greatly enhanced. The new <code>StressCut</code> command (see section 4.4) was introduced to allow stress processing of long transient simulations.
	<code>ConnectionSurface</code> now support multi-physics FEM problems.
	<code>stack_get</code> and set commands are now supported.
<code>fe_def</code>	Enhanced documentation, label generation, curve joining, ...
<code>fe_fmesh</code>	now packages calls to MATLAB 3D delaunay capabilities.
<code>fe_mkn1,</code> <code>basis</code>	Fixed minor bugs with the generation of dependent coordinate systems and improved handling of round-off errors. Port of right hand side computations to <code>of_mk</code> has been extended.
<code>fe_mpc</code> <code>fe_reduc</code>	a new <code>FixRbe3Alt</code> was introduced to ease reformatting of RBE3 entries. the documentation was rewritten. The Craig-Bampton command was revised to support out-of-core operations with <code>.mat</code> files in the HDF format.
<code>fe_time</code>	support for θ -method and Euler solvers has been added. Improvements of on the fly processing with have been made in <code>of_time</code> . Major improvements for for non-linear time simulation are being developed and will be offered as a SDT extension.
<code>fe_sens</code>	<code>ToFEM</code> option was added to the <code>basis</code> command to improve test mesh orientation procedures.
<code>fe_simul</code>	<code>Static</code> now supports mixed <code>DofSet</code> and <code>DofLoad</code> entries.

<code>sdtweb</code>	m-file opening with tag searches has been enhanced.
<code>curvemodel</code>	This new object (requires MATLAB ≥ 7.6) is used for implicit curve models. This object is the basis for future extensions of signal processing and FEM restitution capabilities in SDT.
<code>idcom</code>	Robustness enhancements and improved support of quality indicator.
<code>id_rm</code>	Minor revisions for data structure support.
<code>ii_mac</code>	now supports a <code>SubDof</code> option for MAC computation on partial DOF sets. Documentation was rewritten. Minor bug corrections on COMAC and MACCO labels.
<code>ii_plp</code>	Extended vertical line generation capabilities.
<code>ii_com</code> ,	Extended support of HDF file reading. Significant extensions of polar
<code>ii_plot</code>	and 2D plots. Many minor bug corrections.
<code>line2</code>	was introduced as topology holder for multi-physics 3D line elements.
<code>p_piezo</code>	Enhanced compatibility with <code>fe_simul</code> and minor bug fixes.
<code>p_beam</code>	Introduced NSM support for subtype 3 and revised section display capabilities.
<code>p_heat</code>	Support and documentation of solvers for transient heat equation has been extended.
<code>ufread</code>	Improved robustness reading headers.
<code>upcom</code>	Major documentation rewriting and better integration with <code>fe_case</code> .
<code>cingui</code>	Major revision in preparation for SDT/Java integration and improved command option handling.
<code>sdthdf</code>	improved robustness and performance.
<code>tria3</code>	now properly supports local fiber orientation.

1.6.3 Notes by MATLAB release

- MATLAB 7.6 to 7.13 (2011b). *SDT 6.4* and *FEMLink 3.7* are developed for these versions of MATLAB and are fully compatible with them.
- MATLAB 7.5 is fully compatible with the exception of the new `curvemodel` object which requires the newer MATLAB object.
- MATLAB7.1 to 7.4 compatibility is no longer tested. But there are only minor limitations in HDF support (only affects users interested in large FEM computations).
- Earlier MATLAB releases are no longer supported.

1.7 Release notes for SDT 6.3 and FEMLink 3.6

1.7.1 Key features

Key features of the SDT 6.3 release are

- `iiplot` was significantly enhanced (multi-dimensional scanning, channel lbel generation, channel selection, ...). The `SubToFig` command was introduced to ease the automated report generation process.
- `fe_curve` signal processing and curve generation were extended and integrated into `iiplot`.
- sensor documentation and robustness was significantly revised to improve ease of use and access for new users (see section 4.3 and `fe_sens` commands `Basis`, `Laser` and `WireExp`).
- Tutorials on wire frame definition (section 3.1) and basic interpolation (section 3.3.2 and `fe_sens('WireExp')` were revised).

Key features of FEMLink 3.6 are

- `ans2sdt` was significantly enhanced in its ability to read CDB files (and some forms of input files). Local coordinates are now supported. Material properties are read more consistently. Partial support of `.sub` and `.cms` files was introduced.
- `nasread` was revised for enhanced PBEAML, PBARL read/write, ASET, GENEL and OEF support, OQG read performance, ...
- `ufread` has new or improved support of files 776, 791, 1710, 2435, 2467, 2477

OpenFEM

- Significant extensions of the ability to specify fields at nodes, DOFs and integration points were introduced. See section 7.13 (`sdtweb('VectFromDir')`). Associated `fe_mknl` commands `OrientMap` and `MapMerge` were fully revised.
- `elem0` command `GaussObserve` that supports generation of observation information at Gauss points was extended.
- `fe_time` underwent many detail improvements. An implementation of the `Theta` method was added.

- [p_beam](#) underwent a major revision to support a wider range of pre-defined sections.

For MATLAB compatibility see section 1.8.3.

1.7.2 Detail by function

The following functions have been modified.

feplot

ColorBar was fully revised to allow consistent property editing. Inits for **cf.def.LabFcn** are more consistent.

ColorData has been extended. **Eval** options now support radial or tangential motion colors. **-EdgeAlpha** can now be specified directly to obtain transparent edges.

ScaleMatch now works more consistently in mixed FEM/test displays. the material and property tabs now generate view of all elements with the associated material (earlier faces internal to the body were not shown)

AnimAvi for AVI generation was rewritten for more robustness.

fesuper

improved restitution and minor robustness enhancements

feutil

performance and robustness was enhanced for **AddNode**. **ShellMap** now supports material orientation maps. **AddTest** (model merging) now supports material and property renumbering. Orientation problems in the **divide** command were fixed for surfaces.

MatId, **ProId** now support easier property renumbering.

AddElt is a new command easing addition of element groups.

FindNode now supports a selection within a cylinder with **cyl**

SetPro was introduced to associated fields with element properties stack entries (for example orientation maps)

RotateNode was introduced to allow rotations without node duplication.

fe_case

the **ConnectionScrew** command was significantly extended.

Assemble now supports a **-reset** option to force reassembly (see **sdtweb('simul#feass')**). A number of minor problems were also fixed.

fe_cyclic

Solve-FixTan is now supported to fix the global rotation mode in 0 diameter computations. An error was corrected for periodic static and mode solutions.

fe_curve

Test and **window** commands for signal generation and windowing were fully revised.

fe_def

was significantly extended for GUI handling, deformation structure manipulation (see section 7.8 for **SubDef**, **SubDof**, **Exp**), **Curve[Join,Cat]** were extended, ...

fe_eig

method 5 was rewritten to optimize accuracy and memory handling

fe_gmsh

the OpenFEM driver for GMSH was revised

fe_mpc

a **rbe32c** command for RBE3 to MPC transformation was added.

1.7.3 Notes by MATLAB release

- MATLAB 7.4 to 7.11 (2010b). *SDT 6.3* and [FEMLink 3.6](#) are developed for these versions of MATLAB and are fully compatible with them.
- MATLAB7.1 to 7.3 compatibility is tested and there a minor limitations in HDF support. This limitation really only affects users interested in large FEM computations.
- MATLAB 6.5 and 7.0 are not compatible with this release of SDT.

1.8 Release notes for SDT 6.2 and FEMLink 3.5

1.8.1 Key features

Key features of the SDT 6.2 release are

- a major revision of the `iiplot` interface was introduced to improve label handling, support the direct display of time simulation results, integration of signal processing capabilities of `ii_mmif`, 2D displays associated with time/frequency plots, ...
- `fecom`, and `iicom` usability is improved with new icons, revised menus, `iiplot/feplot` interaction, interactive mesh editing, default model properties, ...
- improved the readability of the documentation through keyword highlighting in both the HTML and PDF, see section 1.3.
- introduction of a generic file for test setup definitions typically filled with Excel. This significantly simplified the definition of sensors and triaxes normal to the supporting surface as well as laser vibrometer measurement specification.
- full rewrite of node matching algorithms which enhances speed for mesh generation, sensor matching, incompatible mesh handling, ...

Key features of FEMLink 3.5 are

- `ufread` has been modified to improve robustness to fields containing blanks, enhance reading speed and correct problems with local coordinate systems.
- `abaqus` has been extended and optimized.
- `samcef` import/export capabilities have been extended.
- `ans2sdt` was revised for a better support of degenerate elements.
- `nasread` was extended to support variations introduced in NASTRAN 2007. Stress reading from `op2` files was improved.

OpenFEM has undergone many detail improvements and the following revisions

- The support of general fields at nodes has been significantly generalized with labels identifying the fields for easier generation of families of elements with implementations depending on fields actually present (typical fields are constitutive

properties, material orientations, thickness, temperature, pressure, ...). The first version of a property interpolation mechanism based on tables is introduced.

- a better mechanism to return to MATLAB after the field extraction and Jacobian computations of each element is introduced.
- composite shell support has been extended.

For MATLAB compatibility see section ??.

1.8.2 Detail by function

The following functions have been modified.

<code>iiplot</code>	has undergone significant extensions for improved dataset support (direct display of FEM responses, selection in multi-dimension datasets, ...), mouse interactivity, improved menus and icons. Many detailed robustness enhancements.
<code>idcom</code>	now support cursor interaction in the identification error plot.
<code>fe_sens</code>	introduced cell array description of complex test setups (see section 4.3.2), improved sensor matching robustness
<code>ii_mmif</code>	signal processing capabilities have been significantly extended. Processing of FEM time deformations can now be done directly.
<code>fe2ss</code>	now supports parametric models
<code>fesuper</code>	many detail improvements in superlement building and restitution
<code>feplot</code> , <code>fecom</code>	improved support for the display of stack entries. Display of sensor entries has been improved through bug corrections on the cursor, compatibility with <code>TextDof</code> command.
<code>fe_case</code>	improved of RBE3 support.
<code>fe_cyclic</code>	support of periodic rather than cyclic solutions. Major extensions of the cyclic symmetry capabilities are under way be will be distributed as a SDT-Rotor module.
<code>fe_mkn1</code> , <code>fe_case</code>	Significant efforts where done to clarify debugging information when improper models are used.
<code>fe_quality</code>	This new function supports many estimates of finite element mesh quality.
<code>feutil</code>	improved face support. Corrected bugs in <code>r</code> based node selection. All mesh building commands have been ported so that <code>femesh</code> can be bypassed altogether. This seems easier to learn for new users.
<code>fe_stres</code>	the function has been renamed <code>fe_stress</code> (the limitation on 8 characters being very obsolete). Bugs in shell stress processing have been corrected.
<code>q4cs</code>	the SDT composite/piezo element has undergone a major revision for improved orientation handling and stress computations.

1.8.3 Notes by MATLAB release

- MATLAB 7.4 to 7.9 (2009b) *SDT 6.2* and *FEMLink 3.5* are developed for these versions of MATLAB and are fully compatible with them.
- MATLAB7.0 to 7.3 compatibility is tested and there a minor limitations in HDF support. This limitation really only affects users interested in large

FEM computations.

- MATLAB 6.5 compatibility is no longer tested and major incompatibilities exist related to graphics, HDF support and possibly BLAS calls. SDT is not compatible with any earlier version of MATLAB.
- MATLAB \geq 7.4 (2007a), the doc command no longer searches non MathWorks help, use `sdtweb` to open SDT help pages. Help search is however available in the help browser. You can change the default `setpref('SDT', 'Browser', '-helpbrowser')` to modify the location where `sdtweb` opens pages.

Modal test tutorial

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An experimental modal analysis project can be decomposed in following steps

- before the test, preparation and design (see section 2.2)
- acquisition of test data, import into the SDT, direct exploitation of measurements (visualization, operational deflection shapes, ...) (see section 2.1)
- identification of modal properties from test data (see section 2.3)
- handling of MIMO tests and other model transformations (output of identified models to state-space, normal mode, ... formats, taking reciprocity into account, ...) (see section 2.4)

Further steps (test/analysis correlation, shape expansion, structural dynamics modification) are discussed in chapter section 3.

2.1 iipplot interface tutorial

`iipplot` is the response viewer used by SDT. It is essential for the identification procedures but can also be used to visualize FEM simulation results.

As detailed in section 2.3, identification problems should be solved using the standard commands for identification provided in `idcom` while running the `iipplot` interface for data visualization. To perform an identification correctly, you need to have some familiarity with the interface and in particular with the `iicom` commands that let you modify what you display.

2.1.1 The main figure

For simple data viewing you can open an `iipplot` figure using `ci=iipplot` (or `ci=iipplot(2)` to specify a figure number). For identification routines you should use `ci=idcom` (standard datasets are then used see section 2.3).

To familiarize yourself with the `iipplot` interface, run `demosdt('demogartidpro')`. Which opens the `iipplot` figure and the associated `iipplot(2) properties` figure whose tabs are detailed in the following sections.

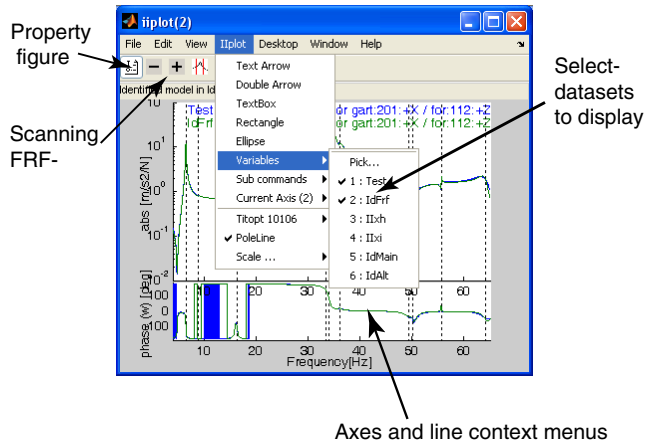






















Figure 2.1: Display figure of the `iiplot` interface.

Toolbar

	Toggles the display or not of the <code>iiproperty</code> figure.
	Previous channel/deformation, see <code>iicom ch+</code> .
	Next channel/deformation.
	Fixed zoom on FRF, see <code>iicom wmin</code> . Note that the variable zoom (drag box) is always active, see <code>iimouse zoom</code> .
	Start cursor, see <code>iimouse Cursor</code> .
	Refresh the displayed axes.
	No subplot. See <code>iicom Sub[1,1]</code> .
	2 subplots. See <code>iicom Sub[2,1]</code> .
	Amplitude and phase subplots. See <code>iicom Submagpha</code> .
	switch lin/log scale for x axis. See <code>iicom xlin</code> .
	switch lin/log scale for y axis. See <code>iicom ylog</code> .
	switch lin/log scale for z axis. See <code>iicom xlog</code> .
	Show absolute value. See <code>iicom Showabs</code> .
	Show phase. See <code>iicom Showpha</code> .
	Show real part. See <code>iicom Showrea</code> .
	Show imaginary part. See <code>iicom Showima</code> .
	Show real and imaginary part. See <code>iicom Showr&i</code> .
	Show Nyquist diagram. See <code>iicom Shownyq</code> .
	Show unwrapped phase. See <code>iicom Showphu</code> .
	Snapshot. See <code>iicom ImWrite</code>

Mouse operation and keyboard shortcuts

Mouse and keypress operations are handled by `iimouse` within `iiproperty`, `feplot`, and `ii_mac` figures. For a list of active keys press `?` in the current figure.

Drag your mouse on the plot to select a region of interest and see how you directly zoom to this region. Double click on the same plot to go back to the initial zoom. On some platforms the double click is sensitive to speed and you may need to type the `i` key with the axis of interest active. An axis becomes active when you click on it.

Open the `ContextMenu` associated with any axis (click anywhere in the axis using the right mouse button), select `Cursor`, and see how you have a vertical cursor giving information about data in the axis. To stop the cursor use a right click or press the `c` key. Note how the left click gives you detailed information on the current point or the left click history. In `iipplot` you can for example use that to measure distances.

Click on pole lines (vertical dotted lines) and FRFs and see how additional information on what you just clicked on is given. You can hide the info area by clicking on it.

Context menus

The `axes ContextMenu` (click on the axis using the right mouse button) lets you select , set axes title options, set pole line defaults, ...

- `Cursor` tracks mouse movements and displays information about pointed object. For ODS cursor see `iicom ods`.
- `Show` chooses what to display.
- `Compute... [MMIF,CMIF...]` chooses what to compute and display. The `iicom('show [MMIF,CMIF...]')` command line is similar. Details on what can be computed are given in `ii_mmif`.
- `Variables in current axis...` chooses which variable to display, see `iicom IIX`.
- `iipplot properties`, same as `iicom('pro')`, opens the property figure.
- `Scale... [x lin, x log...]` chooses the axis scale as the. See `iicom xlin` or use `iimouse('axisscale[xlin,xlog...]')` commands.
- `TitOpt` chooses the title, axis and legend labels-format.
- `PoleLine` pole line selection.
- `Views...` chooses the views, see `iimouse view`.
- `colorbar` shows the colorbar and is equivalent to `cingui('ColorBarMenu')` command line.
- `Zoom reset` is the same as the `iimouse('resetvie')` command line to reset the zoom.

- `setlines` calls the associated function.

The `line ContextMenu` lets you can set line type, width, color ...

The `title/label ContextMenu` lets you move, delete, edit ... the text

After running through these steps, you should master the basics of the `iipplot` interface. To learn more, you should take time to see which commands are available by reading the *Reference* sections for `iicom` (general list of commands for plot manipulations), `iimouse` (mouse and key press support for SDT and non SDT figures), `iipplot` (standard plots derived from FRFs and test results that are supported).

2.1.2 The curve stack

`iipplot` considers data sets in the following format

- **Response data** related to `UFF58` format
- Curves generated by SDT
- **Shapes at DOFs** related to `UFF55` format

This data is stored in `iipplot` figures as a `Stack` field (a cell array with the first column giving `'curve'` type entries, the second giving a name for each dataset and the last containing the data, see `stack.get`). To allow easier access to the data, `SDT handle` objects are used. Thus the following calls are equivalent ways to get access to the data

```
ci=iicom('curveload','gartid');
iicom(ci,'pro');iicom(ci,'CurTab Stack'); % show stack tab

% Normal use : the figure pointer stack
ci.Stack % show content of iipplot stack
ci.Stack{'Test'} % a copy of the same data, selected by name
ci.Stack{1,3} % the same by index
% Use regular expression ('II.*' here) for multiple match
ci=stack_rm(ci,'curve','#II.*')

% If you really insist on low level calls
r1=get(2,'userdata'); % object containing the data (same as ci)
```

```

s=ci.vfields.Stack.GetData % get a copy of the stack (cell array with
                          % type,name,data where data is stored)
s{1,3} % the first data set

% Alternative use (obsolete) : the XF stack pointer
XF1=iicom(ci,'curvexf');
XF1('Test') % still the same dataset, indexed by name
XF2=XF1.GetData; % Copy the data from the figure to variable XF2

```

The `ci.Stack` handler allows regular expression based access, as for `cf.Stack`. The text then begins by the `#` character.

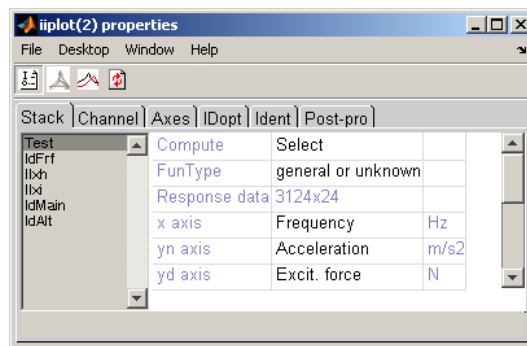


Figure 2.2: Stack tab of the `iiplot` interface.

The graphical representation of the stack shown in figure 2.2 lets you do a number of manipulations which are available through the context menu of the list of datasets in the stack

Compute gives access to data processing commands in `ii_mmif`. You perform the analysis from the command line with `iicom(ci,'sum','Test')`. The list of available post processing functions is given by `ii_mmif list`.

Load lets you load more data with `iicom(ci,'curveload-append','gartid')`, replace the current data with `iicom(ci,'curveload','gartid')`

Display lets you display one or more selected dataset in the `iiplot` figure (see corresponding command `iicom IIx`).

Save lets you save one or more dataset (see corresponding command `iicom CurveSave`).

Join combines selected datasets that have comparable dimensions (see corresponding command `iicom CurveJoin`).

Cat concatenates selected datasets along time or frequency dimension (see corresponding command `iicom CurveCat`).

Remove removes selected dataset (see corresponding command `iicom CurveRemove`).

NewId opens a new `idcom` figure with the selected dataset (see corresponding command `iicom CurveNewId`).

2.1.3 Handling what you display, axes and channel tabs

`iipplot` lets you display multiple axes see `iicom Sub`. Information about each axis is show in the `axes` tab.

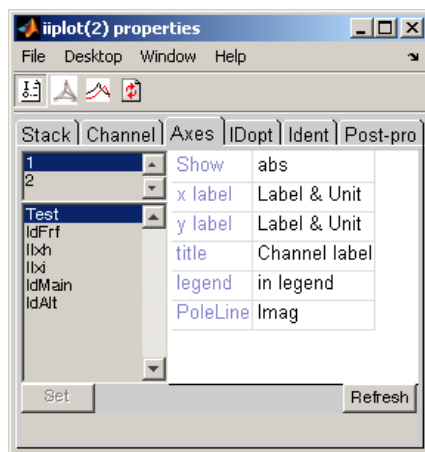


Figure 2.3: Axes tabs of the `iipplot` interface.

For example open the interface with the commands below and see a few thing you can do

```
ci=idcom;iicom(ci,'CurveLoad sdt_id');
ci.Stack{'IdFrfr'}=ci.Stack{'Test'}; % copy dataset
ci.Stack{'IdFrfr'}.xf=ci.Stack{'Test'}.xf*2; % double amplitude
iicom('CurTab Axes');
```

Sub Subplots : Type `iicom submagpha` to display a standard magnitude/phase plot. Open the `IIplot:sub commands` menu and see that you could have achieved the same thing using this pull-down menu. Note that using `ci=iipplot(2)` `iicom(ci,'SubMagPha')` gives you control on which figure the command applies to.

Show Type `iicom(';cax1;showmmi')`; to display the MMIF in the lower plot. Go back to the phase, by making axis 1 active (click on it) and selecting `phase(w)` in the **axis type menu** (which is located just on the right of the current axis button).

IIX select sets you want to display using `iicom(';showabs;ch1');``iicom('iix only',{'Test','IdFr'})`. You could also achieve the same thing using the `IIplot:Variables` menu.

- Note that when you print the figure, you may want to use the `comgui('ImWrite','')` command or `-noui` switch so that the GUI is not printed. It is the same command as for `feplot` image printing (see `iicom ImWrite`).

Once you have selected the datasets to be displayed, you can use the channel tab to scan through the data.

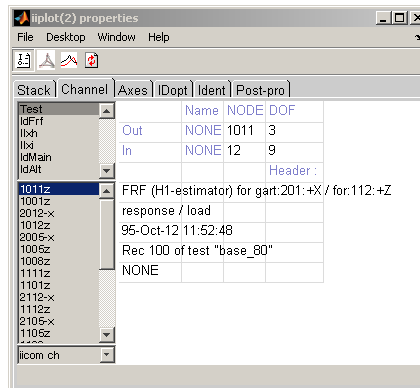


Figure 2.4: Channel tabs of the `iipplot` interface.

Major commands you might want to know

- use the `- +` to scan through different transfer functions. Note that you can also use the `+` or `-` keys when a drawing axis is active.

- Go the **Channel** tab of the property figure and select more than one channel in the list. Note that you can also select channels from the command line using `iicom('ch 1 5')`.

2.1.4 Importing FRF data

There are two main mechanisms to import FRF data into SDT. *Universal files* are easiest if generated by your acquisition system. Writing of an *import script* defining fields used by SDT is also fairly simple and described below (you can then use `ufwrite` to generate universal files for export).

The `ufread` and `ufwrite` functions allow conversions between the `xf` format and files in the Universal File Format which is supported by most measurement systems. A typical call would be

```
fname=demosdt('build gartid.unv'); % generate the gartid.unv file
UFS=ufread(fname);                % read
ci=idcom;    % For identification purposes open IDCOM
ci.Stack{'curve','Test'}=UFS(1); % Define FRFs in set 'Test'
% possibly extract channels 1:4
% ci.Stack{'curve','Test'}=fe_def('SubDofInd',UFS(1),1:4)

% To only view data in figure(11) the following would be sufficient
cj=iipplot(11);    % open an iipplot in figure 11
iipplot(cj,UFS(1)); % show UFS(1) there
```

where you read the database wrapper `UFS` (see `xfopt`), initialize the `idcom` figure, assign dataset 3 of `UFS` to dataset 'Test' 1 of `ci` (assuming that dataset three represents frequency response functions of interest).

Note that some acquisition systems write many universal files for a set of measurements (one file per channel). This is supported by `ufread` with a stored file name

```
UFS=ufread('FileRoot*.unv');
```

Measured frequency responses are stored in the `.xf` field (frequencies in `.w`) and should comply with the specifications of the `xf` format (see details under `xf` page 167). Other fields needed to specify the physical meaning of each FRF are detailed in the `xfopt` reference section. When importing data from your own format or using a universal file where some fields are not correct, the *SDT* will generally function

with default values set by the `xfopt` function, but you should still complete/correct these variables as detailed below.

For correct display in `feplot` and title/legend generation, you should set the `ci.Stack{'Test'}.dof` field (see section 2.2 for details on geometry declaration, and `mdof` reference). For example one can consider a MIMO test with 2 inputs and 4 outputs stored as columns of field `.xf` with the rows corresponding to frequencies stored in field `.w`. Your script will look like

```
ci=idcom;
[XF1,cf]=demosdt('demo2bay xf');% sample data and feplot pointer
out_dof=[3:6]+.02'; % output dofs for 4 sensors in y direction
in_dof=[6.02 3.01]; % input dofs for two shakers at nodes 1 and 10
out_dof=out_dof(:)*ones(1,length(in_dof));
in_dof=ones(length(out_dof),1)*in_dof(:)';
XF1=struct('w',XF1.w, ... % frequencies in Hz
          'xf',XF1.xf, ... % responses (size Nw x (40))
          'dof',[out_dof(:) in_dof(:)]);
ci.Stack{'Test'}=XF1; % sets data and verifies
ci.IDopt.nsna=size(out_dof,1); % define IDCOP prop
ci.IDopt.recip='mimo'; % define IDCOP prop
iicom(ci,'sub');

cf.def=ci.Stack{'Test'}; fecom('ch35'); % frequency of first mode
```

You can also edit these values using the `iiplot properties:channel` tab.

For correct identification using `id_rc`, you should verify the fields of `ci.IDopt`. These correspond to the `IDcomGUI:Options` tab (see section 2.3). You can also edit these values in a script. For correct identification, you should set

```
ci=demosdt('demogartid');
ci.IDopt.Residual='3';
ci.IDopt.DataType='Acc';
ci.IDopt.Absci='Hz';
ci.IDopt.PoleU='Hz';
iicom('wmin 6 40') % sets ci.IDopt.Selected
ci.IDopt.Fit='Complex';
ci.IDopt % display current options
```

For correct transformations using `id_rm`, you should also verify `ci.IDopt.NSNA` (number of sensors/actuators), `ci.IDopt.Reciprocity` and `ci.IDopt.Colllocated`.

For correct labels using `iipplot` you should set the abscissa, and ordinate numerator/denominator types in the data base wrapper. You can edit these values using the `iipplot properties:channel` tab. A typical script would declare frequencies, acceleration, and force using (see list with `xfopt _datatype`)

```
UFS(2).x='Freq';UFS(2).yn='Acc';UFS(2).yd='Load';UFS(2).info
```

2.1.5 Handling displayed units and labels

```
ci=iicom('curveload gartid');
ci.Stack{'Test'}.yn.unit='N';
ci.Stack{'Test'}.yd.unit='M';
iicom sub
```

2.1.6 SDT 5 compatibility

With *SDT 6*, global variables are no longer used and `iipplot` supports display of curves in other settings than identification.

If you have saved *SDT 5* datasets into a `.mat` file, `iicom('CurveLoad FileName')` will place the data into an *SDT 6* stack properly. Otherwise for an operation similar to that of *SDT 5*, where you use `XF(1).xf` rather than the new `ci.Stack{'Test'}.xf`, you should start `iipplot` in its identification mode and obtain a pointer `XF` (*SDT handle* object) to the data sets (now stored in the figure itself) as follows

```
>> ci=iicom('curveid');XF=iicom(ci,'curveXF')
```

```
XF (curve stack in figure 2) =
```

```
XF(1) : [.w    0x0, xf    0x0] 'Test'  : response (general or unknown)
XF(2) : [.w    0x0, xf    0x0] 'IdFrfr' : response (general or unknown)
XF(3) : [.w    0x0, xf    0x0] 'IIxh'  : response (general or unknown)
XF(4) : [.w    0x0, xf    0x0] 'IIxi'  : response (general or unknown)
XF(5) : [.po   0x0, res   0x0] 'IdMain' : shape data
XF(6) : [.po   0x0, res   0x0] 'IdAlt'  : shape data
```

The following table lists the global variables that were used in *SDT 5* and the new procedure to access those fields which should be defined directly.

<code>XFdof</code>	described DOFs at which the responses/shapes are defined, see <code>.dof</code> field for response and shape data in the <code>xfopt</code> section, was a global variable pointed at by the <code>ci.Stack{'name'}.dof</code> fields.
<code>IDopt</code>	which contains options used by identification routines, see <code>idopt</code>) is now stored in <code>ci.IDopt</code> .
<code>IIw</code>	was a global variable pointed at by the <code>ci.Stack{'name'}.w</code> fields.
<code>IIxf</code>	(main data set) was a global variable pointed at by the <code>ci.Stack{'Test'}.xf</code> fields.
<code>IIxe</code>	(identified model) was a global variable pointed at by the <code>ci.Stack{'IdFrfr'}.xf</code> fields.
<code>IIxh</code>	(alternate data set) was a global variable pointed at by the <code>ci.Stack{'IIxh'}.xf</code> fields.
<code>IIxi</code>	(alternate data set) was a global variable pointed at by the <code>ci.Stack{'IIxi'}.xf</code> fields.
<code>IIpo</code>	(main pole set) was a global variable pointed at by the <code>ci.Stack{'IdMain'}.po</code> fields.
<code>IIres</code>	(main residue set) was a global variable pointed at by the <code>ci.Stack{'IdMain'}.res</code> fields.
<code>IIpo1</code>	(alternate pole set) was a global variable pointed at by the <code>ci.Stack{'IdAlt'}.po</code> fields.
<code>IIres1</code>	(alternate residue set) was a global variable pointed at by the <code>ci.Stack{'IdAlt'}.res</code> fields.
<code>XF</code>	was a global variable pointed holding pointers to data sets (it was called a database wrapper). The local pointer variable <code>XF</code> associated with a given <code>iiplot</code> figure can be found using <code>CurrentFig=2;ci=iiplot(CurrentFig);XF=iicom(ci,'curveXF')</code> . The normalized datasets for use with <code>idcom</code> are generated using <code>ci=idcom;XF=iicom(ci,'curvexf')</code> . They contain four response datasets (<code>XF('Test')</code> to <code>XF('IdFrfr')</code>) and two shape datasets (<code>XF('IdMain')</code> and <code>XF('IdAlt')</code>).

2.1.7 iiplot for signal processing

`iiplot` figure lets you perform standard signal processing operations (FFT, MMIF, filtering...) directly from the GUI. Opening `iiplot` properties figure, they are accessible through the contextual menu `compute` (right click on the curve list in the Stack tab). Once an operation has been performed, its parameters can be edited in the GUI, and it can be recomputed using the `Recompute` button.

Following example illustrates some signal processing commands.

```
[mdl,def]=fe_time('demobar10-run'); % build mdl and perform time computation
cf=feplot(2); cf.model=mdl; cf.def=def;

ci=iipplot(3);
fecom(cf,'CursorOnIplot') % display deformations in iipplot

% all following operations can be performed directly in the GUI:
% see the list of curves contained in iipplot figure, Stack tab:
iicom(ci,'pro');iicom(ci,'curtab Stack');
% compute FFT of deformations. Name of entry 'feplot(2)_def(1)'
ename=ci.Stack(:,2); ename=ename{strncmp(ename,'feplot',5)};
ii_mmif('FFT',ci,ename) % compute
fname=sprintf('fft(%s)',ename);
iicom(ci,'curtab Stack',fname); % show FFT options that are editable
% edit options & Recompute:
ci.Stack{fname}.Set={'fmax',50};
iicom(ci,'curtab Stack',fname,'Recompute');

% filter and display (the bandpass removes a lot of transient)
ii_mmif('BandPass -fmin 40 -fmax 50',ci,ename) % compute
fname=sprintf('bandpass(%s)',ename);
ci.Stack{fname}.Set={'fmin',10,'fmax',20};
iicom(ci,'curtab Stack',fname,'Recompute');
iicom(ci,'iix',{ename,fname});
```

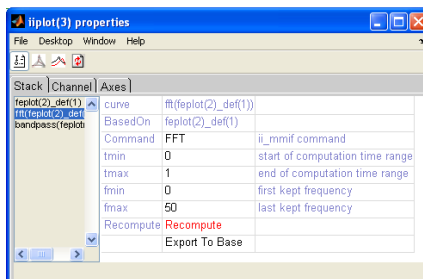


Figure 2.5: GUI for FFT computation

2.1.8 iipplot FAQ

This section lists various questions that were not answered elsewhere.

- **How do I display a channel with an other channel in abscissa?**

The low level call `ci.ua.ob(1,11)=channel;` defines the channel number `channel` of the displayed curve as the abscissa of other channels.

```
ci.ua.ob(1,11)=3; % define channel 3 as abscissa
iipplot;          % display the changes
set(ci.ga,'XLim',[0 1e-3]); % redefine axis bounds
```

- **Channel selection in multi-dimensional arrays**

```
% sdtweb('demosdt.m#DemoGartteCurve') % FRF with 2 damping levels
ci=iipplot(demosdt('demogarttecurve'))
ci.Stack{'New'}
iicom(ci,'ChAllzeta')
```

2.2 Modal test: geometry declaration and data acquisition/import

Before actually taking measurements, it is good practice to prepare a **wire frame-display** (section 2.2.1 and section 4.1.4 for other examples) and define the sensor configuration (section 2.2.2).

The information is typically saved in a specific `.m` file which should look like the `gartte` demo without the various plot commands. The `d.pre` demo also talks about test preparation.

2.2.1 Modal test geometry declaration

A wire-frame model is composed of node and connectivity declarations.

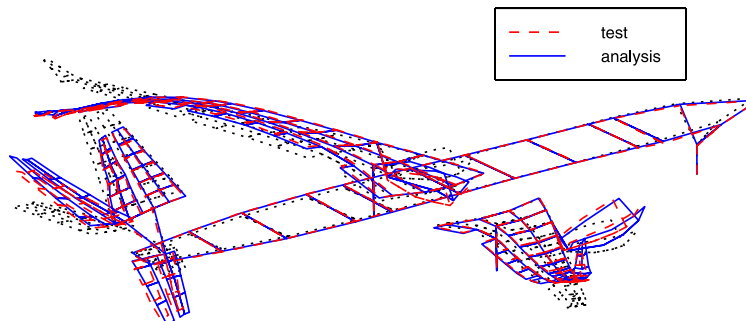


Figure 2.6: Test analysis : wire-frame model.

Starting from scratch (if you have not imported your geometry from universal files). You can declare nodes and wire frame lines using the `fecom` Add editors. Test wire frames are simply groups of beam1 elements with an `EGID` set to -1. For example in the two bay truss (see section 4.1.4)

```
cf=fepplot;cf.model='reset';
% fecom('AddNode') would open a dialog box
fecom('AddNode',[0 1 0; 0 0 0]); % add nodes giving coordinates
fecom('AddNode',[3 1 1 0;4 1 0 0]); % NodeId and xyz
fecom('AddNode',[5      0 0 0    2 0 0;
                6      0 0 0    2 1 0]);
% fecom('AddLine') would add cursor to pick line (see below)
fecom('AddLine',[1 3 2 4 3]); % continuous line in first group
fecom('AddLine',[3 6 0 6 5 0 4 5 0 4 6]); % 0 for discontinuities
fecom('Curtab:Model','Edit')
%fecom('save') % will let you save the model to a mat file
feutilb('write',cf.mdl) % generates a script
```

Note that

- `fecom(cf,'AddLine')`, use after node declaration, starts a cursor letting you build the wire-frame line graphically. Click on nodes continue the line, while the context menu allows breaks, last point removal, exit, and display of the commands in the MATLAB command window. This procedure is particularly useful if you already have a FEM model of your test article.
- `fecom(cf,'AddElt')` accessible in the `Model>Edit` tab can be used to add surface or volume elements graphically.

- the `curor:3DLinePick` command in the `feplot` axis context menu is a general SDT mechanism to pick node numbers.
- other GUI based mesh editing tools are described in section 4.1.6.
- `femesh ObjectBeamLine` and related commands are also typically used to define the experimental mesh (see also `feutil`).
- If you have a FE mesh, you should define the wireframe as a set of sensors, see section 3.1.1.

The `feplot` and `fecom` functions provide a number of tools that are designed to help in visualizing test results. You should take the time to go through the `gartid`, `gartte` and `gartco` demos to learn more about them.

2.2.2 Sensor/shaker configurations

The geometry declaration defines fields `.Node` and `.Elt`. The next step is to declare sensors. Once a sensor configuration defined and consistent with input/output pair declarations in measurements (see section 2.1.4), you can directly animate measured shapes (called Operational Deflection Shapes) as detailed in section 2.2.4. Except for roving hammer tests, the number of input locations is usually small and only used for MIMO identification (see section 2.4).

In the basic configuration with translation sensors, sensor declaration is simply done with a `.tdof` field. Acceptable forms are

- a DOF definition vector (see `mdof`) allows the description of translation DOFs in global directions. The convention that DOFs `.07` to `.09` correspond to translations in the $-x$, $-y$, $-z$ directions is implemented specifically for the common case where test sensors are oriented this way.
- a 5 column format (`[SensID NodeID tx ty tz]` giving a sensor identifier (integer or real), a node identifier (positive integer), and the measurement direction in the test mesh axes. This format supports arbitrary orientation.
- a 2 column form `DOF` where each DOF is associated with a local basis, that must be defined in `TEST.bas`.
- the tabular (cell array) definition of sensors and their position, which is more appropriate for large configurations, and is described in section 4.3.2.

The definition of sensors through a `.tdof` field is the simplest configuration. For more general setups, see section 4.3 for sensor definitions and section 4.3.4 for topology correlation.

For **interpolation of unmeasured DOFs** see section 3.3.2.

The following illustrates the first two forms

```
TEST=demosdt('DemoGartteWire');

% simply give DOFs (as a column vector)
TEST.tdof = [1011.03 1001.03 2012.07 1012.03 2005.07 1005.03 1008.03 ...
  1111.03 1101.03 2112.07 1112.03 2105.07 1105.03 1108.03 1201.07 ...
  2201.08 3201.03 1206.03 1205.08 1302.08 2301.07 1301.03 2303.07 1303.03]';

% Transform to 5 column format, which allow arbitrary orientation
TEST.tdof=fe_sens('tdof',TEST);TEST.tdof

feplot(TEST) % With a .tdof field, a SensDof,Test is defined automatically
fecom('curtab Cases','Test');fecom('ProViewOn')

% You can now display FRFs or modes using
ci=iicom('curveload gartid'); % load data
fecom(';ProviewOff;Showline')
% Display FRF
cf.def=ci.Stack{'Test'}; % automatically uses sensor definition 'Test'
% Identify and display mode
idcom('e .05 6.5')
cf.def=ci.Stack{'IdAlt'}; % automatically uses sensor definition 'Test'
```

This new example, mixes all 3 forms

```
cf=demosdt('demogartteplot') % Load data

% simply give DOFs
cf.mdl=fe_case(cf.mdl,'sensdof','Test', ...
[1011.03 1001.03 2012.07 1012.03 2005.07 1005.03 1008.03 ...
1111.03 1101.03 2112.07 1112.03 2105.07 1105.03 1108.03 1201.07]');

% Give DOF defined in a local basis
cf.mdl=fe_case(cf.mdl,'sensdof append','Test', ...
```

```
[2201.01 1; 3201.03 0; 1206.03 0; 1205.01 1; 1302.01 1]);

% Give identifier, node and measurement direction
cf.mdl=fe_case(cf.mdl,'sends dof append','Test', ...
[1 2301 -1 0 0; 2 1301 0 0 1; 3 2303 -1 0 0; 4 1303 0 0 1]);
fecom('curtab Cases','Test');fecom('ProViewOn')
```

It is also fairly common to glue sensors normal to a surface. The sensor array table (see section 4.3.2) **is the easiest approach** for this objective since it allows mixing global, normal, triax, laser, ... sensors. The following example shows how this can also be done by hand how to obtain normals to a volume and use them to define sensors.

```
% This is an advanced code sample
model=demosdt('demo ubeam');

MAP=feutil('getnormal node MAP',model.Node, ...
    feutil('selelt selface',model)); % select outer boundary for normal

i1=ismember(MAP.ID,[360 365 327 137]); % nodes where sensors are placed
MAP.ID=MAP.ID(i1);MAP.normal=MAP.normal(i1,:);
model=fe_case(model,'sends dof','test', ...
    [(1:length(MAP.ID))' MAP.ID MAP.normal]);

% display the mesh and sensors
cf=clean_get_uf('feplotcf',model);
cf.sel(1)='groupall';cf.sel(2)='-test';
cf.o(1)={'sel2ty7','edgecolor','r','linewidth',2}
```

2.2.3 Data acquisition

The *SDT* does not intend to support the acquisition of test data since tight integration of acquisition hardware and software is mandatory. A number of signal processing tools are gradually being introduced in *iiplot* (see *ii_mmif FFT* or *fe_curve h1h2*). But the current intent is not to use *SDT* as an acquisition driver. The following example generates transfers from time domain data

```
frame=fe_curve('Testacq'); % 3 DOF system response
% Time vector in .X field, measurements in .Y columns
frf=fe_curve('h1h2 1',frame); % compute FRF
```

```
ci=iicom('Curveid');ci.Stack{'Test'}.w=frf.X; ci.Stack{'Test'}.xf=frf.H1;
iicom('Sub');
```

You can find theoretical information on data acquisition for modal analysis in Refs. [2][3][4][5][6].

Import procedures are described in section 2.1.4. The following table gives a partial list of systems with which the *SDT* has been successfully interfaced.

Vendor	Procedure used
Bruel & Kjaer	Export data from Pulse to the UFF and read into <i>SDT</i> with <code>ufread</code> or use the Bridge To Matlab software and <code>pulse2sdt</code> .
Dactron	Export data from RT-Pro software to the UFF. Use the <i>Active-X API</i> to drive the Photon from MATLAB see <code>photon</code> .
LMS	Export data from LMS CADA-X to UFF.
MathWorks	Use <i>Data Acquisition</i> and <i>Signal Processing</i> toolboxes to estimate FRFs and create a script to fill in <i>SDT</i> information (see section 2.1.4).
MTS	Export data from IDEAS-Pro software to UFF.
Polytec	Export data from PSV software to UFF.
Spectral Dynamics	Create a Matlab script to format data from SigLab to <i>SDT</i> format.

2.2.4 Animating test data, operational deflection shapes

Operational Deflection Shapes is a generic name used to designate the spatial relation of forced vibration measured at two or more sensors. Time responses of simultaneously acquired measurements, frequency responses to a possibly unknown input, transfer functions, transmissibilities, ... are example of ODS.

When the response is known at global DOFs no specific information is needed to relate node motion and measurements. Thus any deformation with DOFs will be acceptable. The two basic displays are a wire-frame defined as a FEM model or a wire-frame defined as a `SensDof` entry.

```
% A wire frame and Identification results
[TEST,IdMain]=demosdt('DemoGartteWire')
cf=feplot(TEST); % wire frame
cf.def=IdMain; % to fill .dof field see sdtweb('diiplo#xfread')
% or the low level call : cf.def={IdMain.res.',IdMain.dof,IdMain.po}
```



```

% Sensors in a model and identification results
cf=demosdt('demo gartfeplot'); % load FEM
TEST=demosdt('demo garttwire'); % see sdtweb('pre#presen')
cf.mdl=fe_case(cf.mdl,'sensdof','outputs',TEST)

cf.sel='-outputs'; % Build a selection that displays the wire frame
cf.def=IdMain; % Display motion on sensors

fecom('curtab Plot');

```

When the response is known at sensors that need to be combined (non global directions, non-orthogonal measurements, ...) a **SensDof** entry must really be defined.

When displaying responses with **iipplot** and a test geometry with **feplot**, **iipplot** supports an ODS cursor. Run **demosdt('DemoGartteOds')** then open the context menu associated with any **iipplot** axis and select **ODS Cursor**. The deflection show in the **feplot** figure will change as you move the cursor in the **iipplot** window.

More generally, you can use **fecom InitDef** commands to display any shape as soon as you have a defined geometry and a response at DOFs. The **Deformations** tab of the **feplot** properties figure then lets you select deformations within a set.

```

[cf,ci]=demosdt('DemoGartteOds')
cf.def=ci.Stack{'Test'};
% or the low level call :
% cf.def={ci.Stack{'Test'}.xf,ci.Stack{'Test'}.dof,ci.Stack{'Test'}.w}
fecom('CurTab Plot');

```

You can also display the actual measurements as arrows using

```
cf.sens=ci.Stack{'Test'}.dof; fecom ShowArrow; fecom scc1;
```

For a tutorial on the use of **feplot** see section 4.1.

2.3 Identification of modal properties

Identification is the process of estimating a parametric model (poles and modeshapes) that accurately represents measured data. The main algorithm proposed in the *SDT* is a frequency domain output error method that builds a model in the pole residue form (see section 5.6) through a tuning strategy. Key theoretical notions are pole/residue models, residual terms, and the relation between residues and modeshapes (see `cpx`).

Section 2.3.2 gives a tutorial on the standard procedure. Theoretical details about the underlying algorithm are given in section 2.3.3. Section 2.3.4 addresses its typical shortcomings. Other methods implemented in the *SDT* but not considered as efficient are addressed in later sections.

For the handling of MIMO tests, reciprocity,... see section 2.4. The `gartid` script gives real data and an identification result for the GARTEUR example. The `demo_id` script analyses a simple identification example.

2.3.1 The `idcom` interface

For identification, the `idcom` interface uses a standard set of curves and identification options accessible from the `IDopt` tab or from the command line through the pointer `ci.IDopt`. `idcom(ci)` turns the environment on, `idcom(ci, 'Off')` removes options but not datasets.

```
ci=iicom('Curveid'); ci.Stack
```

```
'curve'   'Test'       [1x1 struct]
'curve'   'IdFrf'      [1x1 struct]
'curve'   'IdMain'    [1x1 struct]
'curve'   'IdAlt'    [1x1 struct]
```

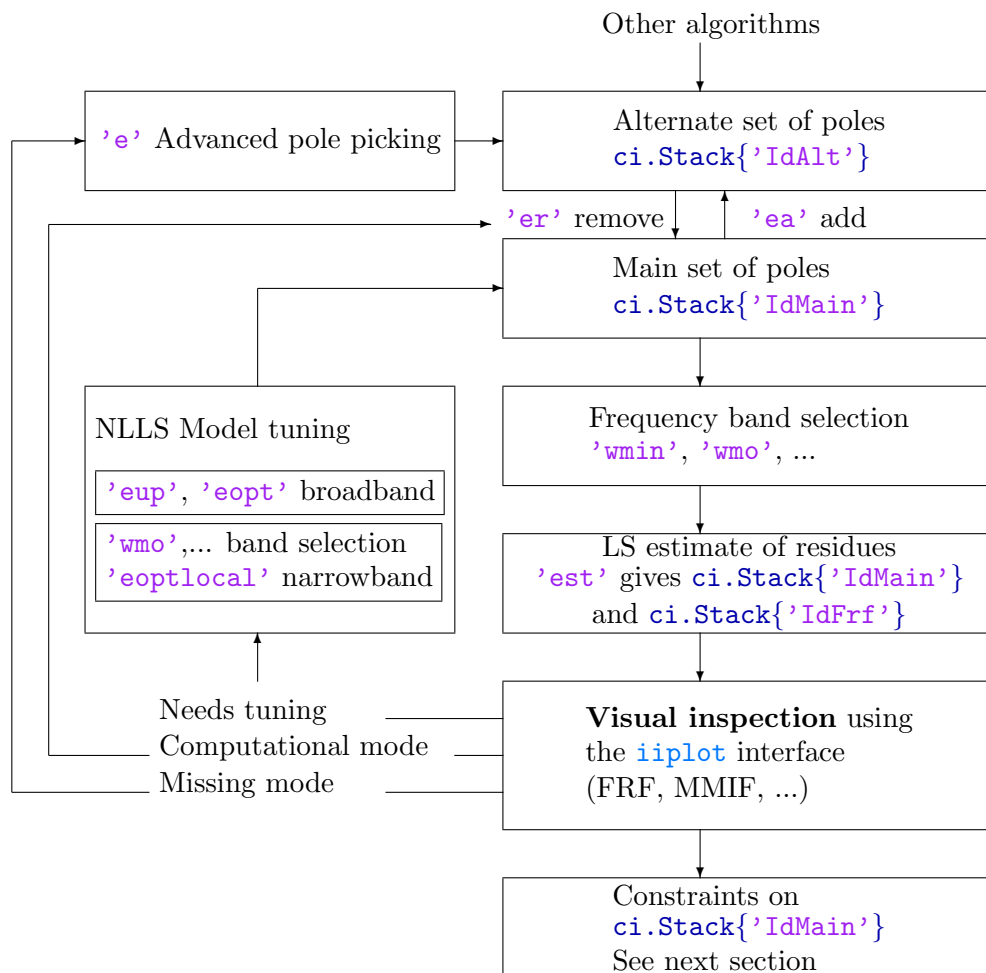
- `Test` contains measured frequency response functions. See section 2.1.4 ways to initialize this data set.
- `IdFrf` contains the synthesis of transfers associated with given set of transfers.
- `IdMain` contains the main set of modes (poles and residues)
- `IdAlt` contains the alternate set of modes (poles and residues)

2.3.2 The `id_rc` procedure step by step

The `id_rc` identification method is based on an iterative refinement of the poles of the current model. Illustrated by the diagram below.

The main steps of the methodology are

- finding initial pole estimates (with the narrow band estimator, `idcom e` command), adding missed poles, removing computational poles (using the arrows between the main and alternate pole sets, `ea` and `er` commands)
- estimating residues and residual terms for a given set of poles (`est` command/button or direct call to `id_rc`)
- optimizing poles (and residues) of the current model using a broad or narrow band update (`eup`, `eopt`, `eoptlocal`, ... commands/buttons, with frequency band selection using the `wmin`, `wmo`, ... commands/buttons)



After verification of the **Options** tab of the `idcom` GUI figure, the **Identification** tab shown below gives you easy access to these steps (to open this figure, just run `idcom` from the MATLAB prompt). More details on how to proceed for each step are given below using data of the `demo_id` script.

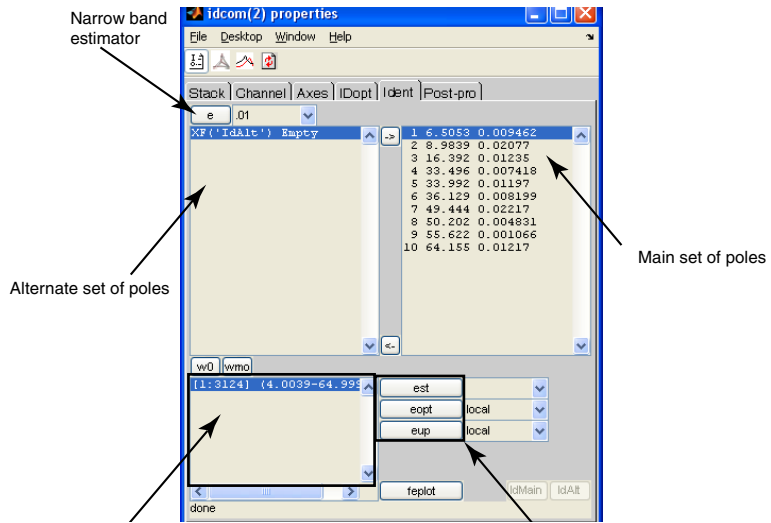


Figure 2.7: `idcom` tab in the `iiplot` property figure

The iteratively refined model is fully characterized by its poles (and the measured data). It is thus convenient to cut/paste the pole estimates into and out of a text editor (you can use the context menu of the main pole set to display this in the MATLAB command window). Saving the current pole set in a text file as the lines

```
ci.Stack{'IdMain'}.po = [...
    1.1298e+02    1.0009e-02
    1.6974e+02    1.2615e-02
    2.3190e+02    8.9411e-03];
```

gives you all you need to recreate an identified model (even if you delete the current one) but also lets you refine the model by adding the line corresponding to a pole that you might have omitted. The context menu associated with the pole set listboxes lets you easily generate this list.

1 finding initial pole estimates, adding missed poles, removing computational poles

Getting an initial estimate of the poles of the model is the first difficulty. Dynamic responses of structures, typically show lightly damped resonances. The easiest way to build an initial estimate of the poles is thus to use a sequence of narrow band single pole estimations near peaks of the response or minima of the Multivariate

Mode Indicator function (use `iicom Showmmi` and see `ii_mmif` for a full list of mode indicator functions).

The `idcom e` command (based on a call to the `ii_poest` function) lets you to indicate a frequency (with the mouse or by giving a frequency value) and seeks a single pole narrow band model near this frequency (the pole is stored in `ci.Stack{'IdAlt'}`). Once the estimate found the `iipplot` drawing axes are updated to overlay `ci.Stack{'Test'}` and `ci.Stack{'IdFrff'}`.

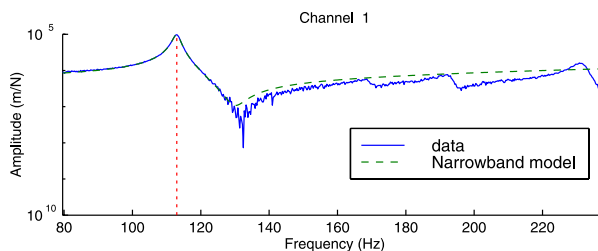


Figure 2.8: Pole estimation.

In the plot shown above the fit is clearly quite good. This can also be judged by the information displayed by `ii_poest`

```
LinLS: 1.563e-11, LogLS 8.974e-05, nw 10
mean(reLE) 0.00, scatter 0.00
Found pole at 1.1299e+02 9.9994e-03
```

which indicates the linear and quadratic costs in the narrow frequency band used to find the pole, the number of points in the band, the mean relative error (norm of difference between test and model over norm of response which should be below 0.1), and the level of scatter (norm of real part over norm of residues, which should be small if the structure is close to having modal damping).

If you have a good fit and the pole differs from poles already in your current model, you can add the estimated pole (add poles in `ci.Stack{'IdAlt'}` to those in `ci.Stack{'IdMain'}`) using the `idcom ea` command (or the associated button). If the fit is not appropriate you can change the number of selected points/bandwidth and/or the central frequency. In rare cases where the local pole estimate does not give appropriate results you can add a pole by just indicating its frequency (`f` command) or you can use the polynomial (`id_poly`), direct system parameter (`id_dspi`), or any other identification algorithm to find your poles. You can also consider the

`idcom find` command which uses the MMIF to seek poles that are present in your data but not in `ci.Stack{'IdMain'}`.

In cases where you have added too many poles to your current model, the `idcom er` command then lets you remove certain poles.

This phase of the identification relies heavily on user involvement. You are expected to visualize the different FRFs (use the +/- buttons/keys), check different frequency bands (zoom with the mouse and use `iicom w` commands), use Bode, Nyquist, MMIF, etc. (see `iicom Show` commands). The `iiplot` graphical user interface was designed to help you in this process and you should learn how to use it (you can get started in section 2.1).

2 estimating residues and residual terms

Once a model is created (you have estimated a set of poles), `idcom est` determines residues and displays the synthesized FRFs stored in `ci.Stack{'IdFrf'}`. A careful visualization of the data often leads to the discovery that some poles are missing from the initial model. The `idcom e` and `ea` commands can again be used to find initial estimates for the missing poles.

The need to add/remove poles is determined by careful examination of the match between the test data `ci.Stack{'Test'}` and identified model `ci.Stack{'IdFrf'}`. You should take the time to scan through different sensors, look at amplitude, phase, Nyquist, ...

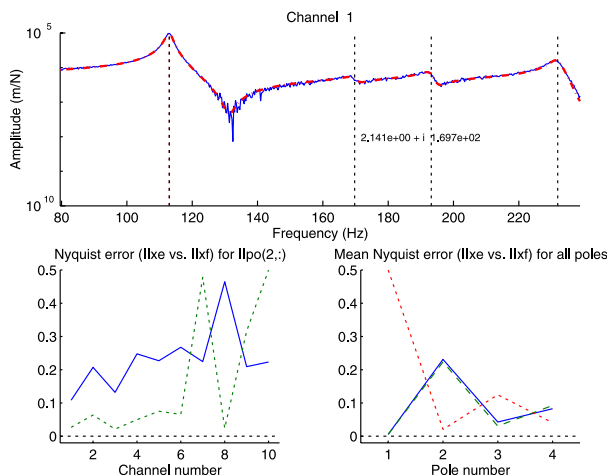


Figure 2.9: Pole estimation.

Quality and error plots are of particular interest. The quality plot (lower right, obtained with `iicom Showqual`) gives an indication of the quality of the fit near each pole. Here pole 2 does not have a very good fit (relative error close to 0.2) but the response level (dotted line) is very small. The error plot (lower left, obtained with `iicom Showerr`) shows the same information for the current pole and each transfer function (you change the current pole by clicking on pole lines in the top plot). Here it confirms that the relative Nyquist error is close to 0.2 for most channels. This clearly indicates the need to update this pole as detailed in the next section (in this example, the relative Nyquist error is close to 0.1 after updating).

3 updating poles of the current model using a broad or narrow frequency band update

The various procedures used to build the initial pole set (see step 1 above) tend to give good but not perfect approximations of the pole sets. In particular, they tend to optimize the model for a cost that differs from the broadband quadratic cost that is really of interest here and thus result in biased pole estimates.

It is therefore highly desirable to perform non-linear update of the poles in `ci.Stack{'IdMain'}`. This update, which corresponds to a Non-Linear Least-Squares minimization, can be performed using the commands `idcom eup` (`id_rc` function) and `eopt` (`id_rcopt` function). The optimization problem is very non linear and non convex, good results are thus only found when improving results that are already acceptable (the result

of phase 2 looks similar to the measured transfer function).

When using the `eup` command `id_rc` starts by reminding you of the currently selected options (accessible from the figure pointer `ci.IDopt`) for the type of residual corrections, model selected and, when needed, partial frequency range selected

```
Low and high frequency mode correction
Complex residue symmetric pole pattern
```

the algorithm then does a first estimation of residues and step directions and outputs

```
% mode#   dstep (%)   zeta       fstep (%)   freq
      1      10.000   1.0001e-02  -0.200     7.1043e+02
      2     -10.000   1.0001e-02   0.200     1.0569e+03
      3      10.000   1.0001e-02  -0.200     1.2176e+03
      4      10.000   1.0001e-02  -0.200     1.4587e+03
Quadratic cost
4.6869e-09
Log-mag least-squares cost
6.5772e+01
how many more iterations? ([cr] for 1, 0 to exit) 30
```

which indicates the current pole positions, frequency and damping steps, as well as quadratic and logLS costs for the complete set of FRFs. These indications and particularly the way they improve after a few iterations should be used to determine when to stop iterating.

Here is a typical result after about 20 iterations

```
% mode#   dstep (%)   zeta       fstep (%)   freq
      1     -0.001   1.0005e-02   0.000     7.0993e+02
      2    -0.156   1.0481e-02  -0.001     1.0624e+03
      3    -0.020   9.9943e-03   0.000     1.2140e+03
      4    -0.039   1.0058e-02  -0.001     1.4560e+03
Quadratic cost
4.6869e-09 7.2729e-10 7.2741e-10 7.2686e-10 7.2697e-10
Log-mag least-squares cost
6.5772e+01 3.8229e+01 3.8270e+01 3.8232e+01 3.8196e+01
how many more iterations? ([cr] for 1, 0 to exit) 0
```

Satisfactory convergence can be judged by the convergence of the quadratic and logLS cost function values and the diminution of step sizes on the frequencies and damping ratios. In the example, the damping and frequency step-sizes of all the

poles have been reduced by a factor higher than 50 to levels that are extremely low. Furthermore, both the quadratic and logLS costs have been significantly reduced (the leftmost value is the initial cost, the right most the current) and are now decreasing very slowly. These different factors indicate a good convergence and the model can be accepted (even though it is not exactly optimal).

The step size is divided by 2 every time the sign of the cost gradient changes (which generally corresponds passing over the optimal value). Thus, you need to have all (or at least most) steps divided by 8 for an acceptable convergence. Upon exit from `id_rc`, the `idcom eup` command displays an overlay of the measured data `ci.Stack{'Test'}` and the model with updated poles `ci.Stack{'IdFrfr'}`. As indicated before, you should use the error and quality plots to see if mode tuning is needed.

The optimization is performed in the selected frequency range (`idopt wmin` and `wmax` indices). It is often useful to select a narrow frequency band that contains a few poles and update these poles. When doing so, model poles whose frequency are not within the selected band should be kept but not updated (use the `euplocal` and `eoptlocal` commands). You can also update selected poles using the `'eup ' i'` command (for example if you just added a pole that was previously missing).

`id_rc` (`eup` command) uses an ad-hoc optimization algorithm, that is not guaranteed to improve the result but has been found to be efficient during years of practice. `id_rcopt` (`eopt` command) uses a conjugate gradient algorithm which is guaranteed to improve the result but tends to get stuck at non optimal locations. You should use the `eopt` command when optimizing just one or two poles (for example using `eoptlocal` or `'eopt ' i'` to optimize different poles sequentially).

In many practical applications the results obtained after this first set of iterations are incomplete. Quite often local poles will have been omitted and should now be appended to the current set of poles (going back to step 1). Furthermore some poles may be diverging (damping and/or frequency step not converging towards zero). This divergence will occur if you add too many poles (and these poles should be deleted) and may occur in cases with very closely spaced or local modes where the initial step or the errors linked to other poles change the local optimum for the pole significantly (in this case you should reset the pole to its initial value and restart the optimization).

Once a good complex residue model obtained, one often seeks models that verify other properties of minimality, reciprocity or represented in the second order mass, damping, stiffness form. These approximations are provided using the `id_rm` and

`id_nor` algorithms as detailed in section 2.4.

2.3.3 Background theory

The `id_rc` algorithm (see [7][8]) seeks a non linear least squares approximation of the measured data

$$p_{\text{model}} = \arg \min \sum_{j,k,l=1}^{NS,NA,NW} \left(\alpha_{jk(\text{id})}(\omega_l, p) - \alpha_{jk(\text{test})}(\omega_l) \right)^2 \quad (2.1)$$

for models in the nominal pole/residue form (also often called partial fraction expansion [9])

$$[\alpha(s)] = \sum_{j \text{ identified}} \left(\frac{[R_j]}{s - \lambda_j} + \frac{[\bar{R}_j]}{s - \bar{\lambda}_j} \right) + [E] + \frac{[F]}{s^2} = [\Phi(\lambda_j, s)] [R_j, E, F] \quad (2.2)$$

or its variants detailed under `res` page 165.

These models are linear functions of the residues and residual terms $[R_j, E, F]$ and non linear functions of the poles λ_j . The algorithm thus works in two stages with residues found as solution of a linear least-square problem and poles found through a non linear optimization.

The `id_rc` function (`idcom eup` command) uses an ad-hoc optimization where all poles are optimized simultaneously and steps and directions are found using gradient information. This algorithm is usually the most efficient when optimizing more than two poles simultaneously, but is not guaranteed to converge or even to improve the result.

The `id_rcopt` function (`idcom eopt` command) uses a gradient or conjugate gradient optimization. It is guaranteed to improve the result but tends to be very slow when optimizing poles that are not closely spaced (this is due to the fact that the optimization problem is non convex and poorly conditioned). The standard procedure for the use of these algorithms is described in section 2.3.2. Improved and more robust optimization strategies are still considered and will eventually find their way into the *SDT*.

2.3.4 When `id_rc` fails

This section gives a few examples of cases where a direct use of `id_rc` gave poor results. The proposed solutions may give you hints on what to look for if you encounter a particular problem.

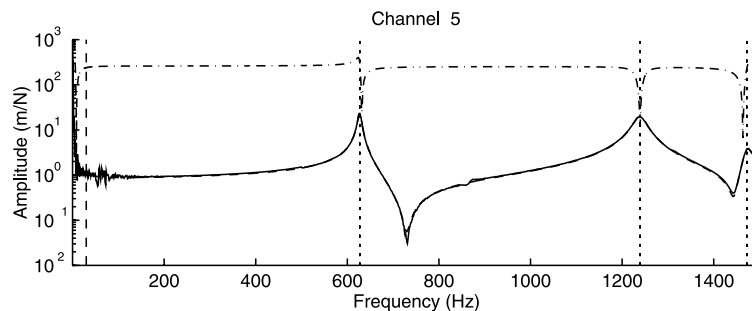


Figure 2.10: Identification problem with low frequency error found for piezoelectric accelerometers

In many cases frequencies of estimated FRFs go down to zero. The first few points in these estimates generally show very large errors which can be attributed to both signal processing errors and sensor limitations. The figure above, shows a typical case where the first few points are in error by orders of magnitude. Of two models with the same poles, the one that keeps the low frequency erroneous points (- - -) has a very large error while a model truncating the low frequency range (- - -) gives an extremely accurate fit of the data (—).

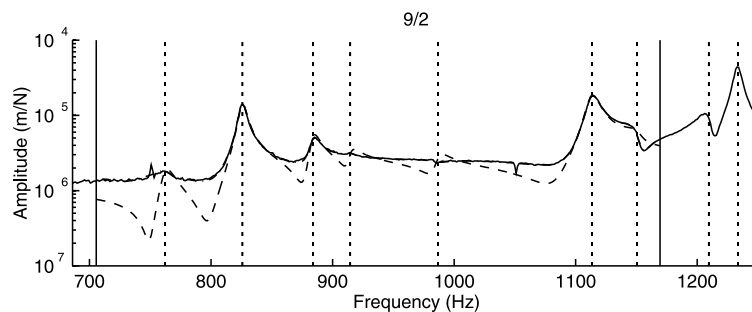


Figure 2.11: Identification problem linked to the proximity of influent out of band modes

The fact that appropriate residual terms are needed to obtain good results can have significant effects. The figure above shows a typical problem where the identification is performed in the band indicated by the two vertical solid lines. When using the 7

poles of the band, two modes above the selected band have a strong contribution so that the fit (---) is poor and shows peaks that are more apparent than needed (in the 900-1100 Hz range the FRF should look flat). When the two modes just above the band are introduced, the fit becomes almost perfect (- - -) (only visible near 750 Hz).

Keeping out of band modes when doing narrow band pole updates is thus quite important. You may also consider identifying groups of modes by doing sequential identifications for segments of your test frequency band [8].

The example below shows a related effect. A very significant improvement is obtained when doing the estimation while removing the first peak from the band. In this case the problem is actually linked to measurement noise on this first peak (the Nyquist plot shown in the lower left corner is far from the theoretical circle).

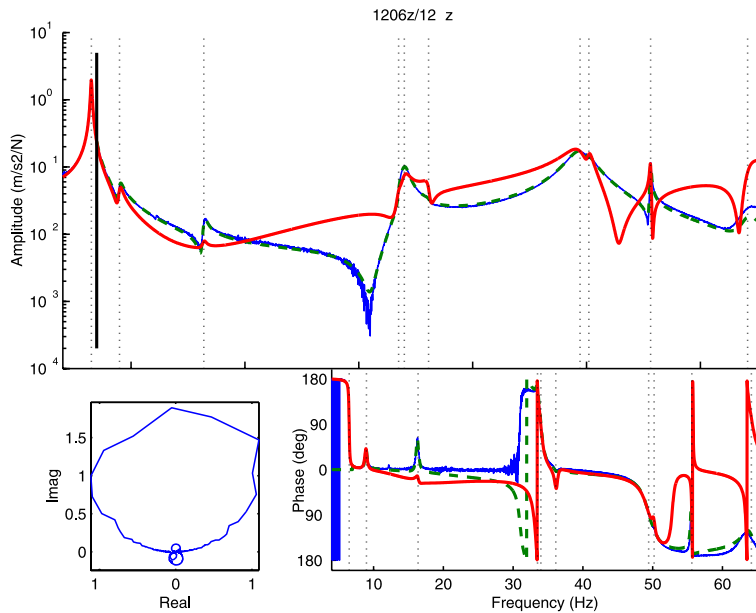


Figure 2.12: Identification problem linked to measurement noise at a major resonance

Other problems are linked to poor test results. Typical sources of difficulties are

- mass loading (resonance shifts from FRF to FRF due to batch acquisition with displaced sensors between batches),

- leakage in the estimated FRFs,
- significant non-linearities (inducing non-symmetric resonances or resonance shifts for various excitation positions),
- medium frequency range behavior (the peaks of more than a few modes overlay significantly it can be very hard to separate the contributions of each mode even with MIMO excitation).

2.3.5 Direct system parameter identification algorithm

A class of identification algorithms makes a direct use of the second order parameterization. Although the general methodology introduced in previous sections was shown to be more efficient in general, the use of such algorithms may still be interesting for first-cut analyses. A major drawback of second order algorithms is that they fail to consider residual terms.

The algorithm proposed in `id.dsapi` is derived from the direct system parameter identification algorithm introduced in Ref. [10]. Constraining the model to have the second-order form

$$\begin{aligned} [-\omega^2 I + i\omega C_T + K_T] \{p(\omega)\} &= [b_T] \{u(\omega)\} \\ \{y(\omega)\} &= [c_T] \{p(\omega)\} \end{aligned} \quad (2.3)$$

it clearly appears that for known $[c_T]$, $\{y_T\}$, $\{u_T\}$ the system matrices $[C_T]$, $[K_T]$, and $[b_T]$ can be found as solutions of a linear least-squares problem.

For a given output frequency response $\{y_T\} = \mathbf{xout}$ and input frequency content $\{u_T\} = \mathbf{xin}$, `id.dsapi` determines an optimal output shape matrix $[c_T]$ and solves the least squares problem for $[C_T]$, $[K_T]$, and $[b_T]$. The results are given as a state-space model of the form

$$\begin{aligned} \begin{Bmatrix} \dot{q} \\ \ddot{q} \end{Bmatrix} &= \begin{bmatrix} 0 & I \\ -K_T & -C_T \end{bmatrix} \begin{Bmatrix} q \\ \dot{q} \end{Bmatrix} + \begin{bmatrix} 0 \\ b_T \end{bmatrix} \{u(t)\} \\ \{y(t)\} &= [c_T \ 0] \begin{Bmatrix} q \\ \dot{q} \end{Bmatrix} \end{aligned} \quad (2.4)$$

The frequency content of the input $\{u\}$ has a strong influence on the results obtained with `id.dsapi`. Quite often it is efficient to use it as a weighting, rather than using a white input (column of ones) in which case the columns of $\{y\}$ are the transfer functions.

As no conditions are imposed on the reciprocity (symmetry) of the system matrices $[C_T]$ and $[K_T]$ and input/output shape matrices, the results of the algorithm are not directly related to the normal mode models identified by the general method. Results obtained by this method are thus not directly applicable to the prediction problems treated in section 2.4.2.

2.3.6 Orthogonal polynomial identification algorithm

Among other parameterizations used for identification purposes, polynomial representations of transfer functions (5.27) have been investigated in more detail. However for structures with a number of lightly damped poles, numerical conditioning is often a problem. These problems are less acute when using orthogonal polynomials as proposed in Ref. [11]. This orthogonal polynomial method is implemented in `id_poly`, which is meant as a flexible tool for initial analyses of frequency response functions. This function is available as `idcom poly` command.

2.4 MIMO, Reciprocity, State-space, ...

The pole/residue representation is often not the desired format. Access to transformations is provided by the post-processing tab in the `idcom` properties figure. There you can select the desired output format and the name of the variable in the base MATLAB workspace you want the results to be stored in.

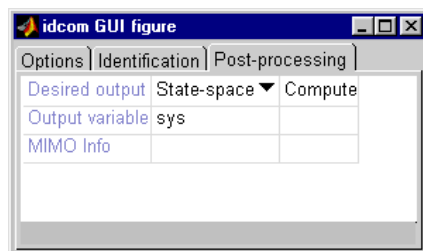


Figure 2.13: idcom interface

The `id_rm` algorithm is used for the creation of minimal and/or reciprocal pole/residue models (from the command line use `sys=id_rm(ci.Stack{'IdMain'})`). For the extra step of state-space model creation use `sys=res2ss(ci.Stack{'IdMain'})`. `nor=res2nor(ci.Stack{'IdMain'})` or `nor=id_nor(ci.Stack{'IdMain'})` allow transformations to the normal mode form. Finally direct conversions to other formats are given by `struct=res2xf(ci.Stack{'IdMain'},w)` with `w=ci.Stack'Test'.w`, and `[num,den]=res2tf(...)`. These calls are illustrated in `demo_id`.

2.4.1 Multiplicity (minimal state-space model)

Theory

As mentioned under `res` page 165, the residue matrix of a mode can be written as the product of the input and output shape matrices, so that the modal contribution takes the form

$$\frac{R_j}{s - \lambda_j} = \frac{\{c\psi_j\} \{\psi_j^T b\}}{s - \lambda_j} \quad (2.5)$$

For a single mode, the product $\{c\psi_j\} \{\psi_j^T b\}$ has rank 1. Thus for a truly MIMO test (with more than one input and output), the residue matrix found by `id_rc` usually

has full rank and cannot be written as shown in (2.5). In some cases, two poles of a structure are so close that they can be considered as a multiple pole $\lambda_j = \lambda_{j+1}$, so that

$$\frac{R_j}{s - \lambda_j} = \frac{\{c\psi_j\} \{\psi_j^T b\} + \{c\psi_{j+1}\} \{\psi_{j+1}^T b\}}{s - \lambda_j} \quad (2.6)$$

In such cases, the residue matrix $[R_j]$ has rank two. **Minimality** (i.e. rank constraint on the residue matrix) is achieved by computing, for each mode, the singular value decomposition of the residue matrix $R_j = U\Sigma V^T$. By definition of the singular value decomposition

$$[\tilde{R}_j]_{NS \times NA} = \{U_1\}_{NS \times 1} \sigma_1 \{V_1\}_{NA \times 1}^T \quad (2.7)$$

is the best rank 1 approximation (in the matrix norm sense) of R_j . Furthermore, the ratio σ_2/σ_1 is a measure of the relative error made by retaining only the first dyad. This ratio gives, for MIMO tests, an indication of the coherence of estimated mode shapes and occasionally an indication of the pole multiplicity if two poles are sufficiently close to be considered as identical (see the example below).

Minimal pole/residue models are directly linked to a state-space model of the form

$$\begin{aligned} \left(s [I]_{2N \times 2N} - [\lambda_j] \right) \{\eta\} &= [\psi^T b] \{u\} \\ \{y\} &= [c\psi] \{\eta\} \end{aligned} \quad (2.8)$$

which can then be transformed to a real valued state-space model (see [res2ss](#)) or a second order normal mode model (see section 2.4.3).

Practice

`id_rm` builds a rank constrained approximation of the residue matrix associated to each pole. When not enforcing reciprocity, the output of the call

```
ci=demosdt('Demo demo_id')
ci.IDopt.nсна=[5 2]; ci.IDopt.reci='no';
RES = id_rm(ci.Stack{'IdMain'},[1 2 1 1]);
% or low level call
[pb,cp,new_res]=id_rm(ci.Stack{'IdMain'}.res,ci.Stack{'IdMain'}.po, ...
ci.IDopt,[1 2 1 1]);
```

returns an output that has has the form

```
The system has 5 sensors and 2 actuators
FRF 7 (actuator 2 sensor 2) is collocated
Po #    freq    mul    Ratio of sing. val. to max
1      7.10e+02  2    :    0.3000 k  0.0029
2      9.10e+02  1    :    0.1000    0.0002
```

3	1.20e+03	1	:	0.0050	0.0001
4	1.50e+03	1	:	0.0300	0.0000

where the first three columns indicate pole number, frequency and retained multiplicity and the following give an indication of the difference between the full rank residue matrix and the rank constrained one (the singular value ratio should be much smaller than 1).

In the result show above, pole 1 is close to being rank 2 since the difference between the full order residue matrix and a rank 1 approximation is of the order of 30% while the difference with a rank 2 approximation is only near 0.2%.

The fact that a rank 1 approximation is not very good can be linked to actual multiplicity but more often indicates poor identification or incoherent data. For poor identification the associated pole should be updated as shown in section 2.3. For incoherent data (for example modes slightly modified due to changing shakers during sequential SIMO tests), one should perform separate identifications for each set of coherent measurements. The rank constrained approximation can then be a way to reconcile the various results obtained for each identification.

If the rank of the residue matrix is truly linked to pole multiplicity, one should try to update the identification in the vicinity of the pole: select a narrow frequency range near this pole, then create and optimize a two or more pole model as shown section 2.3.2. True modal multiplicity being almost impossible to design into a physical structure, it is generally possible to resolve such problems. Keeping multiple poles should thus only remain an intermediate step when not having the time to do better.

2.4.2 Reciprocal models of structures

Theory

In many cases, the structures tested are assumed to be *reciprocal* (the transfers force at A/response at B and force at B/response at A are equal) and one wants to build a reciprocal model. For modal contributions of the form (2.5), reciprocity corresponds to the equality of collocated input and output shape matrices

$$([c_{co1}] \{\psi_j\})^T = \{\psi_j\}^T [b_{co1}] \quad (2.9)$$

For reciprocal structures, the residue matrix associated to collocated FRFs should be symmetric. `id.rm` thus starts computing the symmetric part of the collocated residues $\hat{R}_{jco1} = (R_{jco1} + R_{jco1}^T)/2$. This matrix being symmetric, its singular value decomposition is given by $\hat{R}_{jco1} = U_{co1} \Sigma_{co1} V_{co1}^T$ which leads to the reciprocal

input and output shape matrices

$$\{c_{co1}\psi_j\} = \{\psi_j^T b_{co1}\}^T = \sqrt{\sigma_{1co1}} \{U_{1co1}\} \quad (2.10)$$

Typically, there are many more sensors than inputs. The decomposition (2.10) is thus only used to determine the collocated input shape matrices and the output shape matrices at all sensors are found as solution of a least square problem $\{c\psi_j\} = [R_j] \{\psi_j^T b_{co1}\}^+$ which does require that all inputs have a collocated sensor.

Reciprocity provides scaled input and output shape matrices. This scaling is the same as that obtained with the analytical scaling condition (5.20). The interest of using reciprocal models is to predict non measured transfer functions.

Practice

When collocated transfer functions are declared and `ci.IDopt.Reciprocity='1 FRF'` or `MIMO`, `id_rm` seeks a minimal and reciprocal approximation to the model. For the call

```
ci=demosdt('Demo demo_id')
ci.IDopt.nсна=[5 2]; ci.IDopt.Col=[1 7];
ci.IDopt.reci='mimo';
RES = id_rm(ci.Stack{'IdMain'},[1 1 1 1]);
ci.Stack{'IIxh'}=res2xf(RES,ci.Stack{'Test'}.w); iicom('IIxhOn')
% or low level call
[pb,cp,new_res,new_po]=id_rm(ci.Stack{'IdMain'}.res,ci.Stack{'IdMain'}.p
    ci.IDopt,[1 1 1 1]);
ci.Stack{'IIxh'}.xf = res2xf(new_res,new_po,ci.Stack{'Test'}.w,ci.IDopt)
iicom('IIxhOn')
```

`id_rm` shows information of the form

```
The system has 5 sensors and 2 actuators
FRF 1 (actuator 1 sensor 1) is collocated
FRF 7 (actuator 2 sensor 2) is collocated
Reciprocal MIMO system
Po#    freq    mul    sym.    rel.e.
  1    1.13e+02  1 :    0.0001  0.0002
  2    1.70e+02  1 :    0.0020  0.0040
  3    1.93e+02  1 :    0.0003  0.0005
  4    2.32e+02  1 :    0.0022  0.0044
```

where the output indicates the number of sensors and actuators, the collocated FRFs, the fact the resulting model will enforce MIMO reciprocity, and details the accuracy achieved for each mode.

The algorithm first enforces symmetry on the declared collocated transfer functions the symmetry error `sym`. shows how asymmetric the original residue matrices were. If for a given mode this number is not close to zero, the mode is poorly identified or the data is far from verifying reciprocity and building a reciprocal model makes no sense.

The algorithm then seeks a rank constrained approximation, the relative error number `rel_e`. shows how good an approximation of the initial residue matrix the final result is. If this number is larger than `.1`, you should go back to identifying a minimal but non reciprocal model, determine the actual multiplicity, and update the pole, if it is not very well identified, or verify that your data is really reciprocal.

You can check the accuracy of FRF predicted with the associated model using the synthesized FRFs (`IIxh/ci.Stack{'IIxh'}` in the example above). An alternate FRF generation call would be

```
[a,b,c,d]=res2ss(res,po,idopt);
IIxh=qbode(a,b,c,d,IIw*2*pi);
```

This more expensive computationally, but state-space models are particularly useful for coupled system analysis and control synthesis.

You can also use reciprocal models to predict the response of untested transfer functions. For example the response associated to a shaker placed at the `uind` sensor (not a collocated one) can be computed using

```
ci=demosdt('Demo demo_id')
[psib,cpsi]=id_rm(ci.Stack{'IdMain'}.res,ci.Stack{'IdMain'}.po, ...
    ci.IDopt,[1 1 1 1]);
uind=3; res_u = (cpsi*diag(cpsi(uind,:))).';
ci.Stack{'IdFrff'}=ci.Stack{'Test'};
ci.Stack{'IdFrff'}.xf=...
    res2xf(res_u,ci.Stack{'IdMain'}.po,ci.Stack{'Test'}.w,ci.IDopt);
iiplot
```

You should note that the `res_u` model does not contain any residual terms, since reciprocity does not give any information on those. Good predictions of unmeasured transfers are thus limited to cases where residual terms can be neglected (which is very hard to know *a priori*).

2.4.3 Normal mode form

Modal damping assumption

While the most accurate viscous damping models are obtained with a full damping matrix Γ (supported by `psi2nor` and `id_nor` as detailed in the next section), **modal damping** (where Γ is assumed diagonal which is valid assumption when (2.15) is verified) is used in most industrial applications and is directly supported by `id_rc`, `id_rm` and `res2nor`. The use of this functionality is demonstrated in `demo_id`.

For a modally damped model (diagonal modal damping matrix Γ), the normal mode model (5.4) can be rewritten in a rational fraction form (with truncation and residual terms)

$$[\alpha(s)] = \sum_{j=1}^{NM} \frac{\{c\phi_j\} \{b^T \phi_j\}^T}{s^2 + 2\zeta_j \omega_j s + \omega_j^2} + [E] + \frac{[F]}{s^2} = \sum_{j=1}^{NM} \frac{[T_j]_{NS \times NA}}{s^2 + 2\zeta_j \omega_j s + \omega_j^2} + E(s) \quad (2.11)$$

This parameterization, called *normal mode residue form*, has a symmetric pole pattern and is supported by various functions (`id_rc`, `id_rm`, `res2xf`, ...) through the use of the option `ci.IDopt.Fit='Normal'`. As for the complex residues (5.26), the normal mode residue matrix given by `id_rc` and used by other functions is stacked using one row for each pole or asymptotic correction term and, as the FRFs (see the `xf` format), a column for each SISO transfer function (stacking NS columns for actuator 1, then NS columns for actuator 2, etc.)

Assuming that the constraint of proportional damping is valid, the identified residue matrix T_j is directly related to the true normal modes

$$[T_j] = \{c\phi_j\} \{\phi_j^T b\} \quad (2.12)$$

and the dyadic decomposition of the residue matrix can be used as in the complex mode case (see section 2.4.1 and the function `id_rm`) to obtain a minimal and/or reciprocal models (as well as scaled input and output shape matrices).

The scaling implied by equations (2.11) and (2.12) and used in the functions of the *Toolbox* is consistent with the assumption of unit mass normalization of the normal modes (see details under `nor` page 154). This remains true even for multiple modes. A result rarely obtained by other methods.

When a complex mode identification has been performed (`ci.IDopt.Fit='Complex'` or `'Posit'`), the function `res2nor` also provides a simple approximation of the complex residue model by a normal mode residue model.

Non proportional damping assumption

Theory

The complex modes of a minimal/reciprocal model are related to the mass / damping / stiffness matrices by (see Ref. [12])

$$M = (\tilde{\psi}\Lambda\tilde{\psi}^T)^{-1}, \quad C = -M\tilde{\psi}\Lambda^2\tilde{\psi}^T M, \quad \text{and} \quad K = (\tilde{\psi}\Lambda^{-1}\tilde{\psi}^T)^{-1} \quad (2.13)$$

if and only if the complex modes are also proper. That is, they verify

$$\sum_{j=1}^{2N} \{\tilde{\psi}_j\} \{\tilde{\psi}_j\}^T = [\tilde{\psi}]_{N \times 2N} [\tilde{\psi}]_{N \times 2N}^T = [0]_{N \times N} \quad (2.14)$$

The transformation `id_nor` is thus done in two stages. `id_rm` is used to find a minimal and reciprocal approximation of the identified residue model of the form (2.8). `psi2nor` then determines c and $\tilde{\psi}$ such that the $\tilde{\psi}$ verify the condition (2.14) and $c\tilde{\psi}$ is “optimally” close to the $c\psi$ resulting from `id_rm`. Using the complex modes $\tilde{\psi}$ and the identified poles λ , the matrices are then computed and the model transformed to the standard normal mode form with no further approximation.

The possibility to perform the transformation is based on the fact that the considered group of modes is not significantly coupled to other modes by damping [12]. Groups of modes which can be approximated by a second order non proportionally damped model can be easily detected using the frequency separation criterion which must be verified between modes j in the group and modes k outside the group

$$\frac{\zeta_j \omega_j \zeta_k \omega_k^2}{\omega_j \omega_k} \ll 1 \quad (2.15)$$

If there does not exist a normal mode model that has complex modes close to the identification result $c\psi$, the algorithm may not work. This will happen in particular if $c\psi\Lambda\psi^T c^T = cM^{-1}c^T$ does not have NQ positive eigenvalues (estimated mass not positive definite).

Practice

For comparisons with undamped FE models, it is essential to obtain estimates of normal modes. The most accurate results are obtained using a non-proportionally damped normal mode model obtained with `id_nor`. A coarse approximation is given by `res2nor` (useful if the identification is not good enough to build the minimal and reciprocal model used by `id_nor`). In such cases you can also consider using `id_rc` with the assumption of proportional damping which directly identifies normal modes (see more details in section 2.4.3).

Scaling problems are often encountered when using the reciprocity to condition to scale the complex modes in `id_rm`. The function `id_nor` allows an optimization of

collocated residues based on a comparison of the identified residues and those linked to the normal mode model. You should be aware that `id_nor` only works on very good identification results, so that trying it without spending the time to go through the pole update phase of `id_rc` makes little sense.

The use of this functionality is demonstrated in the following example.

```
ci=demosdt('demodemo_id') % load data and identify
f=ci.Stack{'Test'}.w;
nor = id_nor(ci.Stack{'IdMain'});
nor2xf(nor,f,'hz iiplot "IdFrfr"'); % Compute response

% compute residual effects and add normal model contributions
res2xf(ci.Stack{'IdMain'},f,ci.IDopt,[5 6],'iiplot "Nor+Stat"');% residue
ci.Stack{'Nor+Stat'}.xf=ci.Stack{'Nor+Stat'}.xf+nor2xf(nor,f,'hz');
iicom('ch1');
```

The normal mode input `nor.pb` and output `nor.cp` matrices correspond to those of an analytical model with mass normalized modes. They can be compared (`ii_mac`) or combined (`fe_exp`) with analytical models and the modal frequencies `nor.freq` and damping matrix `nor.ga` can be used for predictions (see more details in section 3.4).

The `id_nor` and `res2nor` algorithms only seek approximations the modes. For FRF predictions one will often have to add the residual terms. The figure below (taken from `demo_id`) shows an example where including residual terms tremendously improves the prediction. Out of band modes and residual terms are here represented by the $E(s)$ term. Second order models are said to be complete when $E(s)$ can be neglected [13]. The addition of residual terms was illustrated in the example above.

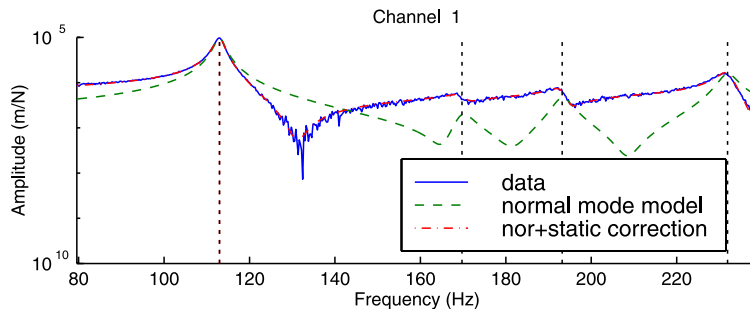


Figure 2.14: FRF xx

Test/analysis correlation tutorial

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Modal testing differs from system identification in the fact that responses are measured at a number of sensors which have a spatial distribution which allows the visualization of the measured motion. Visualization is key for a proper assessment of the quality of an experimental result. One typically considers three levels of models.

- Input/output models are defined at sensors. In the figure, one represents these sensors as arrows corresponding to the line of sight measurements of a laser vibrometer. Input/output models are the direct result of the identification procedure described in chapter 2.
- Wire frame models are used to visualize test results. They are an essential verification tool for the experimentalist. Designing a test well, includes making sure that the wire frame representation is sufficiently detailed to give the experimentalist a good understanding of the measured motion. With non-triaxial measurements, a significant difficulty is to handle the perception of motion assumed to be zero.
- Finite element models are used for test/analysis correlation. In most industrial applications, test and FEM nodes are not coincident so that special care must be taken when predicting FEM motion at test nodes/sensors (shape observation) or estimating test motion at FEM DOFs (shape expansion).

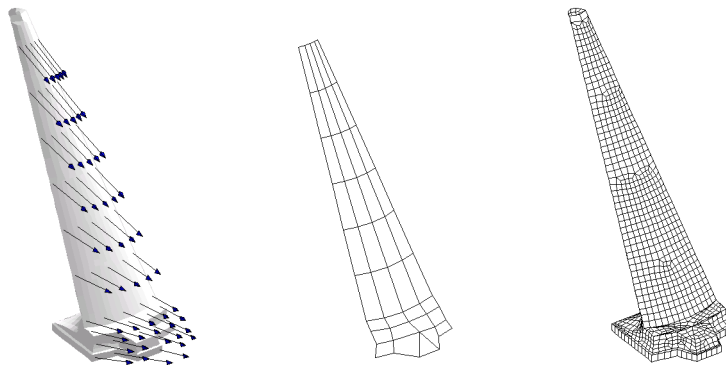


Figure 3.1: FE and wire-frame models

The tools for the declaration of the wire-frame model and of sensor setups are detailed in section 2.2. Topology correlation and sensor/shaker placement tools are details in section 3.1. A summary of general tools used to compare sets of shapes is made in section 3.2. Shape expansion, which deals with the transformations between the wire-frame and FE models, is introduced in section 3.3. The results of correlation

can be used for hybrid models combining experimental and analytical results (see section 3.4) or for finite element model updating (see section 6.5).

3.1 Topology correlation and test preparation

Topology correlation is the phase where one correlates test and model geometrical and sensor/shaker configurations. Most of this effort is handled by `fe_sens` with some use of `femesh`.

Starting with SDT 6.0, FEM sensors (see section 4.3) can be associated with wire frame model, the strategy where the two models were merged is thus obsolete.

As described in the following sections the three important phases of topology correlation are

- combining test and FEM model including coordinate system definition for the test nodes if there is a coordinate system mismatch,
- building of an observation matrix allowing the prediction of measurements based on FEM deformations,
- sensor and shaker placement.

3.1.1 Defining sensors in the FEM model

Prior steps are to declare

- a FEM model (see section 4.2). For this simple example, the `FEM` model must describe nodes, elements and DOFs.
- a test wire-frame model (stored in `TEST` in the demo) with sensors in the `.tdof` field, as detailed in section 2.2.1 for the geometry and section 2.2.2 for sensors

One then declares the wire frame (with sensors) as `SensDof` case entry as done below (see also the `gartte` demo). The objective of this declaration is to allow observation of the FEM response at sensors (see `sensor Sens`).

```
cf=demosdt('demo gartfeplot'); % load FEM
TEST=demosdt('demo garttewire'); % see sdtweb('pre#presen')
```

```

cf.mdl=fe_case(cf.mdl,'sensdof','outputs',TEST)

% View the Case entry in the properties figure
fecom(cf,'curtabCase','outputs');fecom('ProViewOn')
fecom('TextStack') % display sensor text
% now display FEM shape on sensors
fe_case(cf.mdl,'sensmatch')
cf.sel(2)='-outputs';
cf.o(1)={'sel 2 def 1 ch 7 ty2 scc .25','edgecolor','r'};

```

Section 4.3 gives many more details the sensor GUI, the available sensors ([sensor trans](#), [sensor triax](#), laser, ...). Section 4.3.4 discusses topology correlation variants in more details.

3.1.2 Test and FEM coordinate systems

In many practical applications, the coordinate systems for test and FEM differ. [fe_sens](#) supports the use of a local coordinate system for test nodes with the [basis](#) command. A three step process is considered. Phase 1 is used get the two meshes oriented and coarsely alligned. The guess is more precise if a list of paired nodes on the FEM and TEST meshes can be provided. In phase 2, the values displayed by [fe_sens](#), in phase 1 are fine tuned to obtain the accurate alignment. In phase 3, the local basis definition is eliminated thus giving a `cf.CStack{'sensors'}` entry with both `.Node` and `.tdof` fields in FEM coordinates which makes checks easier.

In peculiar cases, the FEM and TEST mesh axes differ, and a correction in rotation in the Phase 2 may be easier to use. An additional rotation to apply in the TEST mesh basis can be obtained by fullfilling the field [rotation](#) in Phase 2. The rotations are applied after other modifications so that the user can directly interpret the current [feplot](#) display. The rotation field corresponds to a `basis rotate` call. The command string corresponding to a rotation of 10 degrees along axis y is then `'ry=10;'`. Several rotations can be combined: `'ry=10; rx=-5;'` will thus first perform a rotation along y of 10 degrees and a rotation along x of -5 degrees. These combinations are left to the user's choice since rotation operations are not symmetric (e.g. `'rz=5;rx=10;'` is a different call from `'rx=10;rz=5;'`).

```

cf=demosdt('demo garttebasis'); % Load the demo data
cf.CStack{'sensors'} % contains a SensDof entry with sensors and wireframe

% Phase 1: initial adjustments done once
% if the sensors are well distributed over the whole structure

```

```

fe_sens('basis estimate',cf,'sensors');

% Phase 1: initial adjustments done once, when node pairs are given
% if a list of paired nodes on the TEST and FEM can be provided
% For help on 3DLinePick see sdtweb('3DLinePick')
cf.sel='reset';    % Use 3DLinePick to select FEM ref nodes
cf.sel='-sensors'; % Use 3DLinePick to select TEST ref
i1=[62 47 33 39;          % Reference FEM NodeId
    2112 2012 2301 2303]'; % Reference TEST NodeId
cf.sel='reset'; % show the FEM part you seek
fe_sens('basis estimate',cf,'sensors',i1);

%Phase 2 save the commands in an executable form
% The 'BasisEstimate' command displays these lines, you can
% perform slight adjustments to improve the estimate
fe_sens('basis',cf,'sensors', ...
    'x',    [0 1 0], ... % x_test in FEM coordinates
    'y',    [0 0 1], ... % y_test in FEM coordinates
    'origin',[-1 0 -0.005],... % test origin in FEM coordinates
    'scale', [0.01],... % test/FEM length unit change
    'rotation',''); % additional rotations

%Phase 3 : Force change of TEST.Node and TEST.tdof to FEM coordinates
fe_sens('basisToFEM',cf.mdl,'sensors')
fe_case(cf.mdl,'sensmatch')

```

Note that FEM that use local coordinates for displacement are discussed in [sensor trans](#).

3.1.3 Sensor/shaker placement

In cases where an analytical model of a structure is available before the modal test, it is good practice to use the model to design the sensor/shaker configuration.

Typical objectives for sensor placement are

- Wire frame representations resulting from the placement should allow a good visualization of test results without expansion. Achieving this objective, enhances the ability of people doing the test to diagnose problems with the test,

which is obviously very desirable.

- seen at sensors, it is desirable that modes *look* different. This is measured by the condition number of $[c\phi]^T [c\phi]$ (modeshape independence, see [14]) or by the magnitude of off-diagonal terms in the auto-MAC matrix (this measures orthogonality). Both independence and orthogonality are strongly related.
- sensitivity of measured modeshape to a particular physical parameter (parameter visibility)

Sensor placement capabilities are accessed using the `fe_sens` function as illustrated in the `gartsens` demo. This function supports the effective independence [14] and maximum sequence algorithms which seek to provide good placement in terms of modeshape independence.

It is always good practice to verify the orthogonality of FEM modes at sensors using the auto-MAC (whose off-diagonal terms should typically be below 0.1)

```
cphi = fe_c(mdof,sdof)*mode; ii_mac('cpa',cphi,'mac auto plot')
```

For shaker placement, you typically want to make sure that

- you excite a set of target modes,
- or will have a combination of simultaneous loads that excites a particular mode and not other nearby modes.

The placement based on the first objective is easily achieved looking at the minimum controllability, the second uses the Multivariate Mode Indicator function (see `ii_mmif`). Appropriate calls are illustrated in the `gartsens` demo.

3.2 Test/analysis correlation

Correlation criteria seek to analyze the similarity and differences between two sets of results. Usual applications are the correlation of test and analysis results and the comparison of various analysis results.

Ideally, correlation criteria should quantify the ability of two models to make the same predictions. Since, the predictions of interest for a particular model can rarely be pinpointed precisely, one has to use general qualities and select, from a list of possible criterion, the ones that can be computed and do a good enough job for the intended purpose.

3.2.1 Shape based criteria

The `ii_mac` interface implements a number of correlation criteria. You should at least learn about the Modal Assurance Criterion (MAC) and Pseudo Orthogonality Checks (POC). These are very popular and should be used first. Other criteria should be used to get more insight when you don't have the desired answer or to make sure that your answer is really foolproof.

Again, *there is no best choice for a correlation criterion* unless you are very specific as to what you are trying to do with your model. Since that rarely happens, you should know the possibilities and stick to what is good enough for the job.

The following table gives a list of criteria implemented in the `ii_mac` interface.

MAC	Modal Assurance Criterion (9.7). The most popular criterion for correlating vectors. Insensitive to vector scaling. Sensitive to sensor selection and level of response at each sensor. Main limitation : can give very misleading results without warning. Main advantage : can be used in all cases. A MAC criterion applied to frequency responses is called FRAC.
POC	Pseudo Orthogonality Checks (9.12). Required in some industries for model validation. This criterion is only defined for modes since other shapes do verify orthogonality conditions. Its scaled insensitive version (9.8) corresponds to a mass weighted MAC and is implemented as the MAC M commands. Main limitation : requires the definition of a mass associated with the known modeshape components. Main advantage : gives a much more reliable indication of correlation than the MAC.
Error	Modeshape pairing (based on the MAC or MAC-M) and relative frequency error and MAC correlation.
Rel	Relative error (9.13). Insensitive to scale when using the modal scale factor. Extremely accurate criterion but does not tell much when correlation poor.
COMAC	Coordinate Modal Assurance Criteria (three variants implemented in <code>ii_mac</code>) compare sets of vectors to analyze which sensors lead poor correlation. Main limitation : does not systematically give good indications. Main advantage : a very fast tool giving more insight into the reasons of poor correlation.
MACCO	What if analysis, where coordinates are sequentially eliminated from the MAC. Slower but more precise than COMAC.

3.2.2 Energy based criteria

The criteria that make the most mechanical sense are derived from the equilibrium equations. For example, modes are defined by the eigenvalue problem (6.84). Thus the dynamic residual

$$\{\hat{R}_j\} = [K - \omega_{j\text{id}}^2 M] \{\phi_{\text{id}j}\} \quad (3.1)$$

should be close to zero. A similar residual (3.5) can be defined for FRFs.

The Euclidean norm of the dynamic residual has often been considered, but it tends to be a rather poor choice for models mixing translations and rotations or having very different levels of response in different parts of the structure.

To go to an energy based norm, the easiest is to build a displacement residual

$$\{R_j\} = [\hat{K}]^{-1} [K - \omega_{j\text{id}}^2 M] \{\phi_{\text{id}j}\} \quad (3.2)$$

and to use the strain $|\tilde{R}_j|_K = \tilde{R}_j^T K \tilde{R}_j$ or kinetic $|\tilde{R}_j|_M = \tilde{R}_j^T M \tilde{R}_j$ energy norms for comparison.

Note that $[\hat{K}]$ need only be a reference stiffness that appropriately captures the system behavior. Thus for cases with rigid body modes, a pseudo-inverse of the stiffness (see section 6.2.4), or a mass shifted stiffness can be used. The displacement residual \tilde{R}_j is sometimes called error in constitutive law (for reasons that have nothing to do with structural dynamics).

This approach is illustrated in the [gartco](#) demo and used for MDRE in [fe_exp](#). While much more powerful than methods implemented in [ii_mac](#), the development of standard energy based criteria is still a fairly open research topic.

3.2.3 Correlation of FRFs

Comparisons of frequency response functions are performed for both identification and finite element updating purposes.

The quadratic cost function associated with the Euclidean norm

$$J_{ij}(\Omega) = \sum_{ij \text{ measured}, k \in \Omega} |\hat{H}_{ij}(s_k) - H_{ij}(s_k)|^2 \quad (3.3)$$

is the most common comparison criterion. The main reason to use it is that it leads to linear least-squares problem for which there are numerically efficient solvers. (`id_rc` uses this cost function for this reason).

The quadratic cost corresponds to an additive description of the error on the transfer functions and, in the absence of weighting, it is mostly sensitive to errors in regions with high levels of response.

The log least-squares cost, defined by

$$J_{ij}(\Omega) = \sum_{ij \text{ measured}, k \in \Omega} \left| \log \left| \frac{\hat{H}_{ij}(s_k)}{H_{ij}(s_k)} \right| \right|^2 \quad (3.4)$$

uses a multiplicative description of the error and is as sensitive to resonances than to anti-resonances. While the use of a non-linear cost function results in much higher computational costs, this cost tends to be much better at distinguishing physically close dynamic systems than the quadratic cost (except when the difference is very small which is why the quadratic cost can be used in identification phases).

The utility function `ii_cost` computes these two costs for two sets of FRFs `xf1` and `xf2` (obtained through test and FE prediction using `nor2xf` for example). The evaluation of these costs provides a quick and efficient way to compare sets of MIMO FRF and is used in identification and model update algorithms.

Note that you might also consider the complex log of the transfer functions which would give a simple mechanism to take phase errors into account (this might become important for extremely accurate identification sometimes needed for control synthesis).

If the response at a given frequency can be expanded to the full finite element DOF set, you should consider an energy criterion based on the dynamic residual in displacement, which in this case takes the form

$$\{R_j\} = [\hat{K}]^{-1} [[Z(\omega)] \{q_{ex}(\omega)\} - [b] \{u(\omega)\}] \quad (3.5)$$

and can be used directly of test/analysis correlation and/or finite element updating.

Shape correlation tools provided by `ii_mac` can also be used to compare frequency responses. Thus the MAC applied to FRFs is sometimes called FRAC.

3.3 Expansion methods

Expansion methods seek to estimate the motion at all DOFs of a finite element model based on measured information (typically modeshapes or frequency response functions) and prior, but not necessarily accurate, information about the structure under test in the form of a reference finite element model. As for all estimation techniques, the quality of expansion results is deteriorated by poor test results and/or poor modeling, but good results can be obtained when one or both are accurate.

The [gartco](#) demonstration illustrates modeshape expansion in the *SDT*. This section summarizes the theory and you are encouraged to download [15][16] from [sdtools.com](#) if you want more details.

3.3.1 Underlying theory for expansion methods

The unified perspective driving the *SDT* architecture is detailed in [15][16]. The proposed classification is based on how various methods combine information about **test** and **modeling** errors.

Test results y_{Test} and expanded shapes q_{ex} are related by the observation equation (4.1). Test error is thus measured by a norm of the difference between the test quantity and the observed expanded shape

$$\epsilon = \|\{y_{Test}\} - [c]\{q_{ex}\}\|_Q^2 \quad (3.6)$$

where the choice of the Q norm is an important issue. While the Euclidian norm ($Q = I$) is used in general, a norm that takes into account an estimated variance of the various components of y_{Test} seems most appropriate. Various energy based metrics have also been considered in [17] although the motivation for using a energy norm on test results is unclear.

The expanded vector is also supposed to verify an equilibrium condition that depends on its nature. Since the model and test results don't match exactly one does not expect the expanded vector to verify this equation exactly which leads to the definition of a residual. Standard residuals are $R_j = Z(\omega_j)\phi_j$ for modeshapes and $R_j = Z(\omega)q - F$ for frequency response to the harmonic load F .

Dynamic residuals correspond to generalized loads, so they should be associated to displacement residuals and an energy norm. A standard solution [18] is to compute the static response to the residual and use the associated strain energy, which is a good indicator of modeling error,

$$\|R_j(q_{ex})\|_K^2 = \{R_j\}^T [\hat{K}]^{-1} \{R_j\} \quad (3.7)$$

where \hat{K} is the stiffness of a reference FEM model and can be a mass-shifted stiffness in the presence of rigid body modes (see section 6.2.4). Variants of this energy norm of the dynamic residual can be found in [17].

like all estimation techniques, expansion methods should clearly indicate a trade-off between test and modeling errors, since both test and model are subject to error. But modeling errors are not easily taken into account. Common expansion techniques thus only use the model to build a subspace of likely displacements.

Interpolation methods, the simplest form of subspace method are discussed in section 3.3.2. Standard subspace methods and their implementation are discussed in section section 3.3.3. Methods taking modeling errors into account are discussed in section 3.3.4.

3.3.2 Basic interpolation methods for unmeasured DOFs

Translations are always measured in a single direction. By summing the measurements of all sensors at a single physical node, it is possible for triaxial measurements to determine the 3-D motion. Using only triaxial measurements is often economically/technically impossible and is not particularly desirable. Assuming that all unmeasured motions are zero is however often not acceptable either (often distorts the perception of test modeshapes in 3-D wire frame displays).

Historically, the first solutions to this problem used geometrical interpolation methods estimating the motion in less important directions based on measurements at a few selected nodes.

Wire-frame displays can be considered as trivial interpolation methods since the motion between two test nodes is interpolated using linear shape functions.

In the *SDT*, you can easily implement interpolation methods using matrices which give the relation between measured DOFs `tdof` and a larger set of deformation DOFs `ndof`. The easiest approach is typically a use of the `fe_sens WireExp` command as in the example below

```
% generate example, see sdtweb('demosdt.m#Sleeper')
cf=demosdt('sleeper');
TR=fe_sens('wireexp',cf.CStack{'Test'})
fe_sens('WireExpShow',cf,TR)
```

```

% display partial shapes as cell array
disp(TR)
r1=[{''} fe_c(TR.adof([1 3 5]))'];
fe_def('subdof-cell',fe_def('subdef',TR,[1 3 5]),[1 2 46 48]')

```

Given an interpolation matrix `TR`, you can animate interpolated shapes using `cf.def={def,exp}`. The interpolation (expansion) matrix `TR` has fields

- `TR.DOF` lists DOFs where the response is interpolated
- `TR.adof` lists input DOFs, these should match identifiers in the first column of a `sens.tdof` field.
- `TR.def` give the displacement at all DOFs corresponding to a unit sensor motion. Note as shown in the example above that a 1.08 ($1 - y$) measurement should lead to a negative value on the 1.02 ($1y$) DOF. The same holds for measurements in arbitrary directions, `TR.def` should be unity when projected in the measurement direction.

The `fe_sens WireExp` command considers the wire frame as a coarse FEM model and uses expansion (see section 3.3.3 for details) to generate the interpolation. This is much more general than typical geometric constructions (linear interpolations, spline), which cannot handle arbitrary geometries.

Manual building of the interpolation matrix can be done by filling in the `TR.def` columns. `fe_sens('WireExpShow',cf,TR)` can then be used to verify the interpolation associated with each sensor (use the +/- buttons to scan trough sensors).

Starting from a basis of vectors `exp.def` with non unit displacements at the measurement DOFs, you can use

```

TR=exp;TR.adof=tdof(:,1);
TR.def=exp.def*pinv(fe_c(exp.DOF,tdof)*exp.def);

```

to minimize the norm of the test error (3.6) for a response within the subspace spanned by `exp.def` and thus generate a unmeasured DOF interpolation matrix.

3.3.3 Subspace based expansion methods

If one can justify that true motion can be well represented by a vector within the subspace characterized by a basis T with no more columns than there are sensors (one assumes that the true displacement is of the form $\{q_{Ex}\} = [T] \{q_R\}$), an estimate

of the true response simply obtained by minimizing test error, that is solving the least-squares problem

$$\{q_R\} = \arg \min || \{y_{Test}\} - [c] [T] \{q_R\} ||_2^2 \quad (3.8)$$

Modeshape expansion based on the subspace of low frequency modes is known as **modal** [19] or **SEREP** [20] expansion. The subtle difference between the two approaches is the fact that, in the original paper, modal expansion preserved test results on test DOFs (DOFs and sensors were assumed to coincide) and interpolated motion on other DOFs. The *SDT* supports modal expansion using

```
yExp = fe_exp(yTest,sens,T)
```

where **yTest** are the measured vectors, **sens** is the sensor configuration (see **fe_sens**) or an observation matrix **c**, and **T** is a set of target modes (computed using **fe_eig** or imported from an other FE code).

An advantage of the modal methods is the fact that you can select less target modes that you have sensors which induces a smoothing of the results which can alleviate some of the problems linked to measurement/identification errors.

The study presented in [15] concludes that modal based methods perform very well when an appropriate set of target modes is selected. The only but essential limitation seems to be the absence of design/verification methodologies for target mode selection. Furthermore it is unclear whether a good selection always exists.

Modeshape expansion based on the subspace of static responses to unit displacements at sensors is known as **static** expansion or Guyan reduction [21].

When expanding modeshapes or FRFs, each deformation is associated to a frequency. It thus seems reasonable to replace the static responses by dynamic responses to loads/displacements at that frequency. This leads to dynamic expansion [22]. In general, computing a subspace for each modeshape frequency is too costly. The alternative of using a single “representative” frequency for all modes was proposed in [23] but suffers from the same limitations as choosing this frequency to be zero (Guyan reduction).

The *SDT* supports full order static and dynamic expansion using

```
yExp=fe_exp(yTest,fTest,sens,m,k,mdof)
```

where **fTest** can a single frequency (0 for static) or have a value for each shape. In the later case, computational times are usually prohibitive so that reduced basis solutions discussed below should be used.

For tests described by observation matrices, the unit displacement problem defining static modes can be replaced by a unit load problem $[T] = [K]^{-1} [c]^T$. For structures without rigid body modes this generates the same subspace as the unit displacement problem. In other cases $[K]$ is singular and can be simply mass-shifted (replaced by $K + \alpha M$ with α usually taken small when compared to the square of the first flexible frequency, see section 6.2.4).

In practice, static expansion can be restated in the form (3.8) where T corresponds to constraint or modes associated to the load collocated to the output shape matrix characterizing sensors (see chapter 6). Restating the problem in terms of minimization is helpful if you want to compute your static responses outside the *SDT* (you won't need to import your mass and stiffness matrices but only the considered static responses).

The weakness of static expansion is the existence of a frequency limit found by computing modes of the structure with all sensors fixed. In many practical applications, this frequency limit is not that low (typically because of lack of sensors in certain areas/directions). You can easily compute this frequency limit using `fe_exp`.

Full order dynamic expansion is typically too expensive to be considered for a full order model. The *SDT* supports reduced basis dynamic expansion where you compute dynamic expansion on a subspace combining modes and static responses to loads at sensors. A typical calling sequence combining modeshape computations and static correction would be

```
[md0,f0,kd] = fe_eig(m,k,[105 30 1e2]);
T = [kd \ ((sens.ctn*sens.cna)') md0];
mdex = fe_exp(Iires.',Iipo(:,1)*2*pi,sens,m,k,mdof,T);
```

You should note however that the minimum dynamic residual expansion (MDRE) discussed in the next section typically gives better results at a marginal computational cost increase, so that you should only use dynamic expansion to expand FRFs (MDRE for FRFs is not currently implemented in `fe_exp`) or operational deflection shapes (for which modeling error is hard to define).

3.3.4 Model based expansion methods

Given metrics on test (3.6) and modeling (3.7) error, one uses a weighted sum of the two types of errors to introduce a generalized least-squares problem

$$\min_{q_{j,ex}} \|R(q_{j,ex})\|_K^2 + \gamma_j \epsilon_j \quad (3.9)$$

MDRE (Minimum Dynamic Residual Expansion) assumes test errors to be zero. MDRE-WE (MDRE With test Error) sets the relative weighting (γ_j coefficient) iteratively until the desired bound on test error is reached (this is really a way to solve the least-squares problem with a quadratic inequality as proposed in [24]).

These methods are currently only implemented for modeshape expansion. When they can be used, they are really superior to subspace methods. The proper strategy to choose the error bound in MDRE-WE is still an open issue but it directly relates to the confidence you have in your model and test results.

3.4 Structural dynamic modification

While test results are typically used for test/analysis correlation and update, experimental data have direct uses. In particular,

- experimental damping ratios are often used for finite element model predictions;
- identified models can be used to predict the response after a modification (if this modification is mechanical, one talks about *structural modification*, if it is a controller one does *closed loop response* prediction);
- identified models can be used to generate control laws in active control applications;
- if some input locations of interest for structural modification have only been tested as output locations, the reciprocity assumption (see section 2.4.2) can be used to predict unmeasured transfers. But these predictions lack residual terms (see section 6.2.3) which are often important in coupled predictions.

Structural modification and closed loop predictions are important application areas of *SDT*. For closed loop predictions, users typically build state-space models with `res2ss` and then use control related tools (*Control Toolbox*, *SIMULINK*). If mechanical modifications can be modeled with a mass/damping/stiffness model directly connected to measured inputs/outputs, predicting the effect of a modification takes the same route as illustrated below. Mass effects correspond to acceleration feedback, damping to velocity feedback, and stiffness to displacement feedback.

The following illustrates on a real experimental dataset the prediction of a 300 g mass loading effect at a locations 1012 – z and 1112 – z (when only 1012 – z is excited in the `gartid` dataset used below).

3 Test/analysis correlation tutorial

```
ci=demosdt('demo gartid est');
ci.Stack{'Test'}.xf=-ci.Stack{'Test'}.xf;% driving 1012-z to 1012z
ci.Stack{'Test'}.dof(:,2)=12.03;
ci.IDopt.recip='1 FRF'; idcom(ci,'est');

ind=fe_c(ci.Stack{'IdMain'}.dof(:,1),[1012;1112],'ind');
po_ol=ci.Stack{'IdMain'}.po;

% Using normal modes
NOR = res2nor(ci.Stack{'IdMain'}); NOR.pb=NOR.cp';
S=nor2ss(NOR,'hz'); % since NOR.idopt tells acc. SS is force to Acc
mass=.3; a_cl = S.a - S.b(:,ind)*S.c(ind,:)*mass;
po_cln=ii_pof(eig(a_cl)/2/pi,3,2)
SS=S;SS.b=SS.b(:,4);SS.d=SS.d(:,4);SS.dof_out=SS.dof_out(4,:);
qbode(SS,ci.Stack{'Test'}.w*2*pi,'iipplot "Normal"');

% Using complex modes
S = res2ss(ci.Stack{'IdMain'},'AllIO');
a_cl = S.a - S.b(:,ind)*S.c(ind,:)*mass;
po_clx=ii_pof(eig(a_cl)/2/pi,3,2)
SS=S;SS.b=SS.b(:,4);SS.d=SS.d(:,4)*0; % non-zero d terms = problem
qbode(SS,ci.Stack{'Test'}.w*2*pi,'iipplot "Cpx"');
iicom('ch4');

% Frequencies
figure(1);in1=1:8;subplot(211);
bar([ po_clx(in1,1) po_cln(in1,1)]./po_ol(in1,[1 1]))
ylabel('\Delta F / F');legend('Complex modes','Normal modes')
set(gca,'ylim',[.5 1])

% Damping
subplot(212);bar([ po_clx(in1,2) po_cln(in1,2)]./po_ol(in1,[2 2]))
ylabel('\Delta \zeta / \zeta');legend('Complex modes','Normal modes')
set(gca,'ylim',[.5 1.5])
```

Notice that the change in the sign of `ci.Stack{'Test'}.xf` needed to have a positive driving point FRFs (this is assumed by `id_rm`). Reciprocity was either applied using complex (the `'AllIO'` command in `res2ss` returns all input/output pairs assuming reciprocity) or normal modes with `NOR.pb=NOR.cp'`.

Closed loop frequency predictions agree very well using complex or normal modes (as well as with FEM predictions) but damping variation estimates are not very good with the complex mode state-space model.

There is much more to *structural dynamic modification* than a generalization of this example to arbitrary point mass, stiffness and damping connections. And you can read [25] or get in touch with SDTools for our latest advances on the subject.

FEM tutorial

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This chapter introduces notions needed to use finite element modeling in the *SDT*. It illustrates how to define mechanical problems (model, boundary conditions, loads, etc.), compute and post-process the response

- using the `feplot` Graphical User Interface,
- or using script commands.

The GUIs are described and the connections between graphical and low level data are detailed for

- the model data structures,
- the case (i.e. DOFs, boundary conditions, loads, ...),
- the response to a specified case,
- the results post-processing .

4.1 FE mesh and the feplot interface

Three kinds of manipulations are possible using the `feplot` GUI

- viewing the model and post-processing the responses,
- setting and displaying the mechanical problem (model properties and cases),
- setting the view properties.

4.1.1 The main feplot figure

`feplot` figures are used to view FE models and hold all the data needed to run simulations. Data in the model can be viewed in the property figure (see section 4.1.3). Data in the figure can be accessed from the command line through pointers as detailed in section 4.1.2. The `feplot` help gives architecture information, while `fecom` lists available commands. Most demonstrations linked to finite element modeling (see section 1.1 for a list) give examples of how to use `feplot` and `fecom`.

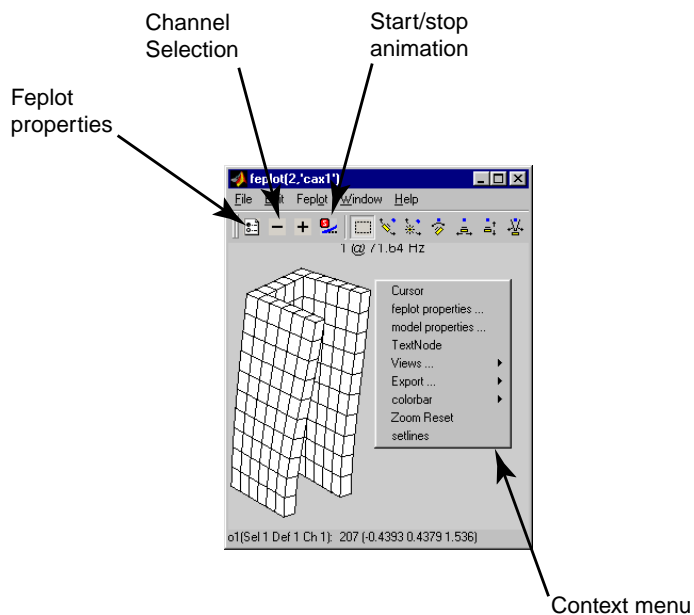


Figure 4.1: Main feplot figure.

The first step of most analyzes is to display a model in the main `feplot` figure. Examples of possible commands are (see `fecom load` for more details)

- `cf=feplot(model)` display the model in a variable and returns a pointer object `cf` to the figure.
- `cf=feplot(5);cf.model=model;` do the same thing but in figure 5. `cf=feplot;cf.model={n}` will work for just nodes and elements. Note that `cf.model` is a method to define the model and is not a pointer. `cf.mdl` is a pointer to the model, see section 4.1.2.
- `feplot('load','File.mat')` load a model from a `.mat` file.








As an example, you can load the data from the `gartfe` demo, get `cf` a *SDT handle* for a `feplot` figure, set the model for this figure and get the standard 3D view of the structure

```
model=demosdt('demogartfe')
cf=feplot;           % open FEPLLOT and define a pointer CF to the figure
cf.model=model;
```

The main capabilities the `feplot` figure are accessible using the figure toolbar, the keyboard shortcuts, the right mouse button (to open context menus) and the menus.

Toolbar

List of icons used in GUIs

	Model properties used to edit the properties of your model.
	Start/stop animation
	Previous Channel/Deformation
	Next Channel/Deformation
	iimouse zoom
	Orbit. Remaining icons are part of MATLAB cameratoolbar functionality.
	Snapshot. See iicom ImWrite .

Keyboard shortcuts

At this level note how you can zoom by selecting a region of interest with your mouse (double click or press the `i` key to zoom back). You can make the axis active by clicking on it and then use any of the `u`, `U`, `v`, `V`, `w`, `W`, `3`, `2` keys to rotate the plot (press the `?` key for a list of [iimousekey](#) shortcuts).

Menus and context menu

The [contextmenu](#) associated with your plot may be opened using the right mouse button and select [Cursor](#). See how the cursor allows you to know node numbers and positions. Use the left mouse button to get more info on the current node (when you have more than one object, the `n` key is used to go to the next object). Use the right button to exit the [cursor](#) mode.

Notice the other things you can do with the [ContextMenu](#) (associated with the figure, the axes and objects). A few important functionalities and the associated commands are

- [Cursor Node](#) tracks mouse movements and displays information about pointed object. This is equivalent to the `iimouse('cursor')` command line.
- `Cursor...[Elt, Sel, Off]` selects what information to display when tracking the mouse. The `iimouse('cursor[onElt, onSel, Off]')` command lines are possible.
- `Cursor... 3DLinePick` (which can be started with `fe_fmesh('3DLineInit')`) allows node picking. Once started, the context menu gives access `info` (lists

picked nodes and distances) and `done` prints the list of picked nodes.

- `TextNode` activates the node labeling. It is equivalent to the `fecom('TextNode')` command line.
- `Triax` displays the orientation triax. It is equivalent to the `fecom('triax')` command line.
- `Undef` shows the undeformed structure. Other options are accessible with the `fecom('undef[dot,line]')` command line.
- `Views`... [View n+x,...] selects default plot orientation. The `iimouse('vn+x,...')` command lines are available.
- `colorbar on` shows the colorbar, for more accurate control see `fecom ColorBar`.
- `Zoom Reset` is the same as the `iimouse('resetvie')` command line to reset the zoom.
- `setlines` is the same as the `setlines` command line.

The figure `Feplot` menu gives you access to the following commands (accessible by `fecom`)

- `Feplot:Feplot/Model properties` opens the property figure (see section 4.1.3).
- `Feplot:Sub commands:Sub IsoViews` (same as `iicom('subiso')`) gets a plot with four views of the same mode. Use `iicom('sub2 2 step')` to get four views of different modes.
- `Feplot:Show` menu generates standard plots. For FE analyses one will generally use surface plots color-coded surface plots using `patch` objects) or wire-frame plots (use `Feplot:Show` menu to switch).
- `Feplot:Misc` shows a `Triax` or opens the channel selector.
- `Feplot:Undef` is used to show or not the undeformed structure.
- `Feplot:Colordata` shows structure with standard colors.
- `Feplot:Selection` shows available selections.
- `Feplot:Renderer` is used to choose the graphical rendering. Continuous animation in OpenGL rendering is possible for models that are not too large. The `fecom SelReduce` can be use to coarsen the mesh otherwise.

- `Feplot:Anim` chooses the animation mode.
- `Feplot:View defaults` changes the orientation view.

4.1.2 Pointers to the figure and the model

`cf1=feplot` returns a pointer to the current `feplot` figure. The handle is used to provide simplified calling formats for data initialization and text information on the current configuration. You can create more than one `feplot` figure with `cf=feplot(FigHandle)`. If many `feplot` figures are open, one can define the target giving an `feplot` figure handle `cf` as a first argument to `fecom` commands.

The model is stored in a graphical object. `cf.model` is a method that calls `fecom InitModel`. `cf1.mdl` is a method that returns a pointer to the model. Modifications to the pointer are reflected to the data stored in the figure. However `mo1=cf.mdl;mo1=model` makes a copy of the variable `model` into a new variable `mo1`.

`cf.Stack` gives access to the model stack as would `cf.mdl.Stack` but allows text based access. Thus `cf.Stack{'EigOpt'}` searches for a name with that entry and returns an empty matrix if it does not exist. If the entry may not exist a type must be given, for example `cf.Stack{'info','EigOpt'}=[5 10 1]`.

`cf.CStack` gives access to the case stack as would calls of the form


`Case=fe_case(cf.mdl,'getcase');stack_get(Case,'FixDof','base')` but it allows more convenient string based selection of the entries.

`cf.Stack` and `cf.CStack` allow regular expressions text based access. First character of such a text is then `#`. One can for example access to all of the stack entries beginning by the string `test` with `cf.Stack{'#test.*'}`. Regular expressions used by `SDT` are standard regular expressions of `Matlab`. For example `.` replaces any character, `*` indicates 0 to any number repetitions of previous character...

4.1.3 The property figure

Finite element models are described by a data structures with the following main fields (for a full list of possible fields see section 7.6)

<code>.Node</code>	nodes
<code>.Elt</code>	elements
<code>.pl</code>	material properties
<code>.il</code>	element properties
<code>.Stack</code>	stack of entries containing additional information cases (boundary conditions, loads, etc.), material names, etc.

The model content can be viewed using the `feplot` property figure. This figure is opened using the  icon, or `fecom('ProInit')`.

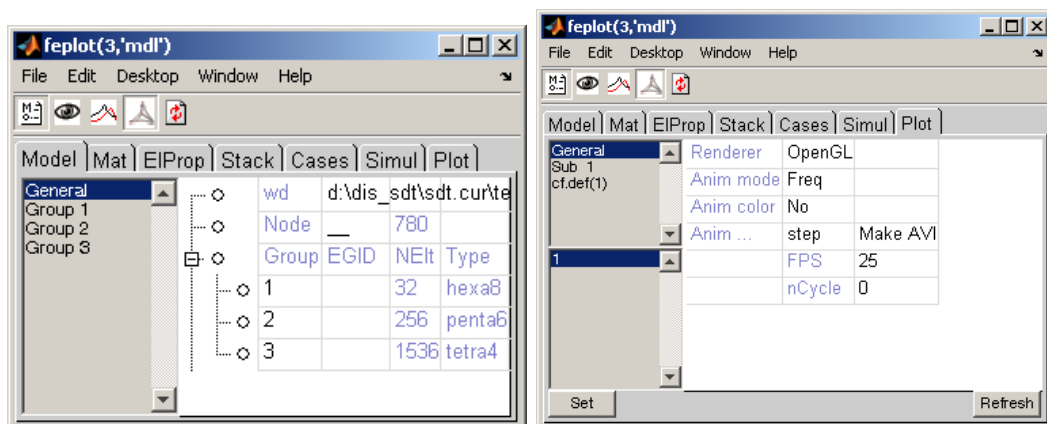








Figure 4.2: Model property interface.

This figure has the following tabs

- **Model** tab gives general information on the model nodes and elements. You can declare those by hand as shown in section 4.1.4, through structured mesh manipulations with `feutil` see section 4.1.5, or through import see section 4.1.8. (see section 4.2 and Figure 4.2). You can visualize one or more groups by selecting them in the left group list of this tab.
- **Mat** tab lists and edits all the material. In the  mode, associated elements in selection are shown. See section 4.2.1.
- **ElProp** tab lists and edits all the properties. See section 4.2.1.
- **Stack** tab lists and edits general information stored in the model (see section 7.7 for possible entries). You can access the model stack with the `cf.Stack` method.

- **Cases** tab lists and edits load and boundary conditions (see section 4.2.3 and Figure 4.8). You can access the case stack with the `cf.CStack` method.
- **Simulate** tab allows to launch the static and dynamic simulation (see section 4.5 and Figure 4.11).

The figure icons have the following uses

	Model properties used to edit the properties of your model.
	Active display of current group, material, element property, stack or case entry. Activate with <code>fecom('ProViewOn');</code>
	Open the iiplot GUI.
	Open/close feplot figure
	Refresh the display, when the model has been modified from script.

4.1.4 Direct declaration of geometry (truss example)

Hand declaration of a model can only be done for small models and later sections address more realistic problems. This example mostly illustrates the form of the model data structure.

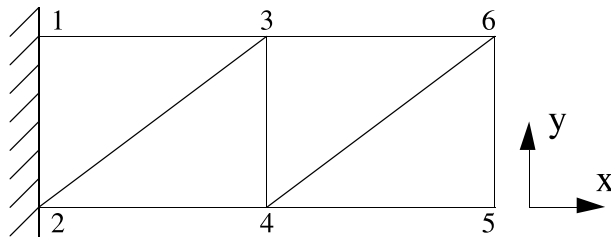


Figure 4.3: FE model.

The geometry is declared in the `model.Node` matrix (see section 7.1 and section 7.1.2). In this case, one defines 6 nodes for the truss and an arbitrary reference node to distinguish principal bending axes (see `beam1`)

```
%      NodeID  unused   x  y  z
model.Node=[ 1      0 0 0   0 1 0; ...
             2      0 0 0   0 0 0; ...
             3      0 0 0   1 1 0; ...
             4      0 0 0   1 0 0; ...
```

```

5      0 0 0   2 0 0; ...
6      0 0 0   2 1 0; ...
7      0 0 0   1 1 1]; % reference node

```

The model description matrix (see section 7.1) describes 4 longerons, 2 diagonals and 2 battens. These can be declared using three groups of `beam1` elements

```

model.Elt=[ ...
% declaration of element group for longerons
Inf      abs('beam1') ; ...
%node1  node2   MatID ProID  nodeR, zeros to fill the matrix
  1      3      1    1     7      0 ; ...
  3      6      1    1     7      0 ; ...
  2      4      1    1     7      0 ; ...
  4      5      1    1     7      0 ; ...
% declaration of element group for diagonals
Inf      abs('beam1') ; ...
  2      3      1    2     7      0 ; ...
  4      6      1    2     7      0 ; ...
% declaration of element group for battens
Inf      abs('beam1') ; ...
  3      4      1    3     7      0 ; ...
  5      6      1    3     7      0 ];

```

You may view the declared geometry

```

cf=fepplot; cf.model=model;          % create feplot axes
fecom(';view2;textnode;triax;'); % manipulate axes

```

The `demo.fe` script illustrates uses of this model.

4.1.5 Building models with femesh

Declaration by hand is clearly not the best way to proceed in general. `femesh` provides a number of commands for finite element model creation. The first input argument should be a string containing a single `femesh` command or a string of chained commands starting by a `;` (parsed by `commode` which also provides a `femesh` command mode).

To understand the examples, you should remember that `femesh` uses the following *standard global variables*

<code>FEnode</code>	main set of nodes
<code>FEn0</code>	selected set of nodes
<code>FEn1</code>	alternate set of nodes
<code>FEelt</code>	main finite element model description matrix
<code>FEe10</code>	selected finite element model description matrix
<code>FEe11</code>	alternate finite element model description matrix

In the example of the previous section (see also the [d_truss](#) demo), you could use `femesh` as follows: initialize, declare the 4 nodes of a single bay by hand, declare the beams of this bay using the `objectbeamline` command

```
FEe10=[]; FEelt=[];
FEnode=[1 0 0 0 0 0 0;2 0 0 0 0 0 1 0; ...
        3 0 0 0 1 0 0;4 0 0 0 1 1 0]; ...
femesh('objectbeamline 1 3 0 2 4 0 3 4 0 1 4');
```

The model of the first bay in is now *selected* (stored in `FEe10`). You can now put it in the main model, translate the selection by 1 in the x direction and add the new selection to the main model

```
femesh(';addsel;transsel 1 0 0;addsel;info');
model=femesh('model'); % export FEnode and FEelt geometry in model
cf=fepplot; cf.model=model;
fecom(';view2;textnode;triax;');
```

You could also build more complex examples. For example, one could remove the second bay, make the diagonals a second group of `bar1` elements, repeat the cell 10 times, rotate the planar truss thus obtained twice to create a 3-D triangular section truss and show the result (see [d_truss](#))

```
femesh('reset');
femesh('test2bay');
femesh('removeelt group2');
femesh('divide group 1 InNode 1 4');
femesh('set group1 name bar1');
femesh(';selgroup2 1;repeatsel 10 1 0 0;addsel');
femesh(';rotatesel 1 60 1 0 0;addsel;');
femesh(';selgroup3:4;rotatesel 2 -60 1 0 0;addsel;');
femesh(';selgroup3:8');
model=femesh('model0'); % export FEnode and FEe10 in model
cf=fepplot; cf.model=model;
fecom(';triaxon;view3;view y+180;view s-10');
```

`femesh` allows many other manipulations (translation, rotation, symmetry, extrusion, generation by revolution, refinement by division of elements, selection of groups, nodes, elements, edges, etc.) which are detailed in the *Reference* section.

Other more complex examples are treated in the following demonstration scripts `d_plate`, `beambar`, `d_ubeam`, `gartfe`.

4.1.6 GUI based mesh editing

This section describes functionality accessible with the `Edit` list item in the `Model` tab. To force display use `fecom('CurtabModel','Edit')`.

- `AddNode` opens a dialog that lets you enter nodes by giving their coordinates `x y z`, their node number and coordinates `NodeId x y z` or all the node information `NodeId CID DID GID x y z`.
- `AddNodeCG` starts the 3D line picker. You can then select a group of nodes by clicking with the left button on these nodes. When you select `Done` with the context menu (right click), a new node is added at the CG of the selected nodes.
- `AddNodeOnEdge` starts the 3D line picker to pick two nodes and adds nodes at the middle point of the segment.
- `AddElt Name` starts the 3D line picker and lets you select nodes to mesh individual elements. With `Done` the elements are added to the model as a group.
- `AddRbe3` starts a line picker to define an RBE3 constraint. The first node picked is slave to the motion of other nodes.
- `RemoveWithNode` starts the 3D line picker. You can then select a group of nodes by clicking with the left button on these nodes. When you select `Done` with the context menu (right click), elements containing the selected nodes are removed.
- `RemoveGroup` opens a dialog to remove some groups.

Below are sample commands to run the functionality from the command line.

```
model=demosdt('demoubeam');cf=fepplot;
fecom('CurtabModel','Edit')
```

```

fecom(cf,'addnode')
fecom(cf,'addnodecg')
fecom(cf,'addnodeOnEdge')
fecom(cf,'RemoveWithNode')
fecom(cf,'RemoveGroup')
fecom(cf,'addElt tria3')

fe_case(cf.mdl,'rbe3','RBE3',[1 97 123456 1 123 98 1 123 99]);
fe_case(cf.mdl,'rbe3 -append','RBE3',[1 100 123456 1 123 101 1 123 102]);
fecom addRbe3

```

4.1.7 Automated meshing capabilities

While this is not the toolbox focus, SDT supports some free meshing capabilities.

`fe_gmsh` is an interface to the open source 3D mesher GMSH. Calls to this external program can be used to generate meshes by direct calls from MATLAB. Examples are given in the function reference.

`fe_tetgen` is an interface to the open source 3D tetrahedral mesh generator. See `help fe_tetgen` for commands.

`fe_fmsh` contains a 2D quad mesher which meshes a coarse mesh containing triangles or quads into quads of a target size. All nodes existing in the rough mesh are preserved.

```

% build rough mesh
model=feutil('Objectquad 1 1',[0 0 0;2 0 0; 2 3 0; 0 3 0],1,1);
model=feutil('Objectquad 1 1',model,[2 0 0;8 0 0; 8 1 0; 2 1 0],1,1);
% start the mesher with a reference distance of .1
model=fe_fmsh('qmesh .1',model.Node,model.Elt);
feplot(model);

```

Other resources in the MATLAB environment are `initmesh` from the PDE toolbox and the `Mesh2D` package.

4.1.8 Importing models from other codes

The base *SDT* supports reading/writing of test related Universal files. All other interfaces are packaged in the *FEMLink* extension. *FEMLink* is installed within the base *SDT* but can only be accessed by licensed users.

You can get a list of currently supported interfaces through the

`comgui('FileExportInfo')`. You will find an up to date list of interfaces with other FEM codes at www.sdtools.com/tofromfem.html). Import of model matrices is discussed in section 4.1.9.

These interfaces evolve with user needs. Please don't hesitate to ask for a patch even during an SDT evaluation by sending a test case to info@sdtools.com.

Interfaces available when this manual was revised were

<code>ans2sdt</code>	reads ANSYS binary files, reads and writes <code>.cdb</code> input (see FEMLink)
<code>abaqus</code>	reads ABAQUS binary output <code>.fil</code> files, reads and writes input and matrix files (<code>.inp</code> , <code>.mtx</code>) (see FEMLink)
<code>nasread</code>	reads the MSC/NASTRAN [26] <code>.f06</code> output file (matrices, tables, real modes, displacements, applied loads, grid point stresses), input <code>bulk</code> file (nodes, elements, properties). FEMLink provides extensions of the basic <code>nasread</code> , <code>output2</code> to model format conversion including element matrix reading, <code>output4</code> file reading, advanced bulk reading capabilities).
<code>naswrite</code>	writes formatted input to the <code>bulk data deck</code> of MSC/NASTRAN (part of SDT), FEMLink adds support for case writing.
<code>nopo</code>	This OpenFEM function reads MODULEF models in binary format.
<code>perm2sdt</code>	reads PERMAS ASCII files (this function is part of FEMLink)
<code>samcef</code>	reads SAMCEF text input and binary output <code>.u18</code> , <code>.u11</code> , <code>.u12</code> files (see FEMLink)
<code>ufread</code>	reads results in the Universal File format (in particular, types: 55 analysis data at nodes, 58 data at DOF, 15 grid point, 82 trace line). Reading of additional FEM related file types is supported by FEMLink through the <code>uf_link</code> function.
<code>ufwrite</code>	writes results in the Universal File format. SDT supports writing of test related datasets. FEMLink supports FEM model writing.

4.1.9 Importing model matrices from other codes

FEMLink handles importing element matrices for NASTRAN (`nasread BuildUp`), ANSYS (`ans2sdt BuildUp`), SAMCEF (`samcef read`) and ABAQUS (`abaqus read`).

Reading of full matrices is supported for NASTRAN in the binary `.op2` and `.op4` formats (writing to `.op4` is also available). For ANSYS, reading of `.matrix` ASCII format is supported. For ABAQUS, reading of ASCII `.mtx` format is supported.

Note that numerical precision is very important when importing model matrices. Storing matrices in 8 digit ASCII format is very often not sufficient.

To incorporate full FEM matrices in a SDT model, you can proceed as follows. A full FEM model matrix is most appropriately integrated as a superelement. The model would typically be composed of

- a mass **m** and stiffness matrix **k** linked to DOFs **mdof** which you have imported with your own code (for example, using `nasread output2` or `output4` and appropriate manipulations to create **mdof**). Note that the `ofact` object provides translation from skyline to sparse format.
- an equivalent mesh defined using standard *SDT* elements. This mesh will be used to plot the imported model and possibly for repeating the model in a periodic structure. If you have no mesh, define nodes and associated mass elements.

`fesuper` provides functions to handle superelements. In particular, `fesuper SEAdd` lets you define a superelement model, without explicitly defining nodes or elements (you can specify only DOFs and element matrices), and add it to another model. Following example loads `ubeam` model, defines additional stiffness and mass matrices (that could have been imported) and a visualization mesh.

```
% Load ubeam model :
model=demosdt('demo ubeam-pro');
cf=feplot; model=cf.mdl;
% Define superelement from element matrices :
SE=struct('DOF',[180.01 189.01]',...
        'K',{[.1 0; 0 0.1] 4e10*[1 -1; -1 1]}},...
        'Klab',{ 'm', 'k' }},...
        'Opt',[1 0; 2 1]);
% Define visualization mesh :
SE.Node=feutil('GetNode 180 | 189',model);
SE.Elt=feutil('ObjectBeamLine 180 189 -egid -1');
% Add as a superelement to model :
model=fesuper('SEadd -unique 1 1 self',model,SE);
```

You can easily define weighting coefficient associated to matrices of the superelement, by defining an element property (see `p_super` for more details). Following line defines a weighting coefficient of 1 for mass and 2 for stiffness (1001 is the `MatId` of the superelement).

```
model.il=[1001 fe_mat('p_super','SI',1) 1 2];
```

You may also want to repeat the superelement defined by element matrices. Following example shows how to define a model, from repeated superelement:

```
% Define matrices (can be imported from other codes) :
model=femesh('testhexa8');
[m,k,mdof]=fe_mk(model);
% Define the superelement:
SE=struct('DOF',[180.01 189.01'],'...
        'K',{[.1 0; 0 0.1] 4e10*[1 -1; -1 1]}},...
        'Klab',{['m','k']}},...
        'Opt',[1 0;2 1]);
SE.Node=model.Node; SE.Elt=model.Elt;
% Add as repeated superelement:
% (need good order of nodes for nodeshift)
model=fesuper('SEAdd -trans 10 0.0 0.0 1.0 4 1000 1000 cube',[],SE);
cf=feplot(model)
```

Superelement based substructuring is demonstrated in [d_cms2](#) which gives you a working example where model matrices are stored in a generic superelement. Note that numerical precision is very important when importing model matrices. Storing matrices in 8 digit ASCII format is very often not sufficient.

4.1.10 feplot FAQ

[feplot](#) lets you define and save advanced views of your model, and export them as [.png](#) pictures.

- **How do I keep group colors constant when I select part of a model?**

One can define different types of color for selection using [fecom ColorData](#). In particular one can color by [GroupId](#), by [ProId](#) or by [MatId](#) using respectively [fecom colordatagroup](#), [colordatapro](#) or [colordatamat](#). Without second argument, colors are attributed automatically. One can define a color map with each row of the form [\[ID Red Green Blue\]](#) as a second argument: [fecom\('colordata',colormap\)](#). All ID do not need to be present in [colormap](#) matrix (colors for missing ID are then automatically attributed). Following example defines 3 color views of the same GART model:

```
cf=demosdt('demo gartFE plot');
% ID Red Green Blue
r1=[(1:10)' [ones(3,1); zeros(7,1)] ...
```

```

    [zeros(3,1); ones(7,1)] zeros(10,1)]; % colormap
fecom('colordatagroup',r1) % all ID associated with color
% redefine groups 4,5 color
cf.Stack{'GroupColor'}(4:5,2:4)=[0 0 1;0 0 1];
fecom('colordatagroup');
% just some ID associated with color
fecom('colordatapro',[1 1 0 0; 3 1 0 0])
fecom('colordatamat') % no color map defined
cf.Stack

```

- **How do I display part of the model as wire frame? (Advanced object handling)**

What is displayed in a `feplot` figure is defined by a set of objects. Once you have plotted your model with `cf=feplot(model)`, you can access to displayed objects through `cf.o(i)` (`i` is the number of the object). Each object is defined by a selection of model elements (`'seli'`) associated to some other properties (see `fecom SetObject`). Selections are defined as `FindElt` commands through `cf.sel(i)`. Displayed objects or selections can be removed using `cf.o(i)=[]` or `cf.sel(i)=[]`.

Following example loads `ubeam` model, defines 2 complementary selections, and displays the second as a wire frame (`ty2`):

```

model=demosdt('demoubeam'); cf=feplot
% define visualisation
cf.sel(1)='WithoutNode{z>1 & z<1.5}';
cf.sel(2)='WithNode{z>1 & z<1.5}';
cf.o(1)={'sel1 ty1','FaceColor',[1 0 0]}; % red patch
cf.o(3)={'sel2 ty2','EdgeColor',[0 0 1]}; % blue wire frame
% reinit visualisation :
cf.sel(1)='groupall';
cf.sel(2)=[]; cf.o(3)=[];

```

- **Is feplot able to display very large models?**

There is no theoretical size limitation for models to be displayed. However, due to the use of Matlab figures, and although optimization efforts have been done, `feplot` can be very slow for large models. This is due to the inefficient use of triangle strips by the Matlab calls to OpenGL, but to ensure robustness SDT still sticks to strict Matlab functionality for GUI operation.

When encountering problems, you should first check that you have an appropriate graphics card, that has a large memory and supports OpenGL and

that the `Renderer` is set to `opengl`. Note also that any X window forwarding (remote terminal) can result in very slow operation: large models should be viewed locally since Matlab does not support an optimized remote client.

To increase fluidity it is possible to reduce the number of displayed patches using `fecom` command `SelReduce rp` where rp is the ratio of patches to be kept. Adjusting rp , fluidity can be significantly improved with minor visual quality loss.

Following example draws a 50x50 patch, and uses `fecom('ReduceSel')` to keep only a patch out of 10:

```
model=fertil('ObjectQuad',[-1 -1 0;-1 1 0;1 1 0;1 -1 0],50,50);
cf=fepplot(model); fecom(cf,'showpatch');
fecom(cf,'SelReduce .1'); % keep only 10% of patches.
```

If you encounter memory problems with `fepplot` consider using `fecom load-hdf`.

- **How do I save figures?**

You should not save `fepplot` figures but models using `fecom Save`.

To save images shown in `fepplot`, you should see `iicom ImWrite`. If using the MATLAB print, you should use the `-noui` switch so that the GUI is not printed. Example `print -noui -depsc2 FileName.eps`.

- MATLAB gives the warning **Warning: RGB color data not yet supported in Painter's mode**. This is due to the use of true colors for `ColorDataMat` and other flat colors. You should save your figure as a bitmap or use the `fecom ShowLine` mode.

- **How do I define a colorbar scale and keep it constant during animation?**

When using `fecom ColorDataEval` commands (useful when displayed deformation is restituted from reduced deformation at each step), color scaling is updated at each step.

One can use `fecom('ScaleColorOne')` to force the colorbar scale to remain constant. In that case one can define the limit of the color map with `set(cf.ga,'clim',[-1 1])` where `cf` is a pointer to target `fepplot` figure, and `-1 1` can be replaced by color map boundaries.

4.2 Other information needed to specify a problem

Once the mesh defined, to prepare analysis one still needs to define

- material and element properties associated to the various elements.
- boundary conditions, constraints (see section 4.2.4) and applied loads (see section 4.2.5)

Graphical editing of case properties is supported by the case tab of the model properties GUI (see section 4.2.3). The associated information is stored in a case data structure which is an entry of the `.Stack` field of the model data structure.

4.2.1 Material and element properties

You can edit material properties using the `Mat` tab of the `Model Properties` figure which lists current materials and lets you choose new ones from the database of each material type. `m_elastic` is the only material function defined for the base `SDT`. It supports elastic materials, linear acoustic fluids, piezo-electric volumes, etc.

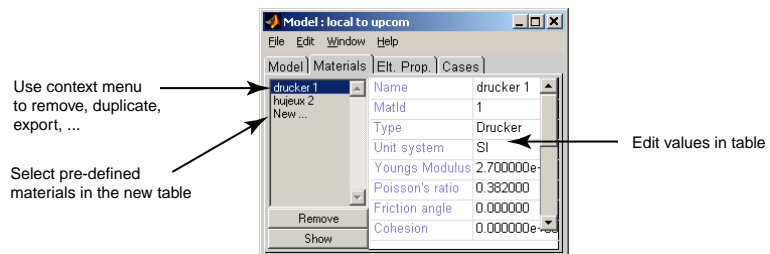



Figure 4.4: Material tab.

Similarly the `ElProp` tab lets you edit element properties. `p_beam` `p_shell` `p_solid` and `p_spring` are supported element property functions.

When the view mode is selected ( icon pressed), you can see the elements affected by each material or element property by selecting it in the associated tab.

You can edit properties using the `Pro` tab of the `Model Properties` figure which lists current properties and lets you choose new ones from the database of each property type (Figure 4.5).

The properties are stored with one property per row in `model.il` (see section 7.3) and `model.il` (see section 7.4). When using scripts, it is often more convenient to use low level definitions of the material properties. For example (see `demo.fe`), one can define aluminum and three sets of beam properties with

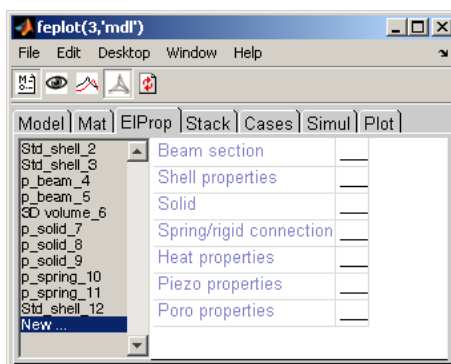


Figure 4.5: Property tab.

```
femesh('reset');
model=femesh('test 2bay plot');
model.pl = m_elastic('dbval 1 steel')
model.il = [ ...
... % ProId SecType          J      I1      I2      A
      1 fe_mat('p_beam','SI',1) 5e-9  5e-9  5e-9  2e-5  0 0 ; ...
      p_beam('dbval 2','circle 4e-3') ; ... % circular section 4 mm
      p_beam('dbval 3','rectangle 4e-3 3e-3')...% rectangular section
];
```

Unit system conversion is supported in property definitions, through two command options.

- `-unit` command option asks for a specific unit system output. It thus expects possible input data in SI, prior to converting (and generating a proper `typ` value).
- `-punit` command option tells the function that a specific unit system is used. It thus expects possible input data in the specified unit system, and generates a proper `typ` value.

The 3 following calls are thus equivalent to define a beam of circular section of 4mm in the MM unit system:

```
il = p_beam('dbval -unit MM 2 circle 4e-3'); % given data in SI, output in MM
il = p_beam('dbval -punit MM 2 circle 4'); % given data in MM, output in MM
il = p_beam('dbval -punit CM -unit MM circle 0.4'); % given data in CM, output
```

To assign a `MatID` or a `ProID` to a group of elements, you can use

- the graphical procedure (in the context menu of the material and property tabs, use the `Select elements and affect ID` procedures and follow the instructions);
- the simple `femesh` set commands. For example `femesh('set group1 mat101 pro103')` will set values 101 and 103 for element group 1.
- more elaborate selections based on `FindElt` commands. Knowing which column of the `Elt` matrix you want to modify, you can use something of the form (see `gartfe`)

```
FEelt(femesh('find EltSelectors'), IDColumn)=ID;
```

You can also get values with `mpid=futil('mpid',elt)`, modify `mpid`, then set values with `elt=futil('mpid',elt,mpid)`.

4.2.2 Other information stored in the stack

The stack can be used to store many other things (options for simulations, results, ...). More details are given in section 7.7. You can get a list of current default entry builders with `fe_def('new')`.

```
info,    EigOpt, getpref('SDT','DefaultEigOpt',[5 20 1e3])
info,    Freq,  getpref('SDT','DefaultFreq',[1:2])
sel,     Sel,   struct('data','groupall','ID',1)
...
```

4.2.3 Cases GUI

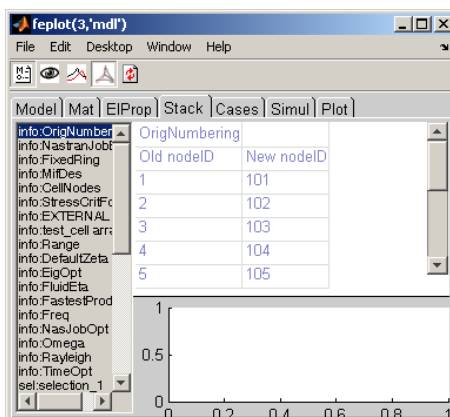


Figure 4.6: Stack tab.

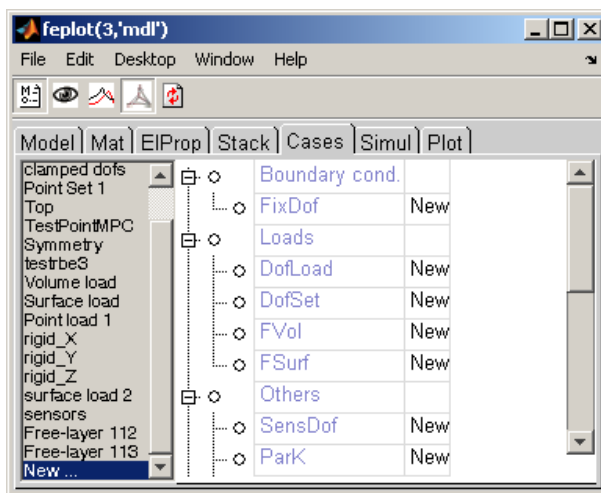


Figure 4.7: Cases properties tab.

When selecting **New ...** in the case property list, as shown in the figure, you get a list of currently supported case properties. You can add a new property by clicking on the associated **new** cell in the table. Once a property is opened you can typically edit it graphically. The following sections show you how to edit these properties through command line or **.m** files.

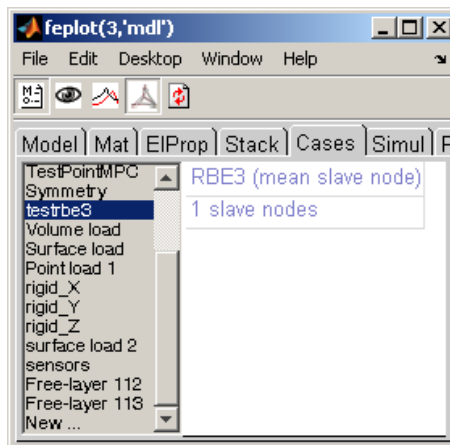


Figure 4.8: Cases properties tab.

4.2.4 Boundary conditions and constraints

Boundary conditions and constraints are described in in `Case.Stack` using `FixDof`, `Rigid`, ... case entries (see `fe_case` and section 7.7). (`KeepDof` still exists but often leads to misunderstanding)

`FixDof` entries are used to easily impose zero displacement on some DOFs. To treat the two bay truss example of section 4.1.4, one will for example use

```
femesh('reset');
model=femesh('test 2bay plot');
model=fe_case(model, ...           % defines a new case
  'FixDof','2-D motion',[.03 .04 .05]', ...
  'FixDof','Clamp edge',[1 2]');
fecom('ProInit') % open model GUI
```

When assembling the model with the specified `Case` (see section 4.2.3), these constraints will be used automatically.

Note that, you may obtain a similar result by building the DOF definition vector for your model using a script. `FindNode` commands allow node selection and `fe_c` provides additional DOF selection capabilities. Details on low level handling of fixed boundary conditions and constraints are given in section 7.14.

4.2.5 Loads

Loads are described in `Case.Stack` using `DOFLoad`, `FVol` and `FSurf` case entries (see `fe_case` and section 7.7).

To treat a 3D beam example with volume forces (x direction), one will for example use

```
femesh('reset');
model = femesh('test ubeam plot');
data = struct('sel','GroupAll','dir',[1 0 0]);
model = fe_case(model,'FVol','Volume load',data);
Load = fe_load(model);
feplot(model,Load);fecom(';undef;triax;ProInit');
```

To treat a 3D beam example with surface forces, one will for example use

```
femesh('reset');
model = femesh('testubeam plot');
data=struct('sel','x==-.5', ...
    'eltsel','withnode {z>1.25}','def',1,'DOF',.19);
model=fe_case(model,'FSurf','Surface load',data);
Load = fe_load(model); feplot(model,Load);
```

To treat a 3D beam example and create two loads, a relative force between DOFs 207x and 241x and two point loads at DOFs 207z and 365z, one will for example use

```
femesh('reset');
model = femesh('test ubeam plot');
data = struct('DOF',[207.01;241.01;207.03],'def',[1 0;-1 0;0 1]);
model = fe_case(model,'DOFLoad','Point load 1',data);
data = struct('DOF',365.03,'def',1);
model = fe_case(model,'DOFLoad','Point load 2',data);
Load = fe_load(model);
feplot(model,Load);
fecom('textnode365 207 241'); fecom('ProInit');
```

The result of `fe_load` contains 3 columns corresponding to the relative force and the two point loads. You might then combine these forces, by summing them

```
Load.def=sum(Load.def,2);
cf.def= Load;
fecom('textnode365 207 241');
```

4.3 Sensors

Sensors are used for test/analysis correlation and in analysis for models where one wants to post-process partial information. This general objective is supported by the use of `SensDof` entries. This section addresses the following issues

- translation measurements associated simplified views (often wire-frame) is classical for modal testing and FEM post-processing. These can be simply defined using a `.tdof` field, see also section 2.2.1 and section 2.2.2 for wire frame geometry and sensor declaration. Commands `trans`, `triax` and `laser` provide simplified calls to generate the associated translation sensors.
- other sensor types typically used in analysis are
 - `rel` relative displacement sensor.
 - `general` general sensor (low level).
 - `resultant` resultant force sensor.
 - `strain` strain or stress sensor.
- topology correlation is the process in which sensor output is related to the DOFs of the underlying FEM. This is implemented as the `SensMatch` command detailed section 4.3.4. In the case of translation measurements, this is only needed for test/analysis correlation.

4.3.1 Sensor GUI, a simple example

Using the `feplot` properties GUI, one can edit and visualize sensors. The following example loads `ubeam` model, defines some sensors and opens the sensor GUI.

```
model=demosdt('demo ubeam-pro');
cf=feplot; model=cf.mdl;

model=fe_case(model,'SensDof append trans','output',...
    [1,0.0,0.5,2.5,0.0,0.0,1.0]); % add a translation sensor
model=fe_case(model,'SensDof append triax','output',8); % add triax sensor
model=fe_case(model,'SensDof append strain','output',...
    [4,0.0,0.5,2.5,0.0,0.0,1.0]); % add strain sensor
```

```

model=fe_case(model,'sensmatch radius1','output'); % match sensor set 'output'

fecom(cf,'promodelviewon');
fecom(cf,'curtab Cases','output'); % open sensor GUI

```

Clicking on **Edit Label** one can edit the full list of sensor labels.

The whole sensor set can be visualized as arrows in the feplot figure clicking on the eye button on the top of the figure. Once visualization is activated one can activate the cursor on sensors by clicking on **CursorSel**. Then one can edit sensor properties by clicking on corresponding arrow in the feplot figure.

The icons in the GUI can be used to control the display of wire-frame, arrows and links.

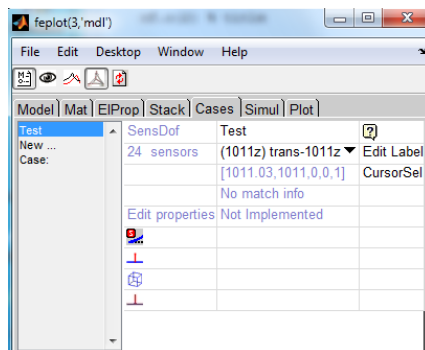


Figure 4.9: GUI for sensor edition

4.3.2 Sensor definition from a cell array

Experimental setups can be defined with a cell array containing all the information relative to the sensors (only displacement/velocity/acceleration sensors are currently supported). This array is meant to be filled any table editor, possibly outside MATLAB. Using EXCEL you can read it with `data=sdtacx('excel read filename',sheetnumber)`.

The first row gives column labels (the order in which they are given is free). Each of the following rows defines a sensor. Known column headers are

- 'lab' contains the names of the sensors. Providing a name for each sensor is mandatory.
- 'SensType' contains optional information such as the name of the sensor manufacturer, their types, *etc.*
- 'SensId' contains the identification numbers of the sensors. Each sensor must have a **unique SensId**.
- 'X', 'Y' and 'Z' contain the cartesian coordinates of each sensor in the **reference frame**. For cylindrical coordinates replace the column headers by 'R', 'Theta' and 'Z' (mixing both types of coordinates inside the cell array is not currently supported). Such columns are mandatory.
- 'DirSpec' contains a specification of the direction in which the measurement is done at each sensor. A minus in front of any specification can be used to generate the opposite direction (-TX for example). Available entries are

'dir x y '	Direction of measurement specified through its components in global coordinates (the vector is normalized).
'X'	[1 0 0], in the reference frame
'Y'	[0 1 0], in the reference frame
'Z'	[0 0 1], in the reference frame
'N'	normal to the element(s) to which the sensor is matched (automatically detected in the subsequent call to SensMatch)
'TX'	tangent to matched surface in the N, X plane.
'TY'	tangent to matched surface in the N, Y plane
'TZ'	tangent to matched surface in the N, Z plane
'N^TX'	tangent orthogonal to the N, X plane
'N^TY'	tangent orthogonal to the N, Y plane
'N^TZ'	tangent orthogonal to the N, Z plane
'laser xs ys zs'	where (x_s, y_s, z_s) are the coordinates of the primary or secondary source (when mirrors are used).

`triax` sensors are dealt with by defining three sensors with the same `'lab'` but different `'SensId'` and `'DirSpec'`. In this case, a straightforward way to define the measurement directions is to make the first axis be the normal to the matching surface. The second axis is then forced to be parallel to the surface and oriented along a preferred reference axis, allowed by the possibility to define `'T*'`. The third axis is therefore automatically built so that the three axes form a direct orthonormal basis with a specification such as `N^T*`. Note that there is no need to always consider the orthonormal basis as a whole and a single `trans` sensor with either `'T*'` or `N^T*` as its direction of measure can be specified.

In the example below, one considers a pentahedron element and aims to observe the displacement just above the slanted face. The first vector is the normal to that face whose coordinates are $[-\sqrt{2}/2, \sqrt{2}/2, 0]$. The second one is chosen (i.) parallel to the observed face, (ii.) in the (x, y) plane and (iii.) along x axis, so that its coordinates are $[\sqrt{2}/2, \sqrt{2}/2, 0]$. Finally, the coordinates of the last vector can only be $[0, 0, -1]$ to comply with the orthonormality conditions. The resulting sensor placement is depicted in figure 4.10

```
cf=feplot;cf.model=femesh('testpenta6');fecom('triax');

% sensor definition as cell array
tcell={'lab', 'SensType', 'SensId', 'X', 'Y', 'Z', 'DirSpec';...
       'sensor 1', '', 1, .4, .6, .5, 'N';
       'sensor 2', '', 2, .4, .6, .5, 'TX';
       'sensor 3', '', 2, .4, .6, 1., 'dir 1 1 1';
       'sensor 4', '', 3, .4, .6, .5, 'N^TX'};disp(tcell)
sens=fe_sens('tdoftable', tcell);
cf.mdl=fe_case(cf.mdl, 'SensDof', 'Test', sens);
cf.mdl=fe_case(cf.mdl, 'SensMatch radius1', 'Test', 'selface');
fecom(cf, 'curtab Cases', 'Test'); fecom(cf, 'ProViewOn')% open sensor GUI
```

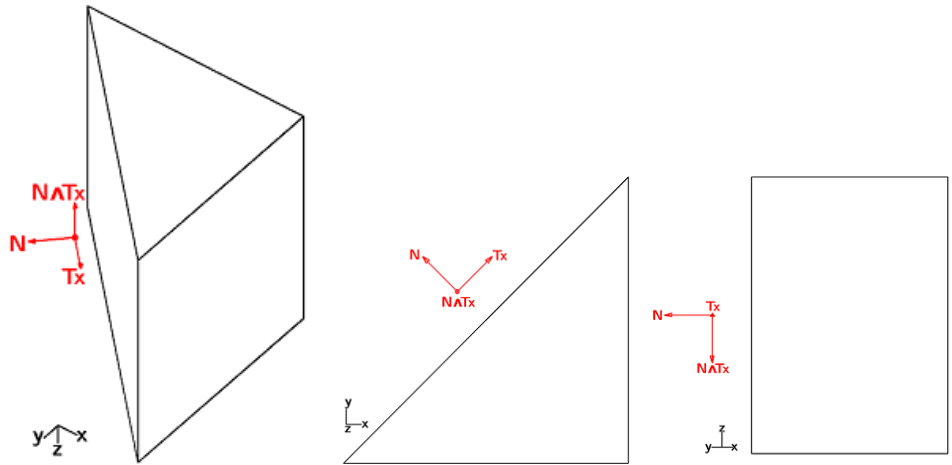


Figure 4.10: Typical axis definition of a triax sensor attached to a [penta6](#)

It is now possible to generate the experimental setup of the [ubeam](#) example described in the previous section by the means of a single cell array containing the data relative to both the [trans](#) and [triax](#) sensors.

```

model=demosdt('demo ubeam-pro');
cf=feplot; model=cf.mdl;
n8=feutil('getnode NodeId 8',model); % triax pos.
tdof={'lab','SensType','SensId','X','Y','Z','DirSpec';...
      'sensor1 - trans','','1,0.0,0.5,2.5','Z';
      'sensor2 - triax','','2,n8(:,5),n8(:,6),n8(:,7)','X';
      'sensor2 - triax','','3,n8(:,5),n8(:,6),n8(:,7)','Y';
      'sensor2 - triax','','4,n8(:,5),n8(:,6),n8(:,7)','Z'};
sens=fe_sens('tdoftable',tdof);
cf.mdl=fe_case(cf.mdl,'SensDof','output',sens);
cf.mdl=fe_case(cf.mdl,'SensMatch radius1');
fecom(cf,'curtab Cases','output'); % open sensor GUI

```

4.3.3 Sensor data structure and init commands

This is a reference section on [SensDof](#) case entries. A tutorial on the basic configuration with a test wire frame and translation sensors is given in section 2.2. [SensDof](#) entries can contain the following fields

<code>sens.Node</code>	(optional) node matrix for sensor nodes that are not in the model.
<code>sens.Elt</code>	element description matrix for a wire-frame display of the sensors (typically for test wire-frames).
<code>sens.bas</code>	Coordinate system definitions for <code>sens.Node</code> , see <code>fe_sens basis</code>
<code>sens.tdof</code>	see details below.
<code>sens.DOF</code>	DOF definition vector for the analysis (finite element model). It defines the meaning of columns in <code>sens.cta</code> .
<code>sens.cta</code>	is an observation matrix associated with the observation equation $\{y\} = [c] \{q\}$ (where q is defined on <code>sens.DOF</code>). This is built using the <code>fe_case sens</code> command illustrated below.
<code>sens.Stack</code>	cell array with one row per sensor giving <code>'sens','SensorTag',data</code> with <code>data</code> is a structure. <code>SensorTag</code> is obtained from <code>SensId</code> (first column of <code>tdof</code>) using <code>feutil('stringdof',SensId)</code> . It is used to define the tag uniquely and may differ from the label that the user may want to associated with a sensor which is stored in <code>data.lab</code> .

The `sens.tdof` field

- nominally is 5 column matrix with rows containing `[SensID NodeID nx ny nz]` giving a sensor identifier (integer or real), a node identifier (positive integer, if relevant), a direction.
- can be single column DOF definition vector which can be transformed to 5 column format using `tdof = fe_sens('tdof',sens.tdof)`
- `SensId` gives an identifier for each sensor. It should thus be unique and there may be conflicts if it is not.
- `NodeId` specifies a node identifier for the spatial localization of the sensor. If not needed (resultant sensors for example), `NodeId` can be set for zero.

A negative `NodeId` is used to look for the node position in `sens.Node` rather than `model.Node`. Mixed definitions (some `NodeId` positive and other negative) are not supported.

Most initialization calls accept the specification of a physical `x y z` position, a `.vert0` field is then defined.

- `nx ny nz` specifies a measurement direction for sensors that need one.

All sensors are generated with the command

```
fe_case(model,'SensDof <append, combine> Sensor_type',Sensor,data,SensLab)
```


Sensor is the case entry name to which sensors will be added. **data** is a structure, a vector, or a matrix, which describes the sensor to be added. The nature of **data** depends on **Sensor_type** as detailed below. **SensLab** is an optional cell array used to define sensor labels. There should be as much elements in **SensLab** as sensors added. If there is only one string in the cell array **SensLab**, it is used to generate labels substituting for each sensor **\$id** by its SensID, **\$type** by its type (trans, strain ...), **\$j1** by its number in the set currently added. If **SensLab** is not given, default label generation is **\$type_-\$id**.

In the default mode (**'SensDof'** command), new sensors replace any existing ones. In the append mode (**'SensDof append'**), if a sensor is added with an existing **SensID**, the **SensID** of new sensor will be changed to a free **SensID** value. In the combine mode (**'SensDof combine'**), existing sensor with the same **SensID** will be replaced by the new one.

rel

Relative displacement sensor or relative force sensor (spring load). Data passed to the command is [**NodeID1 NodeID2**].

This sensor measures the relative displacement between **NodeID1** and **NodeID2**, along the direction defined from **NodeID1** to **NodeID2**. One can use the command option **-dof** in order to measure along the defined DOF directions (needed if the two nodes are coincident). As many sensors as DOF are then added. For a relative force sensor, one can use the command option **-coeff** to define the associated spring stiffness (sensor value is the product of the relative displacement and the stiffness of the spring).

The following example defines 3 relative displacement sensors (one in the direction of the two nodes, and two others along x and y):

```
model=demosdt('demo ubeam-pro')
data=[30 372];
model=fe_case(model,'SensDof append rel','output',data);
model=fe_case(model,'SensDof append rel -dof 1 2','output',data);
```

general

General sensors are defined by a linear observation equation. This is a low level definition that should be used for sensors that can't be described otherwise. Data passed to the command is a structure with field **.cta** (observation matrix), **.DOF** DOF associated to the observation matrix, and possibly **.lab** giving a label for each row of the observation matrix.

The following example defines a general sensor

```
model=demosdt('demo ubeam-pro');
Sensor=struct('cta',[1 -1;0 1],'DOF',[8.03; 9.03]);
model=fe_case(model,'SensDof append general','output',Sensor);
```

trans

Translation sensors (see also section 2.2.2) can be specified by giving

```
[DOF]
[DOF, BasID]
[SensID, NodeID, nx, ny, nz]
[SensID, x, y, z, nx, ny, nz]
```

This is often used with wire frames, see section 2.2.2. The definition of **test** sensors is given in section 3.1.1.

The basic case is the measurement of a translation corresponding the main directions of a coordinate system. The **DOF** format (1.02 for $1y$, see section 7.5) can then be simply used, the **DOF** values are used as is then used as **SensID**. Note that this form is also acceptable to define sensors for other DOFs (rotation, temperature, ...).

A number of software packages use local coordinate systems rather than a direction to define sensors. SDT provides compatibility as follows.

If `model.bas` contains local coordinate systems and deformations are given in the global frame (**DID** in column 3 of `model.Node` is zero), the directions **nx ny nz** (`sens.tdof` columns 3 to 5) must reflect local definitions. A call giving `[DOF, BasID]` defines the sensor direction in the main directions of basis **BasID** and the sensor direction is adjusted.

If FEM results are given in local coordinates, you should not specify a basis for the sensor definition, the directions **nx ny nz** (`sens.tdof` columns 3 to 5) should be `[1 0 0]`, ... as obtained with a simple `[DOF]` argument in the sensor definition call.

When specifying a **BasId**, it the sensor direction **nx ny nz** is adjusted and given in global FEM coordinates. Observation should thus be made using FEM deformations in global coordinates (with a **DID** set to zero). If your FEM results are given in local coordinates, you should not specify a basis for the sensor definition. You can also perform the local to global transformation with

```
cGL= basis('trans E',model.bas,model.node,def.DOF)
def.def=cGL*def.def
```

The last two input forms specify location as **x y z** or **NodeID**, and direction **nx ny nz** (this vector need not be normalized, sensor value is the scalar product of the direction vector and the displacement vector).

One can add multiple sensors in a single call `fe_case(model, 'SensDof <append> trans', Name, Sensor)` when rows of sensors contain sensor entries of the same form.

Following example defines a translation sensor using each of the forms

```
model=demosdt('demo ubeam-pro')
model.bas=basis('rotate', [], 'r=30;n=[0 1 1]', 100);
model=fe_case(model, 'SensDof append trans', 'output', ...
    [1,0.0,0.5,2.5,0.0,0.0,1.0]);
model=fe_case(model, 'SensDof append trans', 'output', ...
    [2,8,-1.0,0.0,0.0]);
model=fe_case(model, 'SensDof append trans', 'output', ...
    [314.03]);
model=fe_case(model, 'SensDof append trans', 'output', ...
    [324.03 100]);
cf=feplot;cf.sel(2)='-output';cf.o(1)={'sel2 ty 7', 'linewidth', 2};
```

Sens.Stack entries for translation can use the following fields

<code>.vert0</code>	physical position in global coordinates.
<code>.ID</code>	NodeId for physical position. Positive if a model node, negative if SensDof entry node.
<code>.match</code>	cell array describing how the corresponding sensor is matched to the reference model. Columns are ElemF, elt, rstj, StickNode .

dof

One can simply define a set of sensors along model DOFs with a direct **SensDof** call `model=fe_case(model, 'SensDof', 'SensDofName', DofList)`. There is no need in that case to pass through **SensMatch** step in order to get observation matrix.

```
model=demosdt('demo ubeam-pro')
model=fe_case(model, 'SensDof', 'output', [1.01;2.03;10.01]);
Sens=fe_case(model, 'sens', 'output')
```

triax,laser

A triax is the same as defining 3 translation sensors, in each of the 3 translation DOF (0.01, 0.02 and 0.03) of a node. Use `fe_case(model, 'SensDof append triax', Name, NodeId)` with a vector `NodeId` to add multiple triaxes. A positive `NodeId` refers to a FEM node, while a negative refers to a wire frame node.

For scanning laser vibrometer tests

```
fe_sens('laser px py pz',model,SightNodes,'SensDofName')
```

appends translation sensors based on line of sight direction from the laser scanner position `px py pz` to the measurement nodes `SightNodes`. Sighted nodes can be specified as a standard node matrix or using a node selection command such as `'NodeId>1000 & NodeId<1100'` or also giving a vector of `NodeId`. If a test wire frame exists in the `SensDofName` entry, node selection command or `NodeId` list are defined in this model. If you want to flip the measurement direction, use a call of the form

```
cf.CStack{'output'}.tdof(:,3:5)=-cf.CStack{'output'}.tdof(:,3:5)
```

The following example defines some laser sensors, using a test wire frame:

```
cf=demosdt('demo gartfeplot'); model=cf.mdl;% load FEM
TEST=demosdt('demo garttwire'); % see sdtweb('pre#presen')
TEST.tdof=[];%Define test wire frame, but start with no tdof
model=fe_case(model,'SensDof','test',TEST)
model=fe_case(model,'SensDof Append Triax','test',-TEST.Node(1))
```

```
% Add sensors on TEST wire frame location
model=fe_sens('laser 0 0 6',model,-TEST.Node(2:end,1),'test');
% Show result
fecom('curtab Cases','output'); fecom('proviewon');
```

To add a sensor on FEM node you would use `model=fe_sens('laser 0 0 6',model,20,'test')` but this is not possible here because `SensDof` entries do not support mixed definitions on test and FEM nodes.

strain,stress

Note that an extended version of this functionality is now discussed in section 4.4. Strain sensors can be specified by giving

```
[SensID, NodeID]
[SensID, x, y, z]
```

```
[SensID, NodeID, n1x, n1y, n1z]
[SensID, x, y, z, n1x, n1y, n1z]
[SensID, NodeID, n1x, n1y, n1z, n2x, n2y, n2z]
[SensID, x, y, z, n1x, n1y, n1z, n2x, n2y, n2z]
```

when no direction is specified 6 sensors are added for stress/strains in the x, y, z, yz, zx, and xy directions (`SensID` is incremented by steps of 1). With `n1x n1y n1z` (this vector need not be normalized) on measures the axial strain in this direction. For shear, one specifies a second direction `n2x n2y n2z` (this vector need not be normalized) (if not given n_2 is taken equal to n_1). The sensor value is given by $\{n_2\}^T [\epsilon] \{n_1\}$.

`Sensor` can also be a matrix if all rows are of the same type. Then, one can add a set of sensors with a single call to the `fe_case(model, 'SensDof <append> strain', Name, Sensor)` command.

Following example defines a strain sensor with each possible way:

```
model=demosdt('demo ubeam-pro')
model=fe_case(model, 'SensDof append strain', 'output', ...
    [4,0.0,0.5,2.5,0.0,0.0,1.0]);
model=fe_case(model, 'SensDof append strain', 'output', ...
    [6,134,0.5,0.5,0.5]);
model=fe_case(model, 'SensDof append strain', 'output', ...
    [5,0.0,0.4,1.25,1.0,0.0,0.0,0.0,0.0,1.0]);
model=fe_case(model, 'SensDof append strain', 'output', ...
    [7,370,0.0,0.0,1.0,0.0,1.0,0.0]);
```

Stress sensor.

It is the same as the strain sensor. The sensor value is given by $\{n_2\}^T [\sigma] \{n_1\}$.

Following example defines a stress sensor with each possible way:

```
model=demosdt('demo ubeam-pro')
model=fe_case(model, 'SensDof append stress', 'output', ...
    [4,0.0,0.5,2.5,0.0,0.0,1.0]);
model=fe_case(model, 'SensDof append stress', 'output', ...
    [6,134,0.5,0.5,0.5]);
model=fe_case(model, 'SensDof append stress', 'output', ...
    [5,0.0,0.4,1.25,1.0,0.0,0.0,0.0,0.0,1.0]);
model=fe_case(model, 'SensDof append stress', 'output', ...
    [7,370,0.0,0.0,1.0,0.0,1.0,0.0]);
```

Element formulations (see section 6.1) include definitions of fields and their derivatives that are strain/stress in mechanical applications and similar quantities other-

wise. The general formula is $\{\epsilon\} = [B(r, s, t)] \{q\}$. These (generalized) strain vectors are defined for all points of a volume and the default is to use an exact evaluation at the location of the sensor.

In practice, the generalized strains are more accurately predicted at integration points. Placing the sensor arbitrarily can generate some inaccuracy (for example stress and strains are discontinuous across element boundaries two nearby sensors might give different results). The `-stick` option can be used to for placement at specific gauss points. `-stick` by itself forces placement of the sensor and the center of the matching element. This will typically be a more appropriate location to evaluate stresses or strains.

To allow arbitrary positioning some level of reinterpolation is needed. The procedure is then to evaluate strain/stresses at Gauss points and use shape functions for reinterpolation. The process must however involve multiple elements to limit interelement discontinuities. This procedure is currently implemented through the `fe_caseg('StressCut')` command, as detailed in section 4.4.

resultant

Resultant sensors measure the resultant force on a given surface. **Note** that the observation of resultant fields is discussed in section 4.4.3. They can be specified by giving a structure with fields

```
.ID          sensor ID.
.EltSel      FindElt command that gives the elements concerned by the resultant.
.SurfSel     FindNode command that gives the surface where the resultant is computed.
.dir         with 3 components direction of resultant measurement, with 6 origin and direction of resulting moment in global coordinates. This vector need not be normalized (scalar product).
.type        contains the string 'resultant'.
```

Following example defines a resultant sensor:

```
model=demosdt('demo ubeam-pro')
Sensor.ID=1;
Sensor.EltSel='WithNode{z==1.25} & WithNode{z>1.25}';
Sensor.SurfSel='z==1.25';
Sensor.dir=[0.0 0.0 1.0];
Sensor.type='resultant';
model=fe_case(model,'SensDof append resultant','output',Sensor);
```

Resultant sensors are not yet available for superelements model.

4.3.4 Topology correlation and observation matrix

Sens

This command is used after `SensMatch` to build the observation equation that relates the response at sensors to the response a DOFs

$$\{y(t)\}_{NS \times 1} = [c]_{NS \times N} \{q(t)\}_{N \times 1} \quad (4.1)$$

where the c matrix is stored in the `sens.cta` field and DOFs expected for q are given in `sens.tdof`.

After the matching phase, one can build the observation matrix with `SensFull=fe_case(model, 'sens', SensDofEntryName)` or when using a reduced superelement model `SensRed=fe_case(model, 'sensSE', SensDofEntryName)`. Note that with superelements, you can also define a field `.UseSE=1` in the sensor entry to force use of the reduced model. This is needed for the generation of reduced selections in `feplot` (typically `cf.sel='-Test'`).

SensMatch

Once sensors defined (see `trans`, ...), sensors must be matched to elements of the mesh. This is done using

```
model = fe_case(model, 'sensmatch', SensDofEntryName);
```

You may omit to provide the name if there is only one sensor set. The command builds the observation matrix associated to each sensor of the entry `Name`, and stores it as a `.cta` field, and associated `.DOF`, in the sensor stack.

Storing information in the stack allows multiple partial matches before generating the global observation matrix. The observation matrix is then obtained using

```
Sens = fe_case(model, 'sens', SensDofEntryName);
```

The matching operation requires finding the elements that contain each sensor and the position within the reference element shape so that shape functions can be used to interpolate the response. Typical variants are

- a `radius` can be specified to modify the default sphere in which a match is sought. This is typically needed in cases some large elements.

```
model=fe_case(model,'sensmatch radius1.0',Name)
```

- elements on which to match can be specified as a `FindElt` string. In particular, matching nodes outside volumes is not accepted. To obtain a match in cases where test nodes are located outside volume elements, you must thus match on the volume surface using

```
fe_case(model,'sensmatch radius1.0',Name,'selface')
```

which selects external surface of volumes and allows a normal projection towards the surface and thus proper match of sensors outside the model volume.

Note that this selection does not yet let you selected implicit elements within a superelement.

- Matching on elements is not always acceptable, one can then force matching to the closest node. `SensMatch-Near` uses the motion at the matched node. `SensMatch-Rigid` uses a rigid body constraints to account for the distance between the matched node and the sensor (but is thus only applicable to cases with rotations defined at the nearby node).

In an automated match, the sensor is not always matched to the correct elements on which the sensor is glued, you may want to ensure that the observation matrices created by these commands only use nodes associated to a subset of elements. You can use a selection to define element subset on which perform the match. If you want to match one or more specific sensors to specific element subset, you can give cell array with `SensId` of sensor to match in a first column and with element string selector in a second column.

```
model=fe_case(model,'SensMatch',Name,{SensIdVector,'FindEltString'});
```

This is illustrated below in forcing the interpolation of test node 1206 to use FEM nodes in the plane where it is glued.

```
cf=demosdt('demo gartte cor plot');
fe_case(cf,'sensmatch -near')
fecom('curtabCases','sensors');fecom('promodelviewon');
% use fecom CursorSelOn to see how each sensor is matched.
cf.CStack{'sensors'}.Stack{18,3}
% modify link to 1206 to be on proper surface
cf.mdl=fe_case(cf.mdl,'SensMatch-near',...
    'sensors',{1206.02,'withnode {z>.16}'});
cf.CStack{'sensors'}.Stack{18,3}
```



```

% force link to given node (may need to adjust distance)
cf.mdl=fe_case(cf.mdl,'SensMatch-rigid radius .5','sensors',{1205.07,21}
cf.CStack{'sensors'}.Stack{19,3}

fecom('showlinks sensors');fecom('textnode',[1206 1205])

```

DofLoadSensDof

The generation of loads is less general than that of sensors. As a result it may be convenient to use reciprocity to define a load by generating the collocated sensor. When a sensor is defined, and the topology correlation performed with `SensMatch`, one can define an actuator from this sensor using

```
model=fe_case(model,'DofLoad SensDof',Input_Name,'Sens_Name:Sens_Nb')
```

or for model using superelements

```
model=fe_case(model,'DofLoad SensDofSE',Input_Name,'Sens_Name:Sens_Nb').
```

`Sens_Name` is the name of the sensor set entry in the model stack of the translation sensor that defines the actuator, and `Sens_Nb` is its number in this stack entry. `Input_Name` is the name of the actuator entry that will be created in the model stack. It will be a `DofLoad` entry.

Animation of sensor wire-frame models

This is discussed in section 2.2.4.

Obsolete

SDT 5.3 match strategies are still available. Only the `arigid` match has not been ported to SDT 6.1. This section thus documents SDT 5.3 match calls.

For topology correlation, the sensor configuration must be stored in the `sens.tdof` field and active FEM DOFs must be declared in `sens.DOF`. If you do not have your analysis modeshapes yet, you can use `sens.DOF=feutil('getdof',sens.DOF)`. With these fields and a combined test/FEM model you can estimate test node motion from FEM results. Available interpolations are

`near` defines the projection based on a nearest node match.

`rigid` defines the projection based on a nearest node match but assumes a rigid body link between the DOFs of the FE model and the test DOFs to obtain the DOF definition vector `adof` describing DOFs used for FEM results.

`arigid` is a variant of the rigid link that estimates rotations based on translations of other nodes. This interpolation is more accurate than `rigid` for solid elements (since they don't have rotational DOFs) and shells (since the value of drilling rotations is often poorly related to the physical rotation of a small segment).

At each point, you can see which interpolations you are using with `fe_sens('info',sens)`. **Note** that when defining test nodes in a local basis, the node selection commands are applied in the global coordinate system.

The interpolations are stored in the `sens.cta` field. With that information you can predict the response of the FEM model at test nodes. For example

```
[model,def]=demosdt('demo gartte cor');
model=fe_sens('rigid sensors',model); % link sensors to model
% display sensor wire-frame and animate FEM modes
cf=feplot; cf.model=model; cf.sel='-sensors';
cf.def=def;fecom(';undefline;scd.5;ch7')
```

4.4 Stress observation

Observation of stress and resultant fields is an application that requires specific tools for performance. A number of commands are thus available for this purpose. The two main commands are `fe_caseg('StressCut')` and `'StressObserve'`.

This functionality has been significantly stabilized for SDT 6.5 but improvements and minor format changes are still likely for future releases.

4.4.1 Building view mesh

Stresses can be observed at nodes of arbitrary meshes (view meshes that are very much related to test wireframes). You should look-up `feutil('object')` commands for ways to build simple shapes. A few alternate model generation calls are provided in `fe_caseg('StressCut')` as illustrated below and in the example for resultant sensors.

```
% Build straight line by weighting of two nodes
VIEW=fe_caseg('stresscut', ...
    struct('Origin',[0 0 0;0 0 1], ... % [n1,n2]
        'steps',linspace(0,1,10)))
```

```

% Automated build of a cut (works on convex cuts)
model=demosdt('demoubeam-pro');cf=feplot;
R0=struct('Origin',[0 0 .5],'axis',[0 0 1]);
VIEW=fe_caseg('StressCut',R0,cf);
feplot(VIEW) % note problem due to non convex cut

%View at Gauss points
model=demosdt('demoubeam-pro');cf=feplot;
cut=fe_caseg('StressCut-SelOut',struct('type','Gauss'),model);

% Observe beam strains at Gauss points
[model,def]=beam1t('testeig')
mo1=fe_caseg('StressCut',struct('type','BeamGauss'),model);
cut=fe_caseg('stresscut -radius 10 -SelOut',mo1,model);
cut.StressObs.CritFcn=''; % Observe all components
C1=fe_caseg('stressobserve',cut,def)

```

4.4.2 Building and using a selection for stress observation

The first use of `StressCut` is to build a `feplot` selection to be used to view/animate stress fields on the view mesh.

A basic example

```

% build model
model=demosdt('volbeam');cf=feplot(model);

% build view mesh
VIEW=fe_caseg('stresscut', ...
    struct('Origin',[0 .05 .05;1 .05 .05], ... % [n1,n2]
    'steps',linspace(1,0,10)))
% build stress cut view selection
sel=fe_caseg('stresscut -selout',VIEW,cf);cla;feplot % generation observa

cf.def=fe_eig(model,[5 10 0]);
fe_caseg('stresscut',sel,cf) % Overlay view and nominal mesh
fecom('scc2') % Force equal scaling

```

4.4.3 Observing resultant fields

StressCut sensors provide stress post-treatments in model cutoffs. The command interprets a data structure with fields

- `.EltSel` `FindElt` command that gives the elements concerned by the resultant.
- `.SurfSel` `FindNode` command that gives the selection where the resultant is computed.
- `.type` contains the string `'resultant'`.

Following example defines a **StressCut** call to show modal stresses in an internal surface of a volumic model

```
demosdt('demoubeam')
cf=feplot;fecom('showpatch')
cf.mdl=feutil('lin2quad',cf.mdl); % better stress interpolation
def=fe_eig(cf.mdl,[5 10 1e3]);
cf.def=def;
r1=struct('EltSel','withnode {z<2}', ...
         'SurfSel','inelt{innode{z==2}}', ...
         'type','Resultant');
fe_caseg('stresscut',r1,cf);
% adapt transparencies
fecom(cf,'SetProp sel(1).fsProp','FaceAlpha',0.01,'EdgeAlpha',0.2);
```

The observation in `feplot` is performed on the fly, with data stored in `cf.sel(2).StressObs` (for the latter example).

Command option `-SelOut` allows recovering the observation data. Field `.cta` is here compatible with general sensors, for customized observation.

```
cta=fe_caseg('StressCut-SelOut',r1,cf);
```

4.5 Computing/post-processing the response

4.5.1 Simulate GUI

Access to standard solvers is provided through the **Simulate** tab of the **Model properties** figure. Experienced users will typically use the command line equivalent to these tabs as detailed in the following sections.

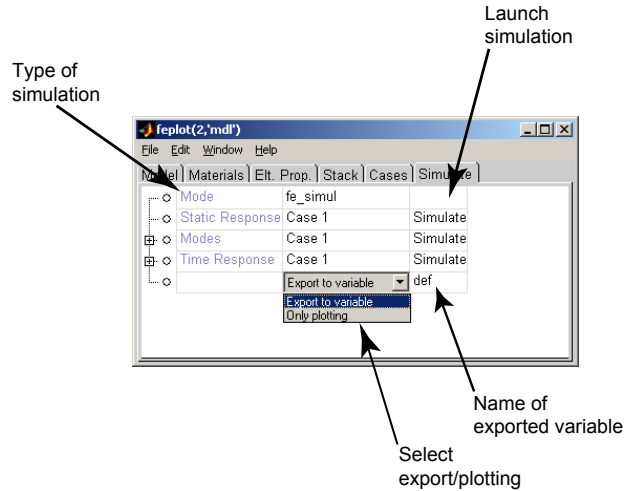


Figure 4.11: Simulation properties tab.

4.5.2 Static responses

The computation of the response to static loads is a typical problem. Once loads and boundary conditions are defined in a case as shown in previous sections, the static response may be computed using the `fe_simul` function.

This is an example of the 3D beam subjected to various type of loads (points, surface and volume loads) and clamped at its base:

```

model=demosdt('demo ubeam vol'); % Initialize a test
def=fe_simul('static',model);% Compute static response
cf=feplot; cf.def=def;% post-process
cf.sel={'Groupall', 'ColorDataStressMises'}

```

Low level calls may also be used. For this purpose it is generally simpler to create system matrices that incorporate the boundary conditions.

`fe_c` (for point loads) and `fe_load` (for distributed loads) can then be used to define unit loads (input shape matrix using *SDT* terminology). For example, a unit vertical input (DOF .02) on node 6 can be simply created by

```

model=demosdt('demo2bay'); Case=fe_case(model,'gett'); %init
% Compute point load
b = fe_c(Case.DOF,[6.02],1)';

```

In many cases the static response can be computed using `Static=kr\b`. For very large models, you will prefer

```
kd=ofact(k); Static = kd\b; ofact('clear',kd);
```

For repeated solutions with the same factored stiffness, you should build the factored stiffness `kd=ofact(k)` and then `Static = kd\b` as many times as needed. Note that `fe_eig` can return the stiffness that was used when computing modes (when using methods without DOF renumbering).

For models with rigid body modes or DOFs with no stiffness contribution (this happens when setting certain element properties to zero), the user interface function `fe_reduc` gives you the appropriate result in a more robust and yet computationally efficient manner

```
Static = fe_reduc('flex',m,k,mdof,b);
```

4.5.3 Normal modes (partial eigenvalue solution)

`fe_eig` computes mass normalized normal modes.

The simple call `def=fe_eig(model)` should only be used for very small models (below 100 DOF). In other cases you will typically only want a partial solution. A typical call would have the form

```

model = demosdt('demo ubeam plot');
cf.def=fe_eig(model,[6 12 0]); % 12 modes with method 6
fecom('colordata stress mises')

```

You should read the `fe_eig` reference section to understand the qualities and limitations of the various algorithms for partial eigenvalue solutions.

You can also load normal modes computed using a finite element package (see section 4.1.8). If the finite element package does not provide mass normalized modes, but a diagonal matrix of generalized masses `mu` (also called modal masses). Mass normalized modeshapes will be obtained using

```
ModeNorm = ModeIn * diag( diag(mu).^(-1/2) );
```

If a mass matrix is given, an alternative is to use `mode = fe_norm(mode,m)`. When both mass and stiffness are given, a Ritz analysis for the complete problem is obtained using `[mode,freq] = fe_norm(mode,m,k)`.

Note that loading modes with in ASCII format 8 digits is usually sufficient for good accuracy whereas the same precision is very often insufficient for model matrices (particularly the stiffness).

4.5.4 State space and other modal models

A typical application of *SDT* is the creation of input/output models in the normal mode `nor`, state space `ss` or FRF `xf` form. (The *SDT* does not replicate existing functions for time response generation such as `lsim` of the *Control Toolbox* which creates time responses using a model in the state-space form).

The creation of such models combines two steps creation of a modal or enriched modal basis; building of input/output model given a set of inputs and outputs.

As detailed in section 4.5.3 a modal basis can be obtained with `fe_eig` or loaded from an external FEM package. Inputs and outputs are easily handled using case entries corresponding to loads (`DofLoad`, `DofSet`, `FVol`, `FSurf`) and sensors (`SensDof`).

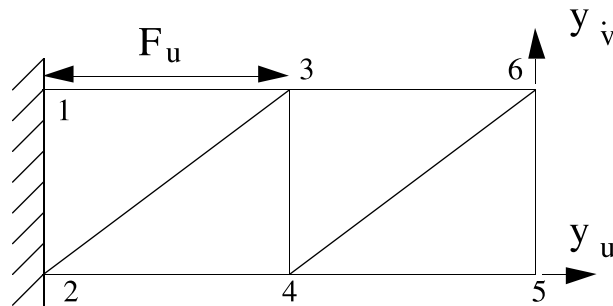


Figure 4.12: Truss example.

For the two bay truss examples shown above, the following script defines a load as the relative force between nodes 1 and 3, and translation sensors at nodes 5 and 6

```
model=demosdt('demo2bay');  
DEF=fe_eig(model,[2 5]); % compute 5 modes  
  
% Define loads and sensors
```

```
Load=struct('DOF',[3.01;1.01],'def',[1;-1]);
Case=fe_case('DofLoad','Relative load',Load, ...
            'SensDof','Tip sensors',[5.01;6.02]);

% Compute FRF and display
w=linspace(80,240,200)';
nor2xf(DEF,.01,Case,w,'hz iipplot "Main" -reset');
```

You can easily obtain velocity or acceleration responses using

```
xf=nor2xf(DEF,.01,Case,w,'hz vel plot');
xf=nor2xf(DEF,.01,Case,w,'hz acc plot');
```

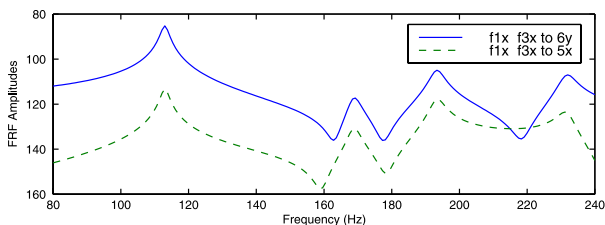


Figure 4.13: FRF synthesis : with and without static correction.

As detailed in section 6.2.3, it is desirable to introduce a static correction for each input. `fe2ss` builds on `fe_reduc` to provide optimized solutions where you compute both modes and static corrections in a single call and return a state-space (or normal mode model) and associated reduction basis. Thus

```
model=demosdt('demo ubeam sens -pro');

model=stack_set(model,'info','Freq',linspace(10,1e3,500)');
model=stack_set(model,'info','DefaultZeta',.01);

[SYS,T]=fe2ss('free 6 10',model); %ii_pof(eig(SYS.a),3)

qbode(SYS,linspace(10,1e3,1500)*2*pi,'iipplot "Initial" -reset');
nor2xf(T,[.04],model,'hz iipplot "Damped" -po');
```

computes 10 modes using a full solution ($Eigopt=[6\ 10\ 0]$), appends the static response to the defined loads, and builds the state-space model corresponding to modal truncation with static correction (see section 6.2.3). **Note** that the load and

sensor definitions were now added to the case in `model` since that case also contains boundary condition definitions which are needed in `fe2ss`.

The different functions using normal mode models support further model truncation. For example, to create a model retaining the first four modes, one can use

```
model=demosdt('demo2bay');
DEF=fe_eig(model,[2 12]); % compute 12 modes
Case=fe_case('DofLoad','Horizontal load',3.01, ...
            'SensDof','Tip sensors',[5.01;6.02]);
SYS =nor2ss(DEF,.01,Case,1:4);
ii_pof(eig(SYS.a)/2/pi,3) % Frequency (Hz), damping
```

A static correction for the displacement contribution of truncated modes is automatically introduced in the form of a non-zero `d` term. When considering velocity outputs, the accuracy of this model can be improved using static correction modes instead of the `d` term. Static correction modes are added if a roll-off frequency `fc` is specified (this frequency should be a decade above the last retained mode and can be replaced by a set of frequencies)

```
SYS =nor2ss(DEF,.01,Case,1:4,[2e3 .2]);
ii_pof(eig(SYS.a)/2/pi,3,1) % Frequency (Hz), damping
```

Note that `nor2xf` always introduces a static correction for both displacement and velocity.

For damping, you can use uniform modal damping (a single damping ration `damp=.01` for example), non uniform modal damping (a damping ratio vector `damp`), non-proportional modal damping (square matrix `ga`), or hysteretic (complex `DEF.data`). This is illustrated in `demo_fe`.


4.5.5 Viewing deformation

The following initializes a set of deformations, shows deformation 7 (first flexible mode), opens the `feplot` properties figure and shows the `Deformations` tab.

```
[model,def]=demosdt('Demo gartfe');
cf=feplot(model,def);
fecom('ch7');
fecom('pro');
fecom(cf,'CurTab Deformations')
```

Note that `cf.def=ci.Stack{'IdMain'}` or `cf.def=Up.def` would be acceptable for database wrapper (`ci.Stack{'IdMain'}` is used by `idcom` to store identification results) or type 3 superelement input.

Scan through the various deformations using the `+/-` buttons/keys or clicking in the deformations list in the `Deformations` tab. From the command line you can use `fecom ch` commands.

Animate the deformations by clicking on the  button. Notice how you can still change the current deformation, rotate, etc. while running the animation. Animation properties can be modified with `fecom Anim` commands or in the `General` tab of the `feplot properties` figure.

Modeshape scaling can be modified with the `l/L` key, with `fecom Scale` commands or in the `Axes` tab of the `feplot properties` figure.

You may also want to visualize the measurement at various sensors (see section 4.3 and `fe_sens`) using a stick or arrow sensor visualization (`fecom showsens` or `fecom showarrow`). On such plots, you can label some or all degrees of freedom using the call `fecom ('doftext', idof)`.

Look at the `fecom` reference section to see what modifications of displayed plots are available.

Superposing deformations

Modeshape superposition is an important application (see plot of section 2.2.1) which is supported by initializing deformations with the two deformation sets given sequentially and a `fecom ch` command declaring more than one deformation. For example you could compare two sets of deformations using

```
[model,def]=demosdt('demo gartfe');
cf=feplot(model);cf.def(1)=def;
def.def=def.def+rand(size(def.def))/5;
cf.def(2)=def;
fecom('show2def');
fecom('scalematch');
```

where the `scalematch` command is used to compare deformations with unequal scaling. You could also show two deformations in the same set

```
cf=demosdt('demo gartfe plot');
fecom(';showline; ch7 10')
```

The `-,+` buttons/commands will then increment both deformations numbers (overlay 8 and 11, etc.).

Element selections

Element selections play a central role in `feplot`. They allow selection of a model subpart (see section 7.12) and contain color information. The following example selects some groups and defines color to be the z component of displacement or all groups with strain energy deformation (see `fecom ColorData` commands)

```
cf=demosdt('demo gartfe plot');
cf.sel(1)={'group4:9 & group ~=8', 'colordata z'};
pause
cf.def=fe_eig(cf.mdl, [6 20 1e3]);
cf.sel(1)={'group all', 'colordata enerk'};
fecom('colorbar');
```

You can also have different objects point to different selections. This model has an experimental mesh stored in element group 11 (it has `EGID -1`). The following commands define a selection for the FEM model (groups 1 to 10) and one for the test wire frame (it has `EGID<0`). The first object `cf.o(1)` displays selection 1 as a surface plot (`ty1` with a blue edge color. The second object displays selection to with a thick red line.

```
cf=demosdt('demo gartfe plot');
cf.sel(1)={'group1:10'}; cf.sel(2)='egid<0';
cf.o(1)={'ty1 def1 sel1', 'edgecolor', 'b'}
cf.o(2)={'ty2sel2', 'edgecolor', 'r', 'linewidth', 2}
```

Note that you can use `FindNode` commands to display some node numbers. For example try `fecom('textnode egid<0 & y>0')`.

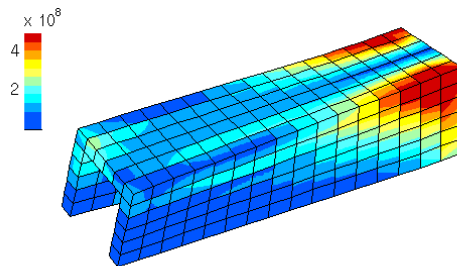


Figure 4.14: Stress level plot.

4.5.6 Time computation

To perform a full order model time integration, one needs to have a model, a load and a curve describing time evolution of the load.

```
% define model and load
model=fe_time('demo bar');fe_case(model,'info')
% define curves
model=fe_curve(model,'set','input','step 1e-4*10');
model=fe_curve(model,'set','ramp','TestRamp .1 2 1.1');
model=fe_curve(model,'set','ricker','TestRicker 10e-4 1');
model=fe_curve(model,'set','sinus',...
    'Test eval sin(2*pi*1000*t)');
model=fe_curve(model,'set','sinus2',...
    struct('X',linspace(0,100,10)',...
    'Y',sin(linspace(0,100,10)'))); % tabulated
% have load reference 'input' curve
model=fe_case(model,'setcurve','Point load 1','input');

cf=feplot(model) % plot the model
```

Once model is plotted in `feplot` one can edit each curve under the model properties Stack tab. Parameters can be modified. Curve can be plotted in `iipplot` using the `Show` popup button. One has to define the number of steps (`NStep`) and the total time to be displayed (`Tf`) and click `Using NStep & Tf`. One can also display curve on the `info TimeOpt` time options by clicking on `Using TimeOpt`.

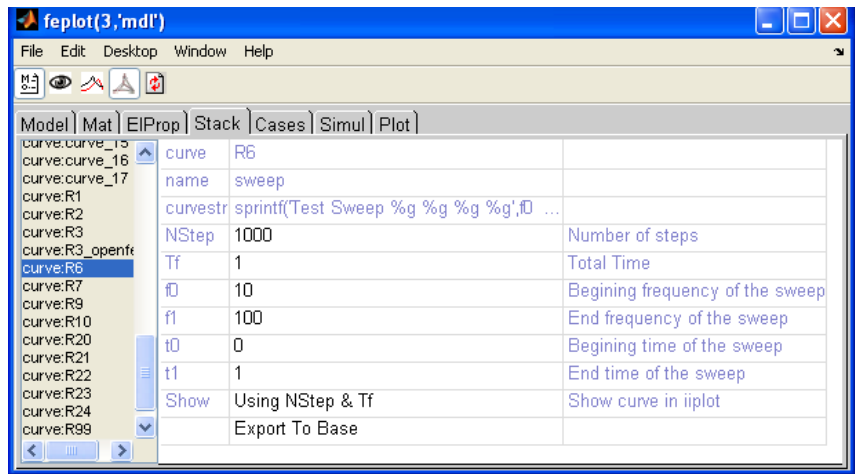


Figure 4.15: GUI associated to a curve

One can change the curve associated to the load in the Case tab.

```
cf.Stack{'info','TimeOpt'}=...
    fe_time('timeopt newmark .25 .5 0 1e-4 100'); % define time computat
cf.def=fe_time(cf.mdl); % compute
figure;plot(cf.def.data,cf.def.def(cf.def.DOF==2.01,:)); % show 2.01 res
```

Time domain responses can also be obtained by inverse transform of frequency responses as illustrated in the following example

```
model=demosdt('demo ubeam sens');DEF=fe_eig(model,[5 10 1e3]);

w=linspace(0,600,6000)'; % define frequencies
R1=nor2xf(DEF,.001,model,w,'hz struct'); % compute freq resp.
R2=ii_mmif('ifft -struct',R1);R2.name='time'; % compute time resp.
iipplot(R2);iicom(';sub 1 1 1 1 3;ylin'); % display
```

4.5.7 Manipulating large finite element models

The flexibility given by the MATLAB language comes at a price for large finite element computations. The two main bottlenecks are model assembly and static computations.

During assembly compiled elements provided with OpenFEM allow much faster element matrix evaluations (since these steps are loop intensive they are hard to optimize in MATLAB). The `sp_util.mex` function alleviates element matrix assembly and large matrix manipulation problems (at the cost of doing some very dirty tricks like modifying input arguments).

Starting with SDT 6.1, `model.Dbfile` can be defined to let SDT know that the file can be used as a database. In particular optimized assembly calls (see section 4.5.8) make use of this functionality. The database is a `.mat` file that uses the HDF5 format defined for MATLAB $i=7.3$.

For static computations, the `ofact` object allows method selection. Currently the most efficient (and default `ofact` method) is the multi-frontal sparse solver `spfmex`. This solver automatically performs equation reordering so this needs not be done elsewhere. It does not use the MATLAB memory stack which is more efficient for large problems but requires `ofact('clear')` calls to free memory associated with a given factor.

With other static solvers (MATLAB `lu` or `chol`, or SDT true skyline `sp_util` method) you need to pay attention to equation renumbering. When assembling large models, `fe_mk` (obsolete compared to `fe_mknl`) will automatically renumber DOFs to minimize matrix bandwidth (for partial backward compatibility automatic renumbering is only done above 1000 DOF).

The real limitation on size is linked to performance drops when swapping. If the factored matrix size exceeds physical memory available to MATLAB in your computer, performance tends to decrease drastically. The model size at which this limit is found is very much model/computer dependent.

Finally in `fe_eig`, method 6 (IRA/Sorensen) uses low level BLAS code and thus tends to have the best memory performance for eigenvalue computations.

Note finally, that you may want to run MATLAB with the `-nojvm` option turned on since it increases the memory addressable by MATLAB (version $i=6.5$).

For out-of-core operations (supported by `fe_mk`, `upcom`, `nasread` and other functions). SDT creates temporary files whose names are generated with `nas2up('tempnameExt')`. You may need to set `sdtdef('tempdir', 'your_dir')` to an appropriate location. The directory should be located on a local disk or a high speed disk array. If you have a RAID array, use a directory there.

4.5.8 Optimized assembly strategies

The handling of large models, often requires careful sequencing of assembly operations. While `fe_mknl`, `fe_load`, and `fe_case`, can be used for user defined procedures, SDT operations typically use the an internal (closed source) assembly call to `fe_case Assemble`. Illustrations of most calls can be found in `fe_simul`.

`[k,mdl,Case,Load]=fe_case(mdl,'assemble matdes 1 NoT loadback',Case);` return the stiffness without constraint elimination and evaluates loads.

`[SE,Case,Load,Sens]=fe_case(mdl,'assemble -matdes 2 1 3 4 -SE NoTload Sens');` returns desired matrices in SE.K, the associated case, load and sensors (as requested in the arguments).

Accepted command options for the assemble call are

- `-fetime` forces the nominal assembly using mass, viscous damping and stiffness, output in this order: `2 3 1`. If a reduced model is defined as an `SE,MVR`, the assembly is shortcut to output `MVR` as the assembled model, and `MVR.Case` as the Case. If the field `.Case` is absent, the case stacked in the base model is output.
- `-reset` forces reassembly even if the `.K` field is defined and filled.
- `keep` retains `model.DOF` even if some DOF are unused.
- `load` requires load assembly and output.
- `sens` requires sensor assembly and output.
- `GetT` outputs a struct containing `Case.Stack`, `Case.T` and `Case.DOF`.
- `NoT` is the usual option to prevent constraint elimination (computation of $T^T K T$). With `NoT` DOFs are given in `model.DOF` or `Case.mDOF`. Without the option they are consistent with `Case.DOF`.
- `-MatDes` specifies the list of desired matrices. Basic types are `2` for mass and `1` for stiffness, for a complete list see `MatType`. `-1` is used separate matrices associated with parameters (see `upcom Par`). `-1.1` removes the subparameters from the nominal matrix. `-2` is used to obtain matrices associated with assembled superelements.
With `5` and a predefined deformation in `'curve'`, `'StaticState'`, the internal load is computed and added to returned loads.
- `InitFcn` allows pre-emptive behavior at the beginning of assembly. `ExitFcn` does the same at exit.

- `-SE` returns the assembled result as a superelement structure. One can use `-SeCDof` (superelement Case DOF) to fill `.DOF` field with constrained DOF (`Case.DOF`).
- `-cell` sets the first output as a cell array containing all assembled matrices.
- `-cfield` keeps the `Case.MatGraph` to allow further reassembly.

Structural dynamic concepts

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This theoretical chapter is intended as a reference for the fundamental notions and associated variables used throughout the *SDT*. This piece of information is grouped here and hypertext reference is given in the HTML version of the manual.

Models of dynamic systems are used for identification phases and links with control applications supported by other MATLAB toolboxes and SIMULINK. Key concepts and variables are

<code>b, c</code>	input/output shape matrices (<code>b, c, pb, cp</code> variables)
<code>nor</code>	normal mode models (<code>freq, damp, cp, pb</code> variables)
<code>damp</code>	damping for full and reduced models
<code>cp, cpx</code>	complex mode models (<code>lambda, psi</code> variables)
<code>res</code>	pole/residue model (<code>res, po</code> variables)
<code>ss</code>	state space model (<code>a, b, c, d</code> variables)
<code>tf</code>	parametric transfer function (<code>num, den</code> variables)
<code>xf</code>	non-parametric transfer function (<code>w, xf</code> variables)

5.1 I/O shape matrices

Dynamic loads applied to a discretized mechanical model can be decomposed into a product $\{F\}_q = [b] \{u(t)\}$ where

- the **input shape matrix** $[b]$ is time invariant and characterizes spatial properties of the applied forces
- the vector of inputs $\{u\}$ allows the description of the time/frequency properties.

Similarly it is assumed that the outputs $\{y\}$ (displacements but also strains, stresses, etc.) are linearly related to the model coordinates $\{q\}$ through the sensor **output shape matrix** ($\{y\} = [c] \{q\}$).

Input and output shape matrices are typically generated with `fe_c` or `fe_load`. Understanding what they represent and how they are transformed when model DOFs/states are changed is essential.

Linear mechanical models take the general forms

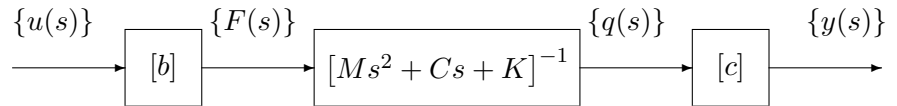
$$\begin{aligned} [Ms^2 + Cs + K]_{N \times N} \{q(s)\} &= [b]_{N \times NA} \{u(s)\}_{NA \times 1} \\ \{y(s)\}_{NS \times 1} &= [c]_{NS \times N} \{q(s)\}_{N \times 1} \end{aligned} \quad (5.1)$$

in the frequency domain (with $Z(s) = Ms^2 + Cs + K$), and

$$\begin{aligned} [M] \{\ddot{q}\} + [C] \{\dot{q}\} + [K] \{q\} &= [b] \{u(t)\} \\ \{y(t)\} &= [c] \{q(t)\} \end{aligned} \quad (5.2)$$

in the time domain.

In the model form (5.1), the first set of equations describes the evolution of $\{q\}$. The components of q are called Degrees Of Freedom (DOFs) by mechanical engineers and states in control theory. The second *observation* equation is rarely considered by mechanical engineers (hopefully the *SDT* may change this). The purpose of this distinction is to lead to the block diagram representation of the structural dynamics



which is very useful for applications in both control and mechanics.

In the simplest case of a point force input at a DOF q_l , the input shape matrix is equal to zero except for DOF l where it takes the value 1

$$[b_l] = \begin{bmatrix} \vdots \\ 0 \\ 1 \\ 0 \\ \vdots \end{bmatrix} \leftarrow l \quad (5.3)$$

Since $\{q_l\} = [b_l]^T \{q\}$, the transpose this Boolean input shape matrix is often called a *localization matrix*. Boolean input/output shape matrices are easily generated by `fe_c` (see the section on DOF selection page 243).

Input/output shape matrices become really useful when not Boolean. For applications considered in the *SDT* they are key to

- distributed FEM loads, see `fe_load`.
- test analysis correlation. Since you often have measurements that do not directly correspond to DOFs (accelerations in non global directions at positions that do not correspond to finite element nodes, see section 2.2.2).

- model reduction. To allow the changes to the DOFs q while retaining the physical meaning of the I/O relation between $\{u\}$ and $\{y\}$ (see section 6).

5.2 Normal mode models

The spectral decomposition is a key notion for the resolution of linear differential equations and the characterization of system dynamics. Predictions of the vibrations of structures are typically done for linear elastic structures or, for non-linear cases, refer to an underlying tangent elastic model.

Spectral decomposition applied to elastic structures leads to *modal analysis*. The main objective is to correctly represent low frequency dynamics by a low order model whose size is typically orders of magnitude smaller than that of the finite element model of an industrial structure.

The use of normal modes defined by the spectral decomposition of the elastic model and corrections (to account for the restricted frequency range of the model) is fundamental in modal analysis.

Associated models are used in the **normal mode model format**

$$\begin{aligned} [[I] s^2 + [\Gamma] s + [\Omega^2]] \{p(s)\} &= [\phi^T b] \{u(s)\} \\ \{y(s)\} &= [c\phi] \{p(s)\} \end{aligned} \quad (5.4)$$

where the modal masses (see details below) are assumed to be unity.

The `nor2res`, `nor2ss`, and `nor2xf` functions are mostly based on this model form (see `nor2ss` theory section). They thus support a low level entry format with four arguments

- `om` *modal stiffness matrix* Ω^2 . In place of a full modal stiffness matrix `om`, a vector of *modal frequencies* `freq` is generally used (**in rad/s** if **Hz** is not specified in the type string). It is then assumed that `om=diag(freq.^2)`. `om` can be complex for models with structural damping (see the section on damping page 156).
- `ga` *modal damping matrix* Γ (viscous). *damping ratios* `damp` corresponding to the modal frequencies `freq` are often used instead of the modal damping matrix `ga` (`damp` cannot be used with a full `om` matrix). If `damp` is a vector of the same size as `freq`, it is then assumed that `ga=diag(2*freq.*damp)`. If `damp` is a scalar, it is assumed that `ga=2*damp*diag(freq)`. The application of these models is discussed in the section on damping page 156).
- `pb` *modal input matrix* $\{\phi_j\}^T [b]$ (input shape matrix associated to the use of modal coordinates).
- `cp` *modal output matrix* $[c] \{\phi_j\}$ (output shape matrix associated to the use of modal coordinates).

Higher level calls, use a data structure with the following fields

- `.freq` frequencies (units given by `.fsc` field, $2*\pi$ for Hz). This field may be empty if a non diagonal `nor.om` is defined.
- `.om` alternate definition for a non diagonal reduced stiffness. Nominally `om` contains `diag(freq.^2)`.
- `.damp` modal damping ratio. Can be a scalar or a vector giving the damping ratio for each frequency in `nor.freq`.
- `.ga` alternate definition for a non diagonal reduced viscous damping.
- `.pb` input shape matrix associated with the generalized coordinates in which `nor.om` and `nor.ga` are defined.
- `.cp` output shape matrix associated with the generalized coordinates in which `nor.om` and `nor.ga` are defined.
- `.dof_in` A six column matrix where each row describes a load by [`SensID` `NodeID` `nx` `ny` `nz` `Type`] giving a sensor identifier (integer or real), a node identifier (positive integer), the projection of the measurement direction on the global axes (if relevant), a `Type`.
- `.lab_in` A cell array of string labels associated with each input.
- `.dof_out` A six column matrix describing outputs following the `.dof_in` format.
- `.lab_out` A cell array of string labels associated with each output.

General load and sensor definitions are then supported using cases (see section 4.2.3).

Transformations to other model formats are provided using `nor2ss` (state-space model), `nor2xf` (FRFs associated to the model in the `xf` format), and `nor2res`

(complex residue model in the `res` format). The use of these functions is demonstrated in `demo_fe`.

Transformations **from** other model formats are provided by `fe2ss`, `fe_eig`, `fe_norm`, ... (from full order finite element model), `id_nor` and `res2nor` (from experimentally identified pole/residue model).

5.3 Damping

Models used to represent dissipation at the local material level and at the global system level should typically be different. Simple viscous behavior is very often not appropriate to describe material damping while a viscous model is appropriate in the normal mode model format (see details in Ref. [27]). This section discusses typical damping models and discusses how piece-wise Rayleigh damping is implemented in SDT.

5.3.1 Viscous damping in the normal mode model form

In the normal mode form, viscous damping is represented by the modal damping matrix Γ which is typically used to represent all the dissipation effects at the system level.

Models with **modal damping** assume that a diagonal Γ is sufficient to represent dissipation at a system level. The non-zero terms of Γ are then usually expressed in terms of damping ratios $\Gamma_{jj} = 2\zeta_j\omega_j$. The damping ratio ζ_j are accepted by most *SDT* functions instead of a full Γ . The variable name `damp` is then used instead of `ga` in the documentation.

For a model with modal damping, the matrices in (6.86) are diagonal so that the contributions of the different normal modes are uncoupled and correspond exactly to the spectral decomposition of the model (see `cpx` page 163 for the definition of complex modes). The rational fraction expression of the dynamic compliance matrix (transfer from the inputs $\{u\}$ to displacement outputs $\{y\}$) takes the form

$$[\alpha(s)] = \sum_{j=1}^N \frac{\{c\phi_j\} \{b^T\phi_j\}^T}{s^2 + 2\zeta_j\omega_j s + \omega_j^2} = \sum_{j=1}^N \frac{[T_j]_{NS \times NA}}{s^2 + 2\zeta_j\omega_j s + \omega_j^2} \quad (5.5)$$

where the contribution of each mode is characterized by the pole frequency ω_j , damping ratio ζ_j , and the residue matrix T_j (which is equal to the product of the

normal mode output shape matrix $\{c\phi_j\}$ by the normal mode input shape matrix $\{\phi_j^T b\}$.

Modal damping is used when lacking better information. One will thus often set a uniform damping ratio ($\zeta_j = 1\%$ or `damp = 0.01`) or experimentally determined damping ratios that are different for each pole (`po=ii_pof(po,3); damp=po(:,2);`).

Historically, modal damping was associated to the **proportional damping model** introduced by Lord Rayleigh which assumes the usefulness of a global viscously damped model with a dynamic stiffness of the form

$$[Z(s)] = [Ms^2 + (\alpha M + \beta K)s + K]$$

While this model indeed leads to a modally damped normal mode model, the α and β coefficients can only be adjusted to represent physical damping mechanisms over very narrow frequency bands. The modal damping matrix thus obtained writes

$$\Gamma = [\alpha + \beta\omega_j^2]$$

which leads to damping ratios

$$2\zeta_j = \frac{\alpha}{\omega_j} + \beta\omega_j$$

Mass coefficient α leads to high damping ratios in the low frequency range. Stiffness coefficient β leads to a damping ratio linearly increasing with the frequency.

Using a diagonal $[\Gamma]$ can introduce significant errors when normal mode coupling through the spatial distribution of damping mechanisms is possible. The condition

$$2\zeta_j\omega_j/|\omega_j - \omega_k| \ll 1$$

proposed by Hasselman [28], gives a good indication of when modal coupling will not occur. One will note that a structure with a group of modes separated by a few percent in frequency and levels of damping close to 1% does not verify this condition. The uncoupling assumption can however still be applied to blocks of modes [12].

A normal mode model with a full Γ matrix is said to be *non-proportionally damped* and is clearly more general/accurate than the simple modal damping model. The *SDT* leaves the choice between the non-proportional model using a matrix `ga` and the proportional model using damping ratio for each of the pole frequencies (in this case one has `ga=2*diag(damp.*freq)` or `ga=2*damp*diag(freq)` if a scalar uniform damping ratio is defined).

For identification phases, standard approximations linked to the assumption of

modal damping are provided by (`id_rc`, `id_rm` and `res2nor`), while `id_nor` provides an original algorithm of the determination of a full Γ matrix. Theoretical aspects of this algorithm and details on the approximation of modal damping are discussed in [12]).

5.3.2 Viscous damping in finite element models

Standard damped finite element models allow the incorporation of viscous and structural damping in the form of real C and complex K matrices respectively.

`fe_mk` could assemble a viscous damping matrix with user defined elements that would support matrix type 3 (viscous damping) using a call of the form `fe_mk(MODEL, 'options', 3)` (see section 7.16 for new element creation). Viscous damping models are rarely appropriate at the finite element level [27], so that it is only supported by `celas` and `cbush` elements. Piece-wise Rayleigh damping where the viscous damping is a combination of element mass and stiffness on element subsets

$$C = \sum_{j=1}^{NS} [\alpha_j^S M_j^S + \beta_j^S K_j^S] \quad (5.6)$$

is supported as follows. For each material or group that is to be considered in the linear combination one defines a row entry giving `GroupId MatId AlphaS BetaS` (note that some elements may be counted twice if they are related to a group and a material entry). One can alternatively define `ProId` as a 5th column (useful for `celas` element that have no `matid`). For example

```
model=demosdt('demogartfe');
model=stack_set(model,'info','Rayleigh', ...
    [10 0 1e-5 0.0; ... % Elements of group 10 (masses)
     9 0 0.0 1e-3; ... % Elements of group 9 (springs)
     0 1 0.0 1e-4; ... % Elements with MatId 1
     0 2 0.0 1e-4]); % Elements with MatId 2
% Note that DOF numbering may be a problem when calling 'Rayleigh'
% See sdtweb simul#feass for preferred assembly in SDT
c=feutilb('Rayleigh',model); figure(1);spy(c);

dc=fe_ceig(model,[1 5 20 1e3]);cf=feplot(model,dc);
```

Such damping models are typically used in time integration applications. `Info,Rayleigh` entries are properly handled by `Assemble` commands.

You can also provide `model=stack_set(model,'info','Rayleigh',[alpha beta])`.

Note that in case of rayleigh damping, elas element viscous damping will also be taken into account.

5.3.3 Hysteretic damping in finite element models

Structural or hysteretic damping represents dissipation by giving a loss factor at the element level leading to a dynamic stiffness of the form

$$Z(s) = [Ms^2 + K + iB] = Ms^2 + \sum_{j=1}^{NE} [K_j^e] (1 + i\eta_j^e) \quad (5.7)$$

The name *loss factor* derives from the fact that η is equal to the ratio of energy dissipated for one cycle $E_d = \int_0^T \sigma \dot{\epsilon} dt$ by 2π the maximum potential energy $E_p = 1/2E$.

If all the dissipative materials used have a loss factor property, these are used by `Assemble` commands with a desired matrix type 4.

Using complex valued constitutive parameters will not work for most element functions. Hysteretic damping models can thus be assembled using the `Rayleigh` command shown above (to assemble the imaginary part of K rather than C or using `upcom` (see section 6.4). The following example defines two loss factors for group 6 and other elements of the Garteur FEM model. Approximate damped poles are then estimated on the basis of real modes (better approximations are discussed in [29])

```
Up=upcom('load GartUp'); cf=feplot(Up);
Up=fe_case(Up,'parReset', ...
    'Par k','Constrained Layer','group 6', ...
    'Par k','Main Structure','group~=6');

%      type cur min max vtype
par = [ 1   1.0 0.1 3.0   1 ; ...
        1   1.0 0.1 3.0   1 ];
Up=upcom(Up,'ParCoef',par);

% assemble using different loss factors for each parameter
B=upcom(Up,'assemble k coef .05 .01');
[m,k]=upcom(Up,'assemble coef 1.0 1.0');
Case=fe_case(Up,'gett');

% Estimate damped poles on real mode basis
```

5 Structural dynamic concepts

```
def=fe_eig({m,k,Case.DOF},[6 20 1e3]);
mr=def.def'*m*def.def; % this is the identity
cr=zeros(size(mr));
kr=def.def'*k*def.def+i*(def.def'*B*def.def);
dr=fe_ceig({mr,cr,kr,[]});dr.def=def.def*dr.def;dr.DOF=def.DOF;
cf.def=dr
```

Note that in this model, the poles λ_j are not complex conjugate since the hysteretic damping model is only valid for positive frequencies (for negative frequencies one should change the sign of the imaginary part of K).

Given a set of complex modes you can compute frequency responses with `res2xf`, or simply use the modal damping ratio found with `fe_ceig`. Continuing the example, above one uses

```
Up=fe_case(Up,'Dofload','Point loads',[4.03;55.03], ...
           'SensDof','Sensors',[4 55 30]'+.03);
Sens=feutilb('placeindof',def.DOF,fe_case(Up,'sens'));
Load=fe_load(Up);
ind=find(dr.data(:,1)>5); % flexible modes

% Standard elastic response with modal damping
f=linspace(5,60,2048);
d1=def; d1.data(7:20,2)=dr.data(ind,2);
nor2xf(d1,Up,f,'hz iipplot "Normal" -reset -po');

% Now complex modes
RES=struct('res',[],'po',dr.data(ind,:),'idopt',idopt('new'));
RES.idopt.residual=2;RES.idopt.fitting='complex';
for j1=1:length(ind); % deal with flexible modes
    Rj=(Sens.cta*dr.def(:,ind(j1))) * ... % c psi
        (dr.def(:,ind(j1)).'*Load.def); % psi^T b
    RES.res(j1,:)=Rj(:).';
end

% Rigid body mode residual
RES.res(end+1,:)=0;
for j1=1:6;
    Rj=(Sens.cta*def.def(:,j1))*(def.def(:,j1)'+Load.def);
    RES.res(end,:)=RES.res(end,:)+Rj(:).';
end
```

```

res2xf(RES,f,'hz iipplot "Res2xf"');

damp=dr.data(ind,2);
d2=def;d2.data(7:20)=sqrt(real(d2.data(7:20).^2)).*sqrt(1+i*damp*2);
nor2xf(d2,Up,f,'hz iipplot "Hysteretic"');
iicom('submagpha');

```

Note that the presence of rigid body modes, which can only be represented as residual terms in the pole/residue format (see section 5.6), makes the example more complex. The plot illustrates differences in responses obtained with true complex modes, viscous modal damping or hysteretic modal damping (case where one uses the pole of the true complex mode with a normal mode shape). Viscous and hysteretic modal damping are nearly identical. With true complex modes, only channels 2 and 4 show a visible difference, and then only near anti-resonances.

To incorporate static corrections, you may want to compute complex modes on bases generated by `fe2ss`, rather than simple modal bases obtained with `fe.eig`.

The use of a constant loss factor can be a crude approximation for materials exhibiting significant damping. Methods used to treat frequency dependent materials are described in Ref. [30].

5.4 State space models

While normal mode models are appropriate for structures, **state-space models** allow the representation of more general linear dynamic systems and are commonly used in the *Control Toolbox* or SIMULINK. The standard form for state space-models is

$$\begin{aligned}\{\dot{x}\} &= [A] \{x(t)\} + [B] \{u(t)\} \\ \{y\} &= [C] \{x(t)\} + [D] \{u(t)\}\end{aligned}\tag{5.8}$$

with inputs $\{u\}$, states $\{x\}$ and outputs $\{y\}$. State-space models are represented in the *SDT*, as generally done in other Toolboxes for use with MATLAB, using four independent matrix variables **a**, **b**, **c**, and **d** (you should also take a look at the LTI state-space object of the *Control Toolbox*).

The natural state-space representation of normal mode models (5.4) is given by

$$\begin{aligned} \begin{Bmatrix} \dot{p} \\ \dot{p} \end{Bmatrix} &= \begin{bmatrix} 0 & I \\ -\Omega^2 & -\Gamma \end{bmatrix} \begin{Bmatrix} p \\ \dot{p} \end{Bmatrix} + \begin{bmatrix} 0 \\ \phi^T b \end{bmatrix} \{u(t)\} \\ \{y(t)\} &= [c\phi \ 0] \begin{Bmatrix} p \\ \dot{p} \end{Bmatrix} \end{aligned} \quad (5.9)$$

Transformations to this form are provided by [nor2ss](#) and [fe2ss](#). Another special form of state-space models is constructed by [res2ss](#).

A state-space representation of the nominal structural model (5.1) is given by

$$\begin{aligned} \begin{Bmatrix} \dot{q} \\ \ddot{q} \end{Bmatrix} &= \begin{bmatrix} 0 & I \\ -M^{-1}K & -M^{-1}C \end{bmatrix} \begin{Bmatrix} q \\ \dot{q} \end{Bmatrix} + \begin{bmatrix} 0 \\ M^{-1}b \end{bmatrix} \{u(t)\} \\ \{y(t)\} &= [c \ 0] \begin{Bmatrix} q \\ \dot{q} \end{Bmatrix} \end{aligned} \quad (5.10)$$

The interest of this representation is mostly academic because it does not preserve symmetry (an useful feature of models of structures associated to the assumption of reciprocity) and because $M^{-1}K$ is usually a full matrix (so that the associated memory requirements for a realistic finite element model would be prohibitive). The *SDT* thus always starts by transforming a model to the normal mode form and the associated state-space model (5.9).

The transfer functions from inputs to outputs are described in the frequency domain by

$$\{y(s)\} = \left([C] [s I - A]^{-1} [B] + [D] \right) \{u(s)\} \quad (5.11)$$

assuming that $[A]$ is diagonalizable in the basis of **complex modes**, model (5.8) is equivalent to the diagonal model

$$\begin{aligned} (s [I] - [\lambda_j]) \{\eta(s)\} &= [\theta_L^T b] \{u\} \\ \{y\} &= [c\theta_R] \{\eta(s)\} \end{aligned} \quad (5.12)$$

where the left and right modeshapes (columns of $[\theta_R]$ and $[\theta_L]$) are solution of

$$\{\theta_{jL}\}^T [A] = \lambda_j \{\theta_{jL}\}^T \quad \text{and} \quad [A] \{\theta_{jR}\} = \lambda_j \{\theta_{jR}\} \quad (5.13)$$

and verify the orthogonality conditions

$$[\theta_L]^T [\theta_R] = [I] \quad \text{and} \quad [\theta_L]^T [A] [\theta_R] = [\lambda_j] \quad (5.14)$$

The diagonal state space form corresponds to the partial fraction expansion

$$\{y(s)\} = \sum_{j=1}^{2N} \frac{\{c\psi_j\} \{\psi_j^T b\}}{s - \lambda_j} = \sum_{j=1}^{2N} \frac{[R_j]_{NS \times NA}}{s - \lambda_j} \quad (5.15)$$

where the contribution of each mode is characterized by the pole location λ_j and the residue matrix R_j (which is equal to the product of the complex modal output

$\{c\theta_j\}$ by the modal input $\{\theta_j^T b\}$).

The partial fraction expansion (5.15) is heavily used for the identification routines implemented in the *SDT* (see the section on the pole/residue representation ref page 165).

5.5 Complex mode models

The standard spectral decomposition discussed for state-space models in the previous section can be applied directly to second order models of structural dynamics. The associated modes are called **complex modes** by opposition to **normal modes** which are associated to elastic models of structures and are always real valued.

Left and right eigenvectors, which are equal for reciprocal structural models, can be defined by the second order eigenvalue problem,

$$\left[M\lambda_j^2 + C\lambda_j + K \right] \{\psi_j\} = \{0\} \quad (5.16)$$

In practice however, mathematical libraries only provide first order eigenvalue solvers to that a transformation to the first order form is needed. Rather than the trivial state-space form (5.10), the following generalized state-space form is preferred

$$\begin{aligned} \begin{bmatrix} C & M \\ M & 0 \end{bmatrix} \begin{Bmatrix} \dot{q} \\ \ddot{q} \end{Bmatrix} + \begin{bmatrix} K & 0 \\ 0 & -M \end{bmatrix} \begin{Bmatrix} q \\ \dot{q} \end{Bmatrix} &= \begin{bmatrix} b \\ 0 \end{bmatrix} \{u\} \\ \{y\} &= \begin{bmatrix} c & 0 \end{bmatrix} \begin{Bmatrix} q \\ \dot{q} \end{Bmatrix} \end{aligned} \quad (5.17)$$

The matrices M , C and K being symmetric (assumption of reciprocity), the generalized state-space model (5.17) is symmetric. The associate left and right eigenvectors are thus equal and found by solving

$$\left(\begin{bmatrix} C & M \\ M & 0 \end{bmatrix} \lambda_j + \begin{bmatrix} K & 0 \\ 0 & -M \end{bmatrix} \right) \{\theta_j\} = \{0\} \quad (5.18)$$

Because of the specific block from of the problem, it can be shown that

$$\{\theta_j\} = \begin{Bmatrix} \psi_j \\ \psi_j \lambda_j \end{Bmatrix} \quad (5.19)$$

where it should be noted that the name complex modeshape is given to both θ_j (for applications in system dynamics) and ψ_j (for applications in structural dynamics).

The initial model being real, complex eigenvalues λ_j come in conjugate pairs associated to conjugate pairs of modeshapes $\{\psi_j\}$. With the exception of systems with

real poles, there are $2N$ complex eigenvalues for the considered symmetric systems ($\psi_{[N+1..2N]} = \bar{\psi}_{[1..N]}$ and $\lambda_{[N+1..2N]} = \bar{\lambda}_{[1..N]}$).

The existence of a set of $2N$ eigenvectors is equivalent to the verification of two orthogonality conditions

$$\begin{aligned} [\theta]^T \begin{bmatrix} C & M \\ M & 0 \end{bmatrix} [\theta] &= \psi^T C \psi + \Lambda \psi^T M \psi + \psi^T M \psi \Lambda &= [\backslash I \backslash]_{2N} \\ [\theta]^T \begin{bmatrix} K & 0 \\ 0 & -M \end{bmatrix} [\theta] &= \psi^T K \psi - \Lambda \psi^T M \psi \Lambda &= -[\backslash \Lambda \backslash]_{2N} \end{aligned} \quad (5.20)$$

where in (5.20) the arbitrary diagonal matrix was chosen to be the identity because it leads to a normalization of complex modes that is equivalent to the collocation constraint used to scale experimentally determined modeshapes ([12] and section 2.4.2).

Note that with hysteretic damping (complex valued stiffness, see section 5.3.2) the modes are not complex conjugate but opposite. To use a complex mode basis one thus needs to replace complex modes whose poles have negative imaginary parts with the conjugate of the corresponding mode whose pole has a positive imaginary part.

For a particular dynamic system, one will only be interested in predicting or measuring how complex modes are excited (modal input shape matrix $\{\theta_j^T B\} = \{\psi_j^T b\}$) or observed (modal output shape matrix $\{C \theta_j\} = \{c \psi_j\}$).

In the structural dynamics community, the **modal input shape matrix** is often called **modal participation factor** (and noted L_j) and the modal output shape matrix simply **modeshape**. A different terminology is preferred here to convey the fact that both notions are dual and that $\{\psi_j^T b_l\} = \{c_l \psi_j\}$ for a reciprocal structure and a collocated pair of inputs and outputs (such that $u \dot{y}$ is the power input to the structure).

For predictions, complex modes can be computed from finite element models using `fe_ceig`. Computing complex modes of full order models is not necessary. You should thus reduce the model on a basis of real vectors (as discussed in [29] and illustrated in section 5.3.2) before calling `fe_ceig` (if you really want complex modes).

For identification phases, complex modes are used in the form of residue matrices product $[R_j] = \{c \psi_j\} \{\psi_j^T b\}$ (see the next section). Modal residues are obtained by `id_rc` and separation of the modal input and output parts is obtained using `id_rm`.

For lightly damped structures, imposing the modal damping assumption, which forces the use of real modeshapes, may give correct result and simplify your identification work very much. Refer to section 2.4.3 for more details.

5.6 Pole/residue models

The spectral decomposition associated to complex modes, leads to a representation of the transfer function as a sum of modal contributions

$$[\alpha(s)] = \sum_{j=1}^{2N} \left(\frac{\{c\psi_j\} \{\psi_j^T b\}}{s - \lambda_j} \right) = \sum_{j=1}^{2N} \left(\frac{[R_j]}{s - \lambda_j} \right) \quad (5.21)$$

For applications in identification from experimental data, one can only determine modes whose poles are located in the test frequency range. The full series thus need to be truncated. The contributions of out-of-band modes cannot be neglected for applications in structural dynamics. One thus introduces a high frequency residual correction for truncated high frequency terms and, when needed, (quite often for suspended test articles) a low frequency residual for modes below the measurement frequency band.

These corrections depend on the type of transfer function so that the *SDT* uses `ci.IDopt` options (see the reference section on the `idopt` function) to define the current type. `ci.IDopt.Residual` specifies which corrections are needed (the default is 3 which includes both a low and high frequency residuals). `ci.IDopt.Data` specifies if the FRF is force to displacement, velocity or acceleration. For a force to displacement transfer function with low and high frequency correction), the **pole/residue model** (also called partial fraction expansion) thus takes the form

$$[\alpha(s)] = \sum_{j \in \text{identified}} \left(\frac{[R_j]}{s - \lambda_j} + \frac{[\bar{R}_j]}{s - \bar{\lambda}_j} \right) + [E] + \frac{[F]}{s^2} \quad (5.22)$$

The *SDT* always stores pole/residue models in the displacement/force format. The expression of the force to acceleration transfer function is thus

$$[A(s)] = \sum_{j \in \text{identified}} \left(\frac{s^2 [R_j]}{s - \lambda_j} + \frac{s^2 [\bar{R}_j]}{s - \bar{\lambda}_j} \right) + s^2 [E] + [F] \quad (5.23)$$

The **nominal** pole/residue model above is used when `ci.IDopt.Fit='Complex'`. This model assumes that complex poles come in conjugate pairs and that the residue matrices are also conjugate which is true for real system.

The **complex residues with asymmetric pole structure** (`ci.IDopt.Fit='Posit'`) only keep the poles with positive imaginary parts

$$[\alpha(s)] = \sum_{j \in \text{identified}} \left(\frac{[R_j]}{s - \lambda_j} \right) + [E] + \frac{[F]}{s^2} \quad (5.24)$$

which allows slightly faster computations when using `id.rc` for the identification but not so much so that the symmetric pole pattern should not be used in general. This

option is only maintained for backward compatibility reasons.

The **normal mode residues with symmetric pole structure** (`ci.IDopt.Fit='Nor'`)

$$[\alpha(s)] = \sum_{j \in \text{identified}} \left(\frac{[T_j]}{s^2 + 2\zeta_j \omega_j s + \omega_j^2} \right) + [E] + \frac{[F]}{s^2} \quad (5.25)$$

can be used to identify normal modes directly under the assumption of modal damping (see `damp` page 156).

Further characterization of the properties of a given pole/residue model is given by a structure detailed under the `xfopt Shapes at DOF` section.

The residue matrices `res` are stored using one row for each pole or asymptotic correction term and, as for FRFs (see the `xf` format), a column for each SISO transfer function (stacking NS columns for actuator 1, then NS columns for actuator 2, etc.).

$$\text{res} = \begin{bmatrix} \vdots & & \dots & \dots & & \dots \\ R_{j(11)} & R_{j(21)} & \dots & R_{j(12)} & R_{j(22)} & \dots \\ \vdots & & \ddots & \vdots & & \ddots \\ E_{11} & E_{21} & \dots & E_{12} & E_{22} & \dots \\ F_{11} & F_{21} & \dots & F_{12} & F_{22} & \dots \end{bmatrix} \quad (5.26)$$

The normal mode residues (`ci.IDopt.Fit='Normal'`) are stored in a similar fashion with for only difference that the T_j are real while the R_j are complex.

5.7 Parametric transfer function

Except for the `id_poly` and `qbode` functions, the *SDT* does not typically use the numerous variants of the ARMAX model that are traditional in system identification applications and lead to the ratio of polynomials called transfer function format (`tf`) in other MATLAB *Toolboxes*. In modal analysis, transfer functions refer to the functions characterizing the relation between inputs and outputs. The `tf` format thus corresponds to the parametric representations of sets of transfer functions in the form of a ratio of polynomials

$$H_j(s) = \frac{a_{j,1}s^{na-1} + a_{j,2}s^{na-2} + \dots + a_{j,na}}{b_{j,1}s^{nb-1} + b_{j,2}s^{nb-2} + \dots + b_{j,nb}} \quad (5.27)$$

The *SDT* stacks the different numerator and denominator polynomials as rows of

numerator and denominator matrices

$$\text{num} = \begin{bmatrix} a_{11} & a_{12} & \dots \\ a_{21} & a_{22} & \dots \\ \vdots & & \ddots \end{bmatrix} \text{ and } \text{den} = \begin{bmatrix} b_{11} & b_{12} & \dots \\ b_{21} & b_{22} & \dots \\ \vdots & & \ddots \end{bmatrix} \quad (5.28)$$

Other MATLAB toolboxes typically only accept a single common denominator (`den` is a single row). This form is also accepted by `qbode` which is used to predict FRFs at a number of frequencies in the non-parametric `xf` format).

The `id_poly` function identifies polynomial representations of sets of test functions and `res2tf` provides a transformation between the pole/residue and polynomial representations of transfer functions.

5.8 Non-parametric transfer function

`Response data` structures are the classical format to store non-parametric transfer functions. `Multi-dim curve` can also be used.

For a linear system at a given frequency ω , the response vector $\{y\}$ at NS sensor locations to a vector $\{u\}$ of NA inputs is described by the NS by NA rectangular matrix of Frequency Responses (FRF)

$$\begin{Bmatrix} y_1(\omega) \\ \vdots \\ y_{NS}(\omega) \end{Bmatrix} = [H] \{u\} = \begin{bmatrix} H_{11}(\omega) & H_{12}(\omega) & \dots \\ H_{21}(\omega) & H_{22}(\omega) & \\ \vdots & & \ddots \end{bmatrix}_{NS \times NA} \begin{Bmatrix} u_1(\omega) \\ \vdots \\ u_{NA}(\omega) \end{Bmatrix} \quad (5.29)$$

The `SDT` stores frequencies at which the FRF are evaluated as a column vector \mathbf{w}

$$\mathbf{w} = \begin{Bmatrix} \omega_1 \\ \vdots \\ \omega_{NW} \end{Bmatrix}_{NW \times 1} \quad (5.30)$$

and SISO FRFs H_{ij} are stored as columns of the matrix `xf` where each row corresponds to a different frequency (indicated in \mathbf{w}). By default, it is assumed that the correspondence between the columns of `xf` and the sensors and actuator numbers is as follows. The NS transfer functions from actuator 1 to the NS sensors are stored as the first NS columns of `xf`, then the NS transfer functions of actuator 2, etc.

$$\mathbf{xf} = \begin{bmatrix} H_{11}(\omega_1) & H_{21}(\omega_1) & \dots & H_{12}(\omega_1) & H_{22}(\omega_1) & \dots \\ H_{11}(\omega_2) & H_{21}(\omega_2) & \dots & H_{12}(\omega_2) & H_{22}(\omega_2) & \dots \\ \vdots & & \ddots & \vdots & & \ddots \end{bmatrix}_{NW \times (NS \times NA)} \quad (5.31)$$

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Further characterization of the properties of a given set of FRFs is given by a structure detailed under [Response data](#) section.

Frequency response functions corresponding to parametric models can be generated in the `xf` format using `qbode` (transformation from `ss` and `tf` formats), `nor2xf`, or `res2xf`. These functions use robustness/speed trade-offs that are different from algorithms implemented in other MATLAB toolboxes and are more appropriate for applications in structural dynamics.

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6.1 FEM problem formulations

This section gives a short theoretical reminder of supported FEM problems. The selection of the formulation for each element group is done through the material and element properties.

6.1.1 3D elasticity

Elements with a `p_solid` property entry with a non-zero integration rule are described under `p_solid`. They correspond exactly to the `*b` elements, which are now obsolete. These elements support 3D mechanics (DOFs `.01` to `.03` at each node) with full anisotropy, geometric non-linearity, integration rule selection, ... The elements have standard limitations. In particular they do not (yet)

- have any correction for shear locking found for high aspect ratios
- have any correction for dilatation locking found for nearly incompressible materials

With `m_elastic` subtypes 1 and 3, `p_solid` deals with 3D mechanics with strain defined by

$$\begin{pmatrix} \epsilon_x \\ \epsilon_y \\ \epsilon_z \\ \gamma_{yz} \\ \gamma_{zx} \\ \gamma_{xy} \end{pmatrix} = \begin{bmatrix} N, x & 0 & 0 \\ 0 & N, y & 0 \\ 0 & 0 & N, z \\ 0 & N, z & N, y \\ N, z & 0 & N, x \\ N, y & N, x & 0 \end{bmatrix} \begin{pmatrix} u \\ v \\ w \end{pmatrix} \quad (6.1)$$

where the engineering notation $\gamma_{yz} = 2\epsilon_{yz}$, ... is used. Stress by

$$\begin{pmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \sigma_{yz} \\ \sigma_{zx} \\ \sigma_{xy} \end{pmatrix} = \begin{bmatrix} d_{1,1}N, x+d_{1,5}N, z+d_{1,6}N, y & d_{1,2}N, y+d_{1,4}N, z+d_{1,6}N, x & d_{1,3}N, z+d_{1,4}N, y+d_{1,5}N, x \\ d_{2,1}N, x+d_{2,5}N, z+d_{2,6}N, y & d_{2,2}N, y+d_{2,4}N, z+d_{2,6}N, x & d_{2,3}N, z+d_{2,4}N, y+d_{2,5}N, x \\ d_{3,1}N, x+d_{3,5}N, z+d_{3,6}N, y & d_{3,2}N, y+d_{3,4}N, z+d_{3,6}N, x & d_{3,3}N, z+d_{3,4}N, y+d_{3,5}N, x \\ d_{4,1}N, x+d_{4,5}N, z+d_{4,6}N, y & d_{4,2}N, y+d_{4,4}N, z+d_{4,6}N, x & d_{4,3}N, z+d_{4,4}N, y+d_{4,5}N, x \\ d_{5,1}N, x+d_{5,5}N, z+d_{5,6}N, y & d_{5,2}N, y+d_{5,4}N, z+d_{5,6}N, x & d_{5,3}N, z+d_{5,4}N, y+d_{5,5}N, x \\ d_{6,1}N, x+d_{6,5}N, z+d_{6,6}N, y & d_{6,2}N, y+d_{6,4}N, z+d_{6,6}N, x & d_{6,3}N, z+d_{6,4}N, y+d_{6,5}N, x \end{bmatrix} \begin{pmatrix} u \\ v \\ w \end{pmatrix}$$

Note that the strain states are $\{\epsilon_x \ \epsilon_y \ \epsilon_z \ \gamma_{yz} \ \gamma_{zx} \ \gamma_{xy}\}$ which may not be the convention of other software. In particular volume elements inherited from MODULEF order shear stresses differently $\sigma_{xy}, \sigma_{yz}, \sigma_{zx}$ (these elements are obtained by setting `p_solid integ` value to zero. In `fe_stress` the stress reordering can be accounted for by the definition of the proper `TensorTopology` matrix.

For isotropic materials

$$D = \begin{bmatrix} \frac{E(1-\nu)}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1 & \frac{\nu}{1-\nu} & \frac{\nu}{1-\nu} \\ \frac{\nu}{1-\nu} & 1 & \frac{\nu}{1-\nu} \\ \frac{\nu}{1-\nu} & \frac{\nu}{1-\nu} & 1 \end{bmatrix} & & 0 \\ & 0 & \begin{bmatrix} G & 0 & 0 \\ 0 & G & 0 \\ 0 & 0 & G \end{bmatrix} \end{bmatrix} \quad (6.2)$$

with at nominal $G = E/(2(1 + \nu))$.

For orthotropic materials, the compliance is given by

$$\{\epsilon\} = [D]^{-1} \{\sigma\} = \begin{bmatrix} 1/E_1 & -\frac{\nu_{21}}{E_2} & -\frac{\nu_{31}}{E_3} & 0 & 0 & 0 \\ -\frac{\nu_{12}}{E_1} & 1/E_2 & -\frac{\nu_{32}}{E_3} & 0 & 0 & 0 \\ -\frac{\nu_{13}}{E_1} & -\frac{\nu_{23}}{E_2} & 1/E_3 & 0 & 0 & 0 \\ 0 & 0 & 0 & \frac{1}{G_{23}} & 0 & 0 \\ 0 & 0 & 0 & 0 & \frac{1}{G_{31}} & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{1}{G_{12}} \end{bmatrix} \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \sigma_{yz} \\ \sigma_{zx} \\ \sigma_{xy} \end{Bmatrix} \quad (6.3)$$

For constitutive law building, see [p_solid](#).

6.1.2 2D elasticity

With [m_elastic](#) subtype 4, [p_solid](#) deals with 2D mechanical volumes with strain defined by (see [q4p constants](#))

$$\begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} = \begin{bmatrix} N, x & 0 \\ 0 & N, y \\ N, y & N, x \end{bmatrix} \begin{Bmatrix} u \\ v \end{Bmatrix} \quad (6.4)$$

and stress by

$$\begin{Bmatrix} \sigma \epsilon_x \\ \sigma \epsilon_y \\ \sigma \gamma_{xy} \end{Bmatrix} = \begin{bmatrix} d_{1,1}N, x + d_{1,3}N, y & d_{1,2}N, y + d_{1,3}N, x \\ d_{2,1}N, x + d_{2,3}N, y & d_{2,2}N, y + d_{2,3}N, x \\ d_{3,1}N, x + d_{3,3}N, y & d_{3,2}N, y + d_{3,3}N, x \end{bmatrix} \begin{Bmatrix} u \\ v \end{Bmatrix} \quad (6.5)$$

For isotropic plane stress ([p_solid form=1](#)), one has

$$D = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \quad (6.6)$$

For isotropic plane strain (`p_solid form=0`), one has

$$D = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1 & \frac{\nu}{1-\nu} & 0 \\ \frac{\nu}{1-\nu} & 1 & 0 \\ 0 & 0 & \frac{1-2\nu}{2(1-\nu)} \end{bmatrix} \quad (6.7)$$

6.1.3 Acoustics

With `m_elastic` subtype 2, `p_solid` deals with 2D and 3D acoustics (see `flui4 constants`) where 3D strain is given by

$$\begin{Bmatrix} p, x \\ p, y \\ p, z \end{Bmatrix} = \begin{bmatrix} N, x \\ N, y \\ N, z \end{bmatrix} \{ p \} \quad (6.8)$$

This replaces the earlier `flui4` ... elements.

6.1.4 Classical lamination theory

Both isotropic and orthotropic materials are considered. In these cases, the general form of the 3D elastic material law is

$$\begin{Bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{33} \\ \tau_{23} \\ \tau_{13} \\ \tau_{12} \end{Bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & 0 & 0 & 0 \\ & C_{22} & C_{23} & 0 & 0 & 0 \\ & & C_{33} & 0 & 0 & 0 \\ & & & C_{44} & 0 & 0 \\ & (s) & & & C_{55} & 0 \\ & & & & & C_{66} \end{bmatrix} \begin{Bmatrix} \epsilon_{11} \\ \epsilon_{22} \\ \epsilon_{33} \\ \gamma_{23} \\ \gamma_{13} \\ \gamma_{12} \end{Bmatrix} \quad (6.9)$$

Plate formulation consists in assuming one dimension, the thickness along x_3 , negligible compared with the surface dimensions. Thus, vertical stress $\sigma_{33} = 0$ on the bottom and upper faces, and assumed to be neglected throughout the thickness,

$$\sigma_{33} = 0 \Rightarrow \epsilon_{33} = -\frac{1}{C_{33}} (C_{13}\epsilon_{11} + C_{23}\epsilon_{22}), \quad (6.10)$$

and for isotropic material,

$$\sigma_{33} = 0 \Rightarrow \epsilon_{33} = -\frac{\nu}{1-\nu} (\epsilon_{11} + \epsilon_{22}). \quad (6.11)$$

By eliminating σ_{33} , the plate constitutive law is written, with engineering notations,

$$\begin{Bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \\ \sigma_{23} \\ \sigma_{13} \end{Bmatrix} = \begin{bmatrix} Q_{11} & Q_{12} & 0 & 0 & 0 \\ Q_{12} & Q_{22} & 0 & 0 & 0 \\ 0 & 0 & Q_{66} & 0 & 0 \\ 0 & 0 & 0 & Q_{44} & 0 \\ 0 & 0 & 0 & 0 & Q_{55} \end{bmatrix} \begin{Bmatrix} \epsilon_{11} \\ \epsilon_{22} \\ \gamma_{12} \\ \gamma_{23} \\ \gamma_{13} \end{Bmatrix}. \quad (6.12)$$

The reduced stiffness coefficients Q_{ij} ($i,j = 1,2,4,5,6$) are related to the 3D stiffness coefficients C_{ij} by

$$Q_{ij} = \begin{cases} C_{ij} - \frac{C_{i3}C_{j3}}{C_{33}} & \text{if } i,j=1,2, \\ C_{ij} & \text{if } i,j=4,5,6. \end{cases} \quad (6.13)$$

The reduced elastic law for an isotropic plate becomes,

$$\begin{Bmatrix} \sigma_{11} \\ \sigma_{22} \\ \tau_{12} \end{Bmatrix} = \frac{E}{(1-\nu^2)} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} \epsilon_{11} \\ \epsilon_{22} \\ \gamma_{12} \end{Bmatrix}, \quad (6.14)$$

and

$$\begin{Bmatrix} \tau_{23} \\ \tau_{13} \end{Bmatrix} = \frac{E}{2(1+\nu)} \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix} \begin{Bmatrix} \gamma_{23} \\ \gamma_{13} \end{Bmatrix}. \quad (6.15)$$

Under Reissner-Mindlin's kinematic assumption the linearized strain tensor is

$$\epsilon = \begin{bmatrix} u_{1,1} + x_3\beta_{1,1} & \frac{1}{2}(u_{1,2} + u_{2,1} + x_3(\beta_{1,2} + \beta_{2,1})) & \frac{1}{2}(\beta_1 + w_{,1}) \\ & u_{2,2} + x_3\beta_{2,2} & \frac{1}{2}(\beta_2 + w_{,2}) \\ (s) & & 0 \end{bmatrix}. \quad (6.16)$$

So, the strain vector is written,

$$\{\epsilon\} = \begin{Bmatrix} \epsilon_{11}^m + x_3 \kappa_{11} \\ \epsilon_{22}^m + x_3 \kappa_{22} \\ \gamma_{12}^m + x_3 \kappa_{12} \\ \gamma_{23} \\ \gamma_{13} \end{Bmatrix}, \quad (6.17)$$

with ϵ^m the membrane, κ the curvature or bending, and γ the shear strains,

$$\epsilon^m = \begin{Bmatrix} u_{1,1} \\ u_{2,2} \\ u_{1,2} + u_{2,1} \end{Bmatrix}, \quad \kappa = \begin{Bmatrix} \beta_{1,1} \\ \beta_{2,2} \\ \beta_{1,2} + \beta_{2,1} \end{Bmatrix}, \quad \gamma = \begin{Bmatrix} \beta_2 + w_{,2} \\ \beta_1 + w_{,1} \end{Bmatrix}, \quad (6.18)$$

Note that the engineering notation with $\gamma_{12} = u_{1,2} + u_{2,1}$ is used here rather than the tensor notation with $\epsilon_{12} = (u_{1,2} + u_{2,1})/2$. Similarly $\kappa_{12} = \beta_{1,2} + \beta_{2,1}$, where a factor 1/2 would be needed for the tensor.

The plate formulation links the stress resultants, membrane forces $N_{\alpha\beta}$, bending moments $M_{\alpha\beta}$ and shear forces $Q_{\alpha 3}$, to the strains, membrane ϵ^m , bending κ and shearing γ ,

$$\begin{Bmatrix} N \\ M \\ Q \end{Bmatrix} = \begin{bmatrix} A & B & 0 \\ B & D & 0 \\ 0 & 0 & F \end{bmatrix} \begin{Bmatrix} \epsilon^m \\ \kappa \\ \gamma \end{Bmatrix}. \quad (6.19)$$

The stress resultants are obtained by integrating the stresses through the thickness of the plate,

$$N_{\alpha\beta} = \int_{hb}^{ht} \sigma_{\alpha\beta} dx_3, \quad M_{\alpha\beta} = \int_{hb}^{ht} x_3 \sigma_{\alpha\beta} dx_3, \quad Q_{\alpha 3} = \int_{hb}^{ht} \sigma_{\alpha 3} dx_3, \quad (6.20)$$

with $\alpha, \beta = 1, 2$.

Therefore, the matrix extensional stiffness matrix $[A]$, extension/bending coupling matrix $[B]$, and the bending stiffness matrix $[D]$ are calculated by integration over the thickness interval $[hb \ ht]$

$$\begin{aligned} A_{ij} &= \int_{hb}^{ht} Q_{ij} dx_3, & B_{ij} &= \int_{hb}^{ht} x_3 Q_{ij} dx_3, \\ D_{ij} &= \int_{hb}^{ht} x_3^2 Q_{ij} dx_3, & F_{ij} &= \int_{hb}^{ht} Q_{ij} dx_3. \end{aligned} \quad (6.21)$$

An improvement of Mindlin's plate theory with transverse shear consists in modifying

the shear coefficients F_{ij} by

$$H_{ij} = k_{ij}F_{ij}, \quad (6.22)$$

where k_{ij} are correction factors. Reddy's 3rd order theory brings to $k_{ij} = \frac{2}{3}$. Very commonly, enriched 3rd order theory are used, and k_{ij} are equal to $\frac{5}{6}$ and give good results. For more details on the assessment of the correction factor, see [31].

For an isotropic symmetric plate ($hb = -ht = h/2$), the in-plane normal forces N_{11} , N_{22} and shear force N_{12} become

$$\begin{Bmatrix} N_{11} \\ N_{22} \\ N_{12} \end{Bmatrix} = \frac{Eh}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ & 1 & 0 \\ (s) & & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} u_{1,1} \\ u_{2,2} \\ u_{1,2} + u_{2,1} \end{Bmatrix}, \quad (6.23)$$

the 2 bending moments M_{11} , M_{22} and twisting moment M_{12}

$$\begin{Bmatrix} M_{11} \\ M_{22} \\ M_{12} \end{Bmatrix} = \frac{Eh^3}{12(1-\nu^2)} \begin{bmatrix} 1 & \nu & 0 \\ & 1 & 0 \\ (s) & & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} \beta_{1,1} \\ \beta_{2,2} \\ \beta_{1,2} + \beta_{2,1} \end{Bmatrix}, \quad (6.24)$$

and the out-of-plane shearing forces Q_{23} and Q_{13} ,

$$\begin{Bmatrix} Q_{23} \\ Q_{13} \end{Bmatrix} = \frac{Eh}{2(1+\nu)} \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix} \begin{Bmatrix} \beta_2 + w_{,2} \\ \beta_1 + w_{,1} \end{Bmatrix}. \quad (6.25)$$

One can notice that because the symmetry of plate, that means the reference plane is the mid-plane of the plate ($x_3(0) = 0$) the extension/bending coupling matrix $[B]$ is equal to zero.

Using expression (6.21) for a constant Q_{ij} , one sees that for a non-zero offset, one has

$$A_{ij} = h[Q_{ij}] \quad B_{ij} = x_3(0)h[Q_{ij}] \quad C_{ij} = (x_3(0)^2h + h^3/12)[Q_{ij}] \quad F_{ij} = h[Q_{ij}] \quad (6.26)$$

where it clearly appears that the constitutive matrix is a polynomial function of h , h^3 , $x_3(0)^2h$ and $x_3(0)h$. If the ply thickness is kept constant, the constitutive law is a polynomial function of $1, x_3(0), x_3(0)^2$.

6.1.5 Piezo-electric volumes

The strain state associated with piezoelectric materials is described by the six classical mechanical strain components and the electrical field components. Following the IEEE standards on piezoelectricity and using matrix notations, S denotes the strain vector and E denotes the electric field vector (V/m) :

$$\left\{ \begin{array}{c} S \\ E \end{array} \right\} = \left\{ \begin{array}{c} \epsilon_x \\ \epsilon_y \\ \epsilon_z \\ \gamma_{yz} \\ \gamma_{zx} \\ \gamma_{xy} \\ E_x \\ E_y \\ E_z \end{array} \right\} = \left[\begin{array}{cccc} N, x & 0 & 0 & 0 \\ 0 & N, y & 0 & 0 \\ 0 & 0 & N, z & 0 \\ 0 & N, z & N, y & 0 \\ N, z & 0 & N, x & 0 \\ N, y & N, x & 0 & 0 \\ 0 & 0 & 0 & -N, x \\ 0 & 0 & 0 & -N, y \\ 0 & 0 & 0 & -N, z \end{array} \right] \left\{ \begin{array}{c} u \\ v \\ w \\ \phi \end{array} \right\} \quad (6.27)$$

where ϕ is the electric potential (V).

The constitutive law associated with this strain state is given by

$$\left\{ \begin{array}{c} T \\ D \end{array} \right\} = \left[\begin{array}{cc} C^E & e^T \\ e & -\epsilon^S \end{array} \right] \left\{ \begin{array}{c} S \\ -E \end{array} \right\} \quad (6.28)$$

in which D is the electrical displacement vector (a density of charge in Cb/m^2), T is the mechanical stress vector (N/m^2). C^E is the matrix of elastic constants at zero electric field ($E = 0$, short-circuited condition, see section 6.1.1 for formulas (there C^E is noted D). Note that using $-E$ rather than E makes the constitutive law symmetric.

Alternatively, one can use the constitutive equations written in the following manner :

$$\left\{ \begin{array}{c} S \\ D \end{array} \right\} = \left[\begin{array}{cc} s^E & d^T \\ d & \epsilon^T \end{array} \right] \left\{ \begin{array}{c} T \\ E \end{array} \right\} \quad (6.29)$$

In which s^E is the matrix of mechanical compliances, $[d]$ is the matrix of piezoelectric constants ($m/V = Cb/N$):

$$[d] = \left[\begin{array}{cccccc} d_{11} & d_{12} & d_{13} & d_{14} & d_{15} & d_{16} \\ d_{21} & d_{22} & d_{23} & d_{24} & d_{25} & d_{26} \\ d_{31} & d_{32} & d_{33} & d_{34} & d_{35} & d_{36} \end{array} \right] \quad (6.30)$$

Matrices $[e]$ and $[d]$ are related through

$$[e] = [d] [C^E] \quad (6.31)$$

Due to crystal symmetries, $[d]$ may have only a few non-zero elements.

Matrix $[\epsilon^S]$ is the matrix of dielectric constants (permittivities) under zero strain

(constant volume) given by

$$[\varepsilon^S] = \begin{bmatrix} \varepsilon_{11}^S & \varepsilon_{12}^S & \varepsilon_{13}^S \\ \varepsilon_{21}^S & \varepsilon_{22}^S & \varepsilon_{23}^S \\ \varepsilon_{31}^S & \varepsilon_{32}^S & \varepsilon_{33}^S \end{bmatrix} \quad (6.32)$$

It is more usual to find the value of ε^T (Permittivity at zero stress) in the datasheet. These two values are related through the following relationship :

$$[\varepsilon^S] = [\varepsilon^T] - [d][e]^T \quad (6.33)$$

For this reason, the input value for the computation should be $[\varepsilon^T]$.

Also notice that usually relative permittivities are given in datasheets:

$$\varepsilon_r = \frac{\varepsilon}{\varepsilon_0} \quad (6.34)$$

ε_0 is the permittivity of vacuum (=8.854e-12 F/m)

The most widely used piezoelectric materials are PVDF and PZT. For both of these, matrix $[\varepsilon^T]$ takes the form

$$[\varepsilon^T] = \begin{bmatrix} \varepsilon_{11}^T & 0 & 0 \\ 0 & \varepsilon_{22}^T & 0 \\ 0 & 0 & \varepsilon_{33}^T \end{bmatrix} \quad (6.35)$$

For PVDF, the matrix of piezoelectric constants is given by

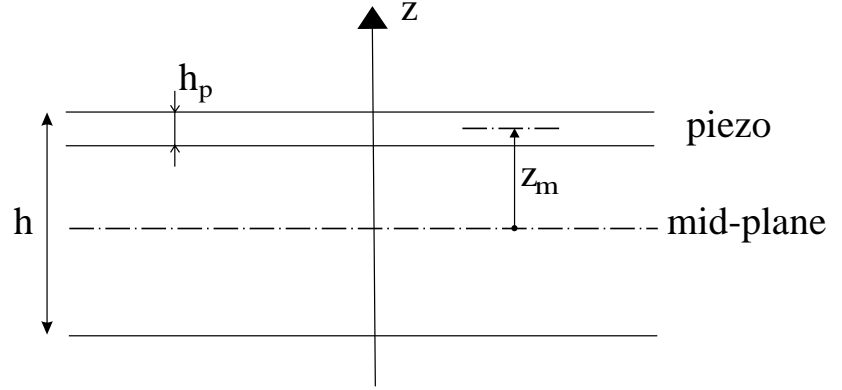
$$[d] = \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ d_{31} & d_{32} & d_{33} & 0 & 0 & 0 \end{bmatrix} \quad (6.36)$$

and for PZT materials :

$$[d] = \begin{bmatrix} 0 & 0 & 0 & 0 & d_{15} & 0 \\ 0 & 0 & 0 & d_{24} & 0 & 0 \\ d_{31} & d_{32} & d_{33} & 0 & 0 & 0 \end{bmatrix} \quad (6.37)$$

6.1.6 Piezo-electric shells

Shell strain is defined by the membrane, curvature and transverse shear as well as the electric field components. It is assumed that in each piezoelectric layer $i = 1 \dots n$, the electric field takes the form $\vec{E} = (0 \ 0 \ E_{zi})$. E_{zi} is assumed to be constant over the thickness h_i of the layer and is therefore given by $E_{zi} = -\frac{\Delta\phi_i}{h_i}$ where $\Delta\phi_i$ is the difference of potential between the electrodes at the top and bottom of the piezoelectric layer i . It is also assumed that the piezoelectric principal axes are parallel to the structural orthotropy axes.



The strain state of a piezoelectric shell takes the form

$$\left\{ \begin{array}{c} \epsilon_{xx} \\ \epsilon_{yy} \\ 2\epsilon_{xy} \\ \kappa_{xx} \\ \kappa_{yy} \\ 2\kappa_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \\ -E_{z1} \\ \dots \\ -E_{zn} \end{array} \right\} = \left[\begin{array}{cccccccc} N, x & 0 & 0 & 0 & 0 & 0 & \dots & 0 \\ 0 & N, y & 0 & 0 & 0 & 0 & \dots & 0 \\ N, y & N, x & 0 & 0 & 0 & 0 & \dots & 0 \\ 0 & 0 & 0 & 0 & -N, x & 0 & \dots & 0 \\ 0 & 0 & 0 & N, y & 0 & 0 & \dots & 0 \\ 0 & 0 & 0 & N, x & -N, y & 0 & \dots & 0 \\ 0 & 0 & N, x & 0 & N & 0 & \dots & 0 \\ 0 & 0 & N, y & -N & 0 & 0 & \dots & 0 \\ 0 & 0 & 0 & 0 & 0 & -\frac{1}{h_1} & \dots & 0 \\ \dots & \dots & \dots & \dots & \dots & 0 & \dots & -\frac{1}{h_n} \end{array} \right] \left\{ \begin{array}{c} u \\ v \\ w \\ ru \\ rw \\ \Delta\phi_1 \\ \dots \\ \Delta\phi_n \end{array} \right\} \quad (6.38)$$

There are thus n additional degrees of freedom $\Delta\phi_i$, n being the number of piezoelectric layers in the laminate shell

The constitutive law associated to this strain state is given by :

$$\begin{pmatrix} N \\ M \\ Q \\ D_{z1} \\ \dots \\ D_{zn} \end{pmatrix} = \begin{bmatrix} A & B & 0 & G_1^T & \dots & G_n^T \\ B & D & 0 & z_{m1}G_1^T & \dots & z_{mn}G_n^T \\ 0 & 0 & F & H_1^T & \dots & H_n^T \\ G_1 & z_{m1}G_1 & H_1 & -\varepsilon_1 & \dots & 0 \\ \dots & \dots & \dots & 0 & \dots & 0 \\ G_n & z_{mn}G_n & H_n & 0 & \dots & -\varepsilon_n \end{bmatrix} \begin{pmatrix} \epsilon \\ \kappa \\ \gamma \\ -E_{z1} \\ \dots \\ -E_{zn} \end{pmatrix} \quad (6.39)$$

where D_{zi} is the electric displacement in piezoelectric layer (assumed constant and in the z -direction), z_{mi} is the distance between the midplane of the shell and the midplane of piezoelectric layer i , and G_i, H_i are given by

$$G_i = \begin{Bmatrix} e_{.1} & e_{.2} & 0 \end{Bmatrix}_i [R_s]_i \quad (6.40)$$

$$H_i = \begin{Bmatrix} e_{.4} & e_{.5} \end{Bmatrix}_i [R]_i \quad (6.41)$$

where $.$ denotes the direction of polarization. If the piezoelectric is used in extension mode, the polarization is in the z -direction, therefore $H_i = 0$ and $G_i = \begin{Bmatrix} e_{31} & e_{32} & 0 \end{Bmatrix}_i$. If the piezoelectric is used in shear mode, the polarization is in the x or y -direction, therefore $G_i = 0$, and $H_i = \{0 \ e_{15}\}_i$ or $H_i = \{e_{24} \ 0\}_i$. It turns out however that the hypothesis of a uniform transverse shear strain distribution through the thickness is not satisfactory, a more elaborate shell element would be necessary. Shear actuation should therefore be used with caution.

$[R_s]_i$ and $[R]_i$ are rotation matrices associated to the angle θ of the piezoelectric layer.

$$[R_s] = \begin{bmatrix} \cos^2 \theta & \sin^2 \theta & \sin \theta \cos \theta \\ \sin^2 \theta & \cos^2 \theta & -\sin \theta \cos \theta \\ -2 \sin \theta \cos \theta & 2 \sin \theta \cos \theta & \cos^2 \theta - \sin^2 \theta \end{bmatrix} \quad (6.42)$$

$$[R] = \begin{bmatrix} \cos \theta & -\sin \theta \\ \sin \theta & \cos \theta \end{bmatrix} \quad (6.43)$$

6.1.7 Geometric non-linearity

The following gives the theory of large transformation problem implemented in OpenFEM function `of_mk_pre.c Mecha3DInteg`.

The principle of virtual work in non-linear total Lagrangian formulation for an hyperelastic medium is

$$\int_{\Omega_0} (\rho_0 u'', \delta v) + \int_{\Omega_0} S : \delta e = \int_{\Omega_0} f \cdot \delta v \quad \forall \delta v \quad (6.44)$$

with p the vector of initial position, $x = p + u$ the current position, and u the displacement vector. The transformation is characterized by

$$F_{i,j} = I + u_{i,j} = \delta_{ij} + \{N_{,j}\}^T \{q_i\} \quad (6.45)$$

where the $N_{,j}$ is the derivative of the shape functions with respect to cartesian coordinates at the current integration point and q_i corresponds to field i (here translations) and element nodes. The notation is thus really valid within a single element and corresponds to the actual implementation of the element family in `elem0` and `of_mk`. Note that in these functions, a reindexing vector is used to go from engineering ($\{e_{11} \ e_{22} \ e_{33} \ 2e_{23} \ 2e_{31} \ 2e_{12}\}$) to tensor $[e_{ij}]$ notations `ind_ts_eg=[1 6 5;6 2 4;5 4 3];e_tensor=e_engineering(ind_ts_eg);`. One can also simplify a number of computations using the fact that the contraction of a symmetric and non symmetric tensor is equal to the contraction of the symmetric tensor by the symmetric part of the non symmetric tensor.

One defines the Green-Lagrange strain tensor $e = 1/2(F^T F - I)$ and its variation

$$de_{ij} = \left(F^T dF \right)_{Sym} = \left(F_{ki} \{N_{,j}\}^T \{q_k\} \right)_{Sym} \quad (6.46)$$

Thus the virtual work of internal loads (which corresponds to the residual in non-linear iterations) is given by

$$\int_{\Omega} S : \delta e = \int_{\Omega} \{\delta q_k\}^T \{N_{,j}\} F_{ki} S_{ij} \quad (6.47)$$

and the tangent stiffness matrix (its derivative with respect to the current position) can be written as

$$K_G = \int_{\Omega} S_{ij} \delta u_{k,i} u_{l,j} + \int_{\Omega} de : \frac{\partial^2 W}{\partial e^2} : \delta e \quad (6.48)$$

which using the notation $u_{i,j} = \{N_{,j}\}^T \{q_i\}$ leads to

$$K_G^e = \int_{\Omega} \{\delta q_m\} \{N_{,l}\} \left(F_{mk} \frac{\partial^2 W}{\partial e^2}{}_{ijkl} F_{ni} + S_{lj} \right) \{N_{,j}\} \{dq_n\} \quad (6.49)$$

The term associated with stress at the current point is generally called geometric stiffness or pre-stress contribution.

In isotropic elasticity, the 2nd tensor of Piola-Kirchhoff stress is given by

$$S = D : e(u) = \frac{\partial^2 W}{\partial e^2} : e(u) = \lambda Tr(e)I + 2\mu e \quad (6.50)$$

the building of the constitutive law matrix D is performed in `p_solid BuildConstit` for isotropic, orthotropic and full anisotropic materials. `of_mk_pre.c nonlin_elas` then implements element level computations. For hyperelastic materials $\frac{\partial^2 W}{\partial e^2}$ is not

constant and is computed at each integration point as implemented in `hyper.c`.

For a geometric non-linear static computation, a Newton solver will thus iterate with

$$[K(q^n)] \{q^{n+1} - q^n\} = R(q^n) = \int_{\Omega} f \cdot dv - \int_{\Omega_0} S(q^n) : \delta e \quad (6.51)$$

where external forces f are assumed to be non following.

6.1.8 Thermal pre-stress

The following gives the theory of the thermoelastic problem implemented in Open-FEM function `of_mk_pre.c nonlin_elas`.

In presence of a temperature difference, the thermal strain is given by $[e_T] = [\alpha](T - T_0)$, where in general the thermal expansion matrix α is proportional to identity (isotropic expansion). The stress is found by computing the contribution of the mechanical deformation

$$S = C : (e - e_T) = \lambda Tr(e)I + 2\mu e - (C : [\alpha])(T - T_0) \quad (6.52)$$

This expression of the stress is then used in the equilibrium (6.44), the tangent matrix computation(6.48), or the Newton iteration (6.51). Note that the fixed contribution $\int_{\Omega_0} (-C : e_T) : \delta e$ can be considered as an internal load of thermal origin.

The modes of the heated structure can be computed with the tangent matrix.

An example of static thermal computation is given in `ofdemos ThermalCube`.

6.1.9 Hyperelasticity

The following gives the theory of the thermoelastic problem implemented in Open-FEM function `hyper.c` (called by `of_mk.c MatrixIntegration`).

For hyperelastic media $S = \partial W / \partial e$ with W the hyperelastic energy. `hyper.c` currently supports Mooney-Rivlin materials for which the energy takes one of following forms

$$W = C_1(J_1 - 3) + C_2(J_2 - 3) + K(J_3 - 1)^2, \quad (6.53)$$

$$W = C_1(J_1 - 3) + C_2(J_2 - 3) + K(J_3 - 1) - (C_1 + 2C_2 + K) \ln(J_3), \quad (6.54)$$

where (J_1, J_2, J_3) are the so-called reduced invariants of the Cauchy-Green tensor

$$C = I + 2e, \quad (6.55)$$

linked to the classical invariants (I_1, I_2, I_3) by

$$J_1 = I_1 I_3^{-\frac{1}{3}}, \quad J_2 = I_2 I_3^{-\frac{2}{3}}, \quad J_3 = I_3^{\frac{1}{2}}, \quad (6.56)$$

where one recalls that

$$I_1 = \text{tr}C, \quad I_2 = \frac{1}{2} \left[(\text{tr}C)^2 - \text{tr}C^2 \right], \quad I_3 = \det C. \quad (6.57)$$

Note : this definition of energy based on reduced invariants is used to have the hydrostatic pressure given directly by $p = -K(J_3 - 1)$ (K “bulk modulus”), and the third term of W is a penalty on incompressibility.

Hence, computing the corresponding tangent stiffness and residual operators will require the derivatives of the above invariants with respect to e (or C). In an orthonormal basis the first-order derivatives are given by:

$$\frac{\partial I_1}{\partial C_{ij}} = \delta_{ij}, \quad \frac{\partial I_2}{\partial C_{ij}} = I_1 \delta_{ij} - C_{ij}, \quad \frac{\partial I_3}{\partial C_{ij}} = I_3 C_{ij}^{-1}, \quad (6.58)$$

where (C_{ij}^{-1}) denotes the coefficients of the inverse matrix of (C_{ij}) . For second-order derivatives we have:

$$\frac{\partial^2 I_1}{\partial C_{ij} \partial C_{kl}} = 0, \quad \frac{\partial^2 I_2}{\partial C_{ij} \partial C_{kl}} = -\delta_{ik} \delta_{jl} + \delta_{ij} \delta_{kl}, \quad \frac{\partial^2 I_3}{\partial C_{ij} \partial C_{kl}} = C_{mn} \epsilon_{ikm} \epsilon_{jln}, \quad (6.59)$$

where the ϵ_{ijk} coefficients are defined by

$$\begin{cases} \epsilon_{ijk} = 0 & \text{when 2 indices coincide} \\ \epsilon_{ijk} = 1 & \text{when } (i, j, k) \text{ even permutation of } (1, 2, 3) \\ \epsilon_{ijk} = -1 & \text{when } (i, j, k) \text{ odd permutation of } (1, 2, 3) \end{cases} \quad (6.60)$$

Note: when the strain components are seen as a column vector (“engineering strains”) in the form $(e_{11}, e_{22}, e_{33}, 2e_{23}, 2e_{31}, 2e_{12})'$, the last two terms of (6.59) thus correspond to the following 2 matrices

$$\begin{pmatrix} 0 & 1 & 1 & 0 & 0 & 0 \\ 1 & 0 & 1 & 0 & 0 & 0 \\ 1 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & -1/2 & 0 & 0 \\ 0 & 0 & 0 & 0 & -1/2 & 0 \\ 0 & 0 & 0 & 0 & 0 & -1/2 \end{pmatrix}, \quad (6.61)$$

$$\begin{pmatrix} 0 & C_{33} & C_{22} & -C_{23} & 0 & 0 \\ C_{33} & 0 & C_{11} & 0 & -C_{13} & 0 \\ C_{22} & C_{11} & 0 & 0 & 0 & -C_{12} \\ -C_{23} & 0 & 0 & -C_{11}/2 & C_{12}/2 & C_{13}/2 \\ 0 & -C_{13} & 0 & C_{12}/2 & -C_{22}/2 & C_{23}/2 \\ 0 & 0 & -C_{12} & C_{13}/2 & C_{23}/2 & -C_{33}/2 \end{pmatrix}. \quad (6.62)$$

We finally use chain-rule differentiation to compute

$$S = \frac{\partial W}{\partial e} = \sum_k \frac{\partial W}{\partial I_k} \frac{\partial I_k}{\partial e}, \quad (6.63)$$

$$\frac{\partial^2 W}{\partial e^2} = \sum_k \frac{\partial W}{\partial I_k} \frac{\partial^2 I_k}{\partial e^2} + \sum_k \sum_l \frac{\partial^2 W}{\partial I_k \partial I_l} \frac{\partial I_k}{\partial e} \frac{\partial I_l}{\partial e}. \quad (6.64)$$

Note that a factor 2 arise each time we differentiate the invariants with respect to e instead of C .

The specification of a material is given by specification of the derivatives of the energy with respect to invariants. The laws are implemented in the `hyper.c EnPassiv` function.

6.1.10 Gyroscopic effects

Written by Arnaud Sternchuss ECP/MSSMat.

In the fixed reference frame which is galilean, the eulerian speed of the particle in \mathbf{x} whose initial position is \mathbf{p} is

$$\frac{\partial \mathbf{x}}{\partial t} = \frac{\partial \mathbf{u}}{\partial t} + \boldsymbol{\Omega} \wedge (\mathbf{p} + \mathbf{u})$$

and its acceleration is

$$\frac{\partial^2 \mathbf{x}}{\partial t^2} = \frac{\partial^2 \mathbf{u}}{\partial t^2} + \frac{\partial \boldsymbol{\Omega}}{\partial t} \wedge (\mathbf{p} + \mathbf{u}) + 2\boldsymbol{\Omega} \wedge \frac{\partial \mathbf{u}}{\partial t} + \boldsymbol{\Omega} \wedge \boldsymbol{\Omega} \wedge (\mathbf{p} + \mathbf{u})$$

$\boldsymbol{\Omega}$ is the rotation vector of the structure with

$$\boldsymbol{\Omega} = \begin{bmatrix} \omega_x \\ \omega_y \\ \omega_z \end{bmatrix}$$

in a (x, y, z) orthonormal frame. The skew-symmetric matrix $[\boldsymbol{\Omega}]$ is defined such that

$$[\boldsymbol{\Omega}] = \begin{bmatrix} 0 & -\omega_z & \omega_y \\ \omega_z & 0 & -\omega_x \\ -\omega_y & \omega_x & 0 \end{bmatrix}$$

The speed can be rewritten

$$\frac{\partial \mathbf{x}}{\partial t} = \frac{\partial \mathbf{u}}{\partial t} + [\boldsymbol{\Omega}] (\mathbf{p} + \mathbf{u})$$

and the acceleration becomes

$$\frac{\partial^2 \mathbf{x}}{\partial t^2} = \frac{\partial^2 \mathbf{u}}{\partial t^2} + \frac{\partial [\boldsymbol{\Omega}]}{\partial t} (\mathbf{p} + \mathbf{u}) + 2[\boldsymbol{\Omega}] \frac{\partial \mathbf{u}}{\partial t} + [\boldsymbol{\Omega}]^2 (\mathbf{p} + \mathbf{u})$$

In this expression appear

- the acceleration in the rotating frame $\frac{\partial^2 \mathbf{u}}{\partial t^2}$,
- the centrifugal acceleration $\mathbf{a}_g = [\Omega]^2 (\mathbf{p} + \mathbf{u})$,
- the Coriolis acceleration $\mathbf{a}_c = \frac{\partial[\Omega]}{\partial t} (\mathbf{p} + \mathbf{u}) + 2 [\Omega] \frac{\partial \mathbf{u}}{\partial t}$.

\mathcal{S}_0^e is an element of the mesh of the initial configuration \mathcal{S}_0 whose density is ρ_0 . $[N]$ is the matrix of shape functions on these elements, one defines the following elementary matrices

$$\begin{aligned} [D_g^e] &= \int_{\mathcal{S}_0^e} 2\rho_0 [N]^\top [\Omega] [N] d\mathcal{S}_0^e && \text{gyroscopic coupling} \\ [K_a^e] &= \int_{\mathcal{S}_0^e} \rho_0 [N]^\top \frac{\partial[\Omega]}{\partial t} [N] d\mathcal{S}_0^e && \text{centrifugal acceleration} \\ [K_g^e] &= \int_{\mathcal{S}_0^e} \rho_0 [N]^\top [\Omega]^2 [N] d\mathcal{S}_0^e && \text{centrifugal softening/stiffening} \end{aligned} \quad (6.65)$$

6.1.11 Centrifugal follower forces

This is the embryo of the theory for the future implementation of centrifugal follower forces.

$$\delta W_\omega = \int_\Omega \rho \omega^2 R(x) \delta v_R, \quad (6.66)$$

where δv_R designates the radial component (in deformed configuration) of δv . One assumes that the rotation axis is along e_z . Noting $n_R = 1/R\{x_1 \ x_2 \ 0\}^T$, one then has

$$\delta v_R = n_R \cdot \delta v. \quad (6.67)$$

Thus the non-linear stiffness term is given by

$$-d\delta W_\omega = - \int_\Omega \rho \omega^2 (dR \delta v_R + R d\delta v_R). \quad (6.68)$$

One has $dR = n_R \cdot dx (= dx_R)$ and $d\delta v_R = dn_R \cdot \delta v$, with

$$dn_R = -\frac{dR}{R} n_R + \frac{1}{R} \{dx_1 \ dx_2 \ 0\}^T.$$

Thus, finally

$$-d\delta W_\omega = - \int_\Omega \rho \omega^2 (du_1 \delta v_1 + du_2 \delta v_2). \quad (6.69)$$

Which gives

$$du_1 \delta v_1 + du_2 \delta v_2 = \{\delta q_\alpha\}^T \{N\} \{N\}^T \{dq_\alpha\}, \quad (6.70)$$

with $\alpha = 1, 2$.

6.1.12 Poroelastic materials

The poroelastic formulation comes from [32], recalled and detailed in [33].

Domain and variables description:

Ω	Poroelastic domain	
$\partial\Omega$	Bounding surface of poroelastic domain	
n	Unit external normal of $\partial\Omega$	
u	Solid phase displacement vector	
u^F	Fluid phase displacement vector	$u^F = \frac{\phi}{\tilde{\rho}_{22}\omega^2}\nabla p - \frac{\tilde{\rho}_{12}}{\tilde{\rho}_{22}}u$
p	Fluid phase pressure	
σ	Stress tensor of solid phase	
σ^t	Total stress tensor of porous material	$\sigma^t = \sigma - \phi \left(1 + \frac{\tilde{Q}}{\tilde{R}}\right) pI$

Weak formulation, for harmonic time dependence at pulsation ω :

$$\int_{\Omega} \sigma(u) : \epsilon(\delta u) \, d\Omega - \omega^2 \int_{\Omega} \tilde{\rho} \, u \cdot \delta u \, d\Omega - \int_{\Omega} \frac{\phi}{\tilde{\alpha}} \nabla p \cdot \delta u \, d\Omega - \int_{\Omega} \phi \left(1 + \frac{\tilde{Q}}{\tilde{R}}\right) p \nabla \cdot \delta u \, d\Omega - \int_{\partial\Omega} (\sigma^t(u) \cdot n) \cdot \delta u \, dS = 0 \quad \forall \delta u \quad (6.71)$$

$$\int_{\Omega} \frac{\phi^2}{\tilde{\alpha}\rho_o\omega^2} \nabla p \cdot \nabla \delta p \, d\Omega - \int_{\Omega} \frac{\phi^2}{\tilde{R}} p \, \delta p \, d\Omega - \int_{\Omega} \frac{\phi}{\tilde{\alpha}} u \cdot \nabla \delta p \, d\Omega - \int_{\Omega} \phi \left(1 + \frac{\tilde{Q}}{\tilde{R}}\right) \delta p \nabla \cdot u \, d\Omega - \int_{\partial\Omega} \phi (u^F - u) \cdot n \, \delta p \, dS = 0 \quad \forall \delta p \quad (6.72)$$

Matrix formulation, for harmonic time dependence at pulsation ω :

$$\begin{bmatrix} K - \omega^2 M & -C_1 - C_2 \\ -C_1^T - C_2^T & \frac{1}{\omega^2} F - K_p \end{bmatrix} \begin{Bmatrix} u \\ p \end{Bmatrix} = \begin{Bmatrix} F_s^t \\ F_f \end{Bmatrix} \quad (6.73)$$

where the frequency-dependent matrices correspond to:

$$\begin{aligned}
\int_{\Omega} \sigma(u) : \epsilon(\delta u) \, d\Omega &\Rightarrow \delta u^T K u \\
\int_{\Omega} \tilde{\rho} u \cdot \delta u \, d\Omega &\Rightarrow \delta u^T M u \\
\int_{\Omega} \frac{\phi^2}{\tilde{\alpha} \rho_o} \nabla p \cdot \nabla \delta p &\Rightarrow \delta p^T K_p p \\
\int_{\Omega} \frac{\phi^2}{\tilde{R}} p \, \delta p &\Rightarrow \delta p^T F p \\
\int_{\Omega} \frac{\phi}{\tilde{\alpha}} \nabla p \cdot \delta u \, d\Omega &\Rightarrow \delta u^T C_1 p \\
\int_{\Omega} \phi \left(1 + \frac{\tilde{Q}}{\tilde{R}} \right) p \nabla \cdot \delta u \, d\Omega &\Rightarrow \delta u^T C_2 p \\
\int_{\partial\Omega} (\sigma^t(u) \cdot n) \cdot \delta u \, dS &\Rightarrow \delta u^T F_s^t \\
\int_{\partial\Omega} \phi (u^F - u) \cdot n \, \delta p \, dS &\Rightarrow \delta p^T F_f
\end{aligned} \tag{6.74}$$

N.B. if the material of the solid phase is homogeneous, the frequency-dependent parameters can be eventually factorized from the matrices:

$$\begin{bmatrix} (1 + i\eta_s) \bar{K} - \omega^2 \tilde{\rho} \bar{M} & -\frac{\phi}{\tilde{\alpha}} \bar{C}_1 - \phi \left(1 + \frac{\tilde{Q}}{\tilde{R}} \right) \bar{C}_2 \\ -\frac{\phi}{\tilde{\alpha}} \bar{C}_1^T - \phi \left(1 + \frac{\tilde{Q}}{\tilde{R}} \right) \bar{C}_2^T & \frac{1}{\omega^2} \frac{\phi^2}{\tilde{R}} \bar{F} - \frac{\phi^2}{\tilde{\alpha} \rho_o} \bar{K}_p \end{bmatrix} \begin{Bmatrix} u \\ p \end{Bmatrix} = \begin{Bmatrix} F_s^t \\ F_f \end{Bmatrix} \tag{6.75}$$

where the matrices marked with bars are frequency independent:

$$\begin{aligned}
K &= (1 + i\eta_s) \bar{K} & M &= \tilde{\rho} \bar{M} & C_1 &= \frac{\phi}{\tilde{\alpha}} \bar{C}_1 \\
C_2 &= \phi \left(1 + \frac{\tilde{Q}}{\tilde{R}} \right) \bar{C}_2 & F &= \frac{\phi^2}{\tilde{R}} \bar{F} & K_p &= \frac{\phi^2}{\tilde{\alpha} \rho_o} \bar{K}_p
\end{aligned} \tag{6.76}$$

Material parameters:

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ϕ	Porosity of the porous material
$\bar{\sigma}$	Resistivity of the porous material
α_∞	Tortuosity of the porous material
Λ	Viscous characteristic length of the porous material
Λ'	Thermal characteristic length of the skeleton
ρ	Density of the skeleton
G	Shear modulus of the skeleton
ν	Poisson coefficient of the skeleton
η_s	Structural loss factor of the skeleton
ρ_o	Fluid density
γ	Heat capacity ratio of fluid (= 1.4 for air)
η	Shear viscosity of fluid (= $1.84 \times 10^{-5} \text{ kg m}^{-1} \text{ s}^{-1}$ for air)

Constants:

$P_o = 1,01 \times 10^5 \text{ Pa}$	Ambient pressure
$Pr = 0.71$	Prandtl number

Poroelastic specific (frequency dependent) variables:

ρ_{11}	Apparent density of solid phase	$\rho_{11} = (1 - \phi)\rho - \rho_{12}$
ρ_{22}	Apparent density of fluid phase	$\rho_{22} = \phi\rho_o - \rho_{12}$
ρ_{12}	Interaction apparent density	$\rho_{12} = -\phi\rho_o(\alpha_\infty - 1)$
$\tilde{\rho}$	Effective density of solid phase	$\tilde{\rho} = \tilde{\rho}_{11} - \frac{(\tilde{\rho}_{12})^2}{\tilde{\rho}_{22}}$
$\tilde{\rho}_{11}$	Effective density of solid phase	$\tilde{\rho}_{11} = \rho_{11} + \frac{\tilde{b}}{i\omega}$
$\tilde{\rho}_{22}$	Effective density of fluid phase	$\tilde{\rho}_{22} = \rho_{22} + \frac{\tilde{b}}{i\omega}$
$\tilde{\rho}_{12}$	Interaction effective density	$\tilde{\rho}_{12} = \rho_{12} - \frac{\tilde{b}}{i\omega}$
\tilde{b}	Viscous damping coefficient	$\tilde{b} = \phi^2 \bar{\sigma} \sqrt{1 + i \frac{4\alpha_\infty^2 \eta \rho_o \omega}{\bar{\sigma}^2 \Lambda^2 \phi^2}}$
$\tilde{\gamma}$	Coupling coefficient	$\tilde{\gamma} = \phi \left(\frac{\tilde{\rho}_{12}}{\tilde{\rho}_{22}} - \frac{\tilde{Q}}{\tilde{R}} \right)$
\tilde{Q}	Elastic coupling coefficient	
	Biot formulation	$\tilde{Q} = \frac{1 - \phi - \frac{K_b}{K_s}}{1 - \phi - \frac{K_b}{K_s} + \phi \frac{K_s}{\tilde{K}_f}} \phi K_s$
	Approximation from $K_b/K_s \ll 1$	$\tilde{Q} = (1 - \phi) \tilde{K}_f$
\tilde{R}	Bulk modulus of air in fraction volume	
	Biot formulation	$\tilde{R} = \frac{\phi^2 K_s}{1 - \phi - \frac{K_b}{K_s} + \phi \frac{K_s}{\tilde{K}_f}}$
	Approximation from $K_b/K_s \ll 1$	$\tilde{R} = \phi \tilde{K}_f$
K_b	Bulk modulus of porous material in vacuo	$K_b = \frac{2G(1 + \nu)}{3(1 - 2\nu)}$
K_s	Bulk modulus of elastic solid	
	est. from Hashin-Shtrikman's upper bound	$K_s = \frac{1+2\phi}{1-\phi} K_b$
\tilde{K}_f	Effective bulk modulus of air in pores	$\tilde{K}_f = \frac{P_o}{1 - \frac{\gamma - 1}{\gamma \alpha'}}$
α'	Function in \tilde{K}_f (Champoux-Allard model)	$\alpha' = 1 + \frac{\omega_T}{2i\omega} \left(1 + \frac{i\omega}{\omega_T} \right)^{\frac{1}{2}}$
ω_T	Thermal characteristic frequency	$\omega_T = \frac{16\eta}{Pr \Lambda^2 \rho_o}$

To add here:

- coupling conditions with poroelastic medium, elastic medium, acoustic medium
- dissipated power in medium

6.1.13 Heat equation

This section is based on an OpenFEM contribution by Bourquin Frédéric and Nas-siopoulos Alexandre from *Laboratoire Central des Ponts et Chaussées*.

The variational form of the Heat equation is given by

$$\int_{\Omega} (\rho c \dot{\theta})(v) dx + \int_{\Omega} (\mathbf{K} \text{grad } \theta)(\text{grad } v) dx + \int_{\partial\Omega} \alpha \theta v d\gamma = \int_{\Omega} f v dx + \int_{\partial\Omega} (g + \alpha \theta_{ext}) v d\gamma \quad (6.77)$$

$$\forall v \in H^1(\Omega)$$

with

- ρ the density, c the specific heat capacity.
- \mathbf{K} the conductivity tensor of the material. The tensor \mathbf{K} is symmetric, positive definite, and is often taken as diagonal. If conduction is isotropic, one can write $\mathbf{K} = k(x)Id$ where $k(x)$ is called the (scalar) conductivity of the material.
- Acceptable loads and boundary conditions are
 - Internal heat source f
 - Prescribed temperature (Dirichlet condition, also called boundary condition of first kind)

$$\theta = \theta_{ext} \quad \text{on} \quad \partial\Omega \quad (6.78)$$

modeled using a `DofSet` case entry.

- Prescribed heat flux g (Neumann condition, also called boundary condition of second kind)

$$(\mathbf{K} \text{grad } \theta) \cdot \vec{n} = g \quad \text{on} \quad \partial\Omega \quad (6.79)$$

leading to a load applied on the surface modeled using a `FVol` case entry.

- Exchange and heat flux (Fourier-Robin condition, also called boundary condition of third kind)

$$(\mathbf{K}grad\theta) \cdot \vec{n} + \alpha(\theta - \theta_{ext}) = g \quad \text{on} \quad \partial\Omega \quad (6.80)$$

leading to a stiffness term (modeled using a group of surface elements with stiffness proportional to α) and a load on the associated surface proportional to $g + \alpha\theta_{ext}$ (modeled using [FVo1](#) case entries).

Test case

One considers a solid square prism of dimensions L_x, L_y, L_z in the three directions (Ox), (Oy) and (Oz) respectively. The solid is made of homogeneous isotropic material, and its conductivity tensor thus reduces to a constant k .

The faces, $\Gamma_i (i = 1..6, \cup_{i=1}^6 \Gamma_i = \partial\Omega)$, are subject to the following boundary conditions and loads

- $f = 40$ is a constant uniform internal heat source
- $\Gamma_1 (x = 0)$: exchange & heat flux (Fourier-Robin) given by $\alpha = 1, g_1 = \alpha\theta_{ext} + \frac{\alpha f L_x^2}{2k} = 25$
- $\Gamma_2 (x = L_x)$: prescribed temperature : $\theta(L_x, y, z) = \theta_{ext} = 20$
- $\Gamma_3 (y = 0), \Gamma_4 (y = L_y), \Gamma_5 (z = 0), \Gamma_6 (z = L_z)$: exchange & heat flux $g + \alpha\theta_{ext} = \alpha\theta_{ext} + \frac{\alpha f}{2k}(L_x^2 - x^2) + g_1 = 25 - \frac{x^2}{20}$

The problem can be solved by the method of separation of variables. It admits the solution

$$\theta(x, y, z) = -\frac{f}{2k}x^2 + \theta_{ext} + \frac{fL_x^2}{2k} = \frac{g(x)}{\alpha} = 25 - \frac{x^2}{20}$$

The resolution for this example can be found in [demo/heat_equation](#).

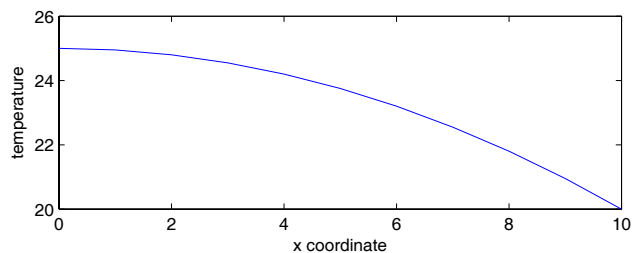


Figure 6.1: Temperature distribution along the x-axis

6.2 Model reduction theory

Finite element models of structures need to have many degrees of freedom to represent the geometrical detail of complex structures. For models of structural dynamics, one is however interested in

- a restricted frequency range ($s = i\omega \in [\omega_1 \ \omega_2]$)
- a small number of inputs and outputs (b, c)
- a limited parameter space α (updated physical parameters, design changes, nonlinearities, etc.)

These restrictions on the expected predictions allow the creation of low order models that accurately represent the dynamics of the full order model in all the considered loading/parameter conditions.

Model reduction notions are key to many *SDT* functions of all areas: to motivate residual terms in pole residue models (`id_rc`, `id_nor`), to allow fine control of model order (`nor2ss`, `nor2xf`), to create normal models of structural dynamics from large order models (`fe2ss`, `fe_reduc`), for test measurement expansion to the full set of DOFs (`fe_exp`), for substructuring using superelements (`fesuper`, `fe_coor`), for parameterized problems including finite element model updating (`upcom`).

6.2.1 General framework

Model reduction procedures are discrete versions of Ritz/Galerkin analyzes: they seek solutions in the subspace generated by a reduction matrix T . Assuming $\{q\} = [T] \{q_R\}$, the second order finite element model (5.1) is projected as follows

$$\begin{aligned} \left[T^T M T s^2 + T^T C T s + T^T K T \right]_{NR \times NR} \{q_R(s)\} &= \left[T^T b \right]_{NR \times NA} \{u(s)\}_{NA \times 1} \\ \{y(s)\}_{NS \times 1} &= [cT]_{NS \times NR} \{q_R(s)\}_{NR \times 1} \end{aligned} \quad (6.81)$$

Modal analysis, model reduction, component mode synthesis, and related methods all deal with an appropriate selection of singular projection bases ($[T]_{NR \times NR}$ with $NR \ll N$). This section summarizes the theory behind these methods with references to other works that give more details.

The solutions provided by *SDT* making two further assumptions which are not hard limitations but allow more consistent treatments while covering all but the most exotic problems. The projection is chosen to preserve reciprocity (left multiplication by T^T and not another matrix). The projection bases are assumed to be real.

An accurate model is defined by the fact that the input/output relation is preserved for a given frequency and parameter range

$$[c] [Z(s, \alpha)]^{-1} [b] \approx [cT] \left[T^T Z(s, \alpha) T \right]^{-1} \left[T^T b \right] \quad (6.82)$$

Traditional modal analysis, combines normal modes and static responses. *Component mode synthesis* methods extend the selection of boundary conditions used to compute the normal modes. The *SDT* further extends the use of reduction bases to parameterized problems.

A key property for model reduction methods is that the input/output behavior of a model only depends on the vector space generated by the projection matrix T . Thus $\text{range}(T) = \text{range}(\tilde{T})$ implies that

$$[cT] \left[T^T Z T \right]^{-1} \left[T^T b \right] = [c\tilde{T}] \left[\tilde{T}^T Z \tilde{T} \right]^{-1} \left[\tilde{T}^T b \right] \quad (6.83)$$

This **equivalence property** is central to the flexibility provided by the *SDT* in CMS applications (it allows the decoupling of the reduction and coupled prediction phases) and modeshape expansion methods (it allows the definition of a static/dynamic expansion on sensors that do not correspond to DOFs).

6.2.2 Normal mode models

Normal modes are defined by the eigenvalue problem

$$- [M] \{\phi_j\} \omega_j^2 + [K]_{N \times N} \{\phi_j\}_{N \times 1} = \{0\}_{N \times 1} \quad (6.84)$$

based on inertia properties (represented by the positive definite mass matrix M) and

underlying elastic properties (represented by a positive semi-definite stiffness K). The matrices being positive there are N independent eigenvectors $\{\phi_j\}$ (forming a matrix noted $[\phi]$) and eigenvalues ω_j^2 (forming a diagonal matrix noted $[\omega_j^2]$).

As solutions of the eigenvalue problem (6.84), the full set of N normal modes verify two **orthogonality conditions** with respect to the mass and the stiffness

$$[\phi]^T [M] [\phi] = [\mu_j]_{N \times N} \quad \text{and} \quad [\phi]^T [K] [\phi] = [\omega_j^2] \quad (6.85)$$

where μ is a diagonal matrix of modal masses (which are quantities depending uniquely on the way the eigenvectors ϕ are scaled).

In the *SDT*, the normal modeshapes are assumed to be mass normalized so that $[\mu] = [I]$ (implying $[\phi]^T [M] [\phi] = [I]$ and $[\phi]^T [K] [\phi] = [\omega_j^2]$). The **mass normalization** of modeshapes is independent from a particular choice of sensors or actuators.

Another traditional normalization is to set a particular component of $\tilde{\phi}_j$ to 1. Using an output shape matrix this is equivalent to $c_l \tilde{\phi}_j = 1$ (the observed motion at sensor c_l is unity). $\tilde{\phi}_j$, the modeshape with a component scaled to 1, is related to the mass normalized modeshape by $\tilde{\phi}_j = \phi_j / (c_l \phi_j)$.

$$m_j(c_l) = (c_l \phi_j)^{-2}$$

is called the **modal or generalized mass** at sensor c_l . A large modal mass denotes small output. For rigid body translation modes and translation sensors, the modal mass corresponds to the mass of the structure. If a diagonal matrix of generalized masses `mu` is provided and `ModeIn` is such that the output c_l is scaled to 1, the mass normalized modeshapes will be obtained by

$$\text{ModeNorm} = \text{ModeIn} * \text{diag}(\text{diag}(\text{mu}) . ^(-1/2));$$

Modal stiffnesses are equal to

$$k_j(c_l) = (c_l \phi_j)^{-2} \omega_j^2$$

The use of mass-normalized modes, simplifies the normal mode form (identity mass matrix) and allows the direct comparison of the contributions of different modes at similar sensors. From the orthogonality conditions, one can show that, for an undamped model and mass normalized modes, the dynamic response is described

by a sum of modal contributions

$$[\alpha(s)] = \sum_{j=1}^N \frac{\{c\phi_j\} \{\phi_j^T b\}}{s^2 + \omega_j^2} \quad (6.86)$$

which correspond to pairs of complex conjugate poles $\lambda_j = \pm i\omega_j$.

In practice, only the first few low frequency modes are determined, the series in (6.86) is truncated, and a correction for the truncated terms is introduced (see section 6.2.3).

Note that the concept of effective mass [34], used for rigid base excitation tests, is very similar to the notion of generalized mass.

6.2.3 Static correction to normal mode models

Normal modes are computed to obtain the spectral decomposition (6.86). In practice, one distinguishes modes that have a resonance in the model bandwidth and need to be kept and higher frequency modes for which one assumes $\omega \ll \omega_j$. This assumption leads to

$$[c] [Ms^2 + K]^{-1} [b] \approx \sum_{j=1}^{N_R} \frac{[c] \{\phi_j\} \{\phi_j\}^T [b]}{s^2 + \omega_j^2} + \sum_{j=N_R+1}^N \frac{[c] \{\phi_j\} \{\phi_j\}^T [b]}{\omega_j^2} \quad (6.87)$$

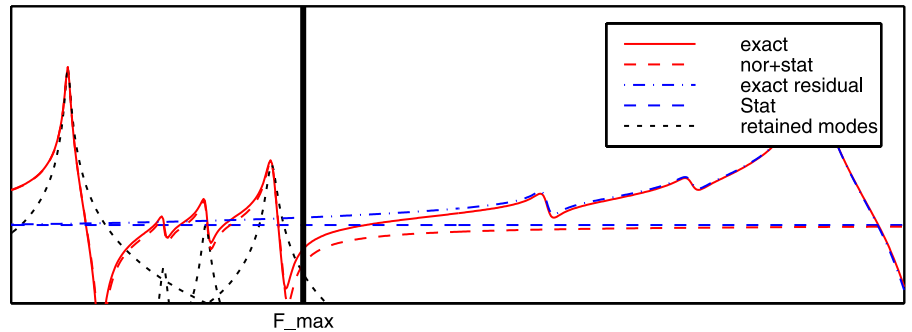


Figure 6.2: Normal mode corrections.

For the example treated in the `demo.fe` script, the figure shows that the exact response can be decomposed into retained modal contributions and an exact residual. In the selected frequency range, the exact residual is very well approximated by a constant often called the **static correction**.

The use of this constant is essential in identification phases and it corresponds to the E term in the pole/residue models used by `id_rc` (see under `res` page 165).

For applications in reduction of finite element models, a little more work is typically done. From the orthogonality conditions (6.85), one can easily show that for a structure with no rigid body modes (modes with $\omega_j = 0$)

$$[T_A] = [K]^{-1} [b] = \sum_{j=1}^N \frac{\{\phi_j\} \{\phi_j^T b\}}{\omega_j^2} \quad (6.88)$$

The static responses $K^{-1}b$ are called **attachment modes** in Component Mode Synthesis applications [35]. The inputs $[b]$ then correspond to unit loads at all interface nodes of a coupled problem.

One has historically often considered **residual attachment modes** defined by

$$[T_{AR}] = [K]^{-1} [b] - \sum_{j=1}^{NR} \frac{\{\phi_j\} \{\phi_j^T b\}}{\omega_j^2} \quad (6.89)$$

where NR is the number of normal modes retained in the reduced model.

The vector spaces spanned by $[\phi_1 \dots \phi_{NR} \ T_A]$ and $[\phi_1 \dots \phi_{NR} \ T_{AR}]$ are clearly the same, so that reduced models obtained with either are dynamically equivalent. For use in the *SDT*, you are encouraged to find a basis of the vector space that diagonalizes the mass and stiffness matrices (normal mode form which can be easily obtained with `fe_norm`).

Reduction on modeshapes is sometimes called the **mode displacement method**, while the addition of the **static correction** leads to the **mode acceleration method**.

When reducing on these bases, the selection of retained normal modes guarantees model validity over the desired frequency band, while adding the static responses guarantees validity for the spatial content of the considered inputs. The reduction is only valid for this restricted spatial/spectral content but very accurate for solicitations that verify these restrictions.

Defining the bandwidth of interest is a standard difficulty with no definite answer. The standard, but conservative, criterion (attributed to Rubin) is to keep modes with frequencies below 1.5 times the highest input frequency of interest.

6.2.4 Static correction with rigid body modes

For a system with NB rigid body modes kept in the model, $[K]$ is singular. Two methods are typically considered to overcome this limitation.

The approach traditionally found in the literature is to compute the static response of all flexible modes. For NB rigid body modes, this is given by

$$[K]^* [b] = \sum_{j=NB+1}^N \frac{\{\phi_j\} \{\phi_j^T b\}}{\omega_j^2} \quad (6.90)$$

This corresponds to the definition of **attachment modes** for free floating structures [35]. The flexible response of the structure can actually be computed as a static problem with an iso-static constraint imposed on the structure (use the `fe_reduc flex` solution and refer to [36] or [37] for more details).

The approach preferred in the *SDT* is to use a mass-shifted stiffness leading to the definition of **shifted attachment modes** as

$$[T_{AS}] = [K + \alpha M]^{-1} [b] = \sum_{j=1}^N \frac{\{\phi_j\} \{\phi_j^T b\}}{(\omega_j^2 + \alpha)} \quad (6.91)$$

While these responses don't exactly span the same subspace as static corrections, they can be computed using the mass-shifted stiffness used for eigenvalue computations. For small mass-shifts (a fraction of the lowest flexible frequency) and when modes are kept too, they are a very accurate replacement for attachment modes. It is the opinion of the author that the additional computational effort linked to the determination of true attachment modes is not mandated and shifted attachment modes are used in the *SDT*.

6.2.5 Other standard reduction bases

For coupled problems linked to model substructuring, it is traditional to state the problem in terms of imposed displacements rather than loads.

Assuming that the imposed displacements correspond to DOFs, one seeks solutions of problems of the form

$$\begin{bmatrix} Z_{II}(s) & Z_{IC}(s) \\ Z_{CI}(s) & Z_{CC}(s) \end{bmatrix} \begin{Bmatrix} \langle q_I(s) \rangle \\ q_C(s) \end{Bmatrix} = \begin{Bmatrix} R_I(s) \\ \langle 0 \rangle \end{Bmatrix} \quad (6.92)$$

where $\langle \rangle$ denotes a given quantity (the displacement q_I are given and the reaction forces R_I computed). The exact response to an imposed harmonic displacement

$q_I(s)$ is given by

$$\{q(s)\} = \begin{bmatrix} I \\ -Z_{CC}^{-1}Z_{CI} \end{bmatrix} \{q_I\} \quad (6.93)$$

The first level of approximation is to use a quasistatic evaluation of this response (evaluate at $s = 0$, that is use $Z(0) = K$). Model reduction on this basis is known as **static or Guyan condensation** [21].

This reduction does not fulfill the requirement of validity over a given frequency range. Craig and Bampton [38] thus complemented the static reduction basis by **fixed interface modes** : normal modes of the structure with the imposed boundary condition $q_I = 0$. These modes correspond to singularities Z_{CC} so their inclusion in the reduction basis allows a direct control of the range over which the reduced model gives a good approximation of the dynamic response.

The Craig-Bampton reduction basis takes the special form

$$\begin{Bmatrix} q_I(s) \\ q_C(s) \end{Bmatrix} = \begin{bmatrix} I & 0 \\ -K_{CC}^{-1}K_{CI} & \phi_C \end{bmatrix} \{q_R\} \quad (6.94)$$

where the fact that the additional fixed interface modes have zero components on the interface DOFs is very useful to allow direct coupling of various component models. `fe_reduc` provides a solver that directly computes the Craig-Bampton reduction basis.

A major reason of the popularity of the Craig-Bampton reduction basis is the fact that the interface DOFs q_I appear explicitly in the generalized DOF vector q_R . This is actually a very poor reason that has strangely rarely been challenged. Since the equivalence property tells that the predictions of a reduced model only depend on the projection subspace, it is possible to select the reduction basis and the generalized DOFs independently. The desired generalized DOFs can always be characterized by an observation matrix c_I . As long as $[c_I][T]$ is not rank deficient, it is thus possible to determine a basis \tilde{T} of the subspace spanned by T such that

$$[c_I][\tilde{T}] = \begin{bmatrix} [I]_{NI \times NI} & [0]_{NI \times (NR - NI)} \end{bmatrix} \quad (6.95)$$

The `fe_coor` function builds such bases, and thus let you use arbitrary reduction bases (loaded interface modes rather than fixed interface modes in particular) while preserving the main interest of the Craig-Bampton reduction basis for coupled system predictions (see example in section 6.3.3).

6.2.6 Substructuring

Substructuring is a process where models are divided into components and component models are reduced before a coupled system prediction is performed. This process is known as **Component Mode Synthesis** in the literature. Ref. [35] details the historical perspective while this section gives the point of view driving the *SDT* architecture (see also [39]).

One starts by considering disjoint components coupled by interface component(s) that are physical parts of the structure and can be modeled by the finite element method. Each component corresponds to a dynamic system characterized by its I/O behavior $H_i(s)$. Inputs and outputs of the component models correspond to interface DOFs.

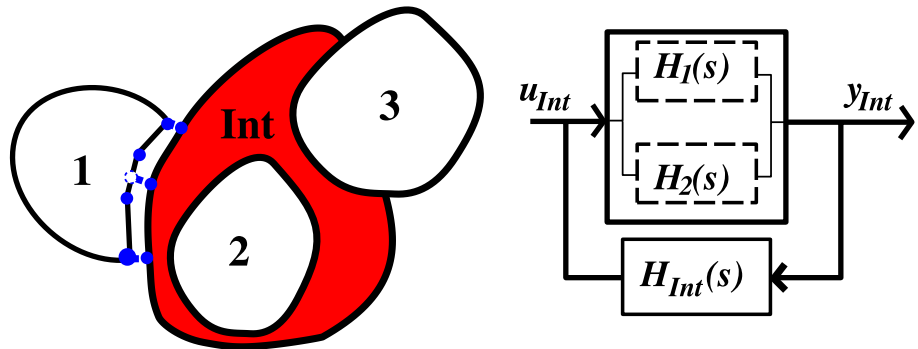


Figure 6.3: CMS procedure.

Traditionally, interface DOFs for the interface model match those of the components (the meshes are compatible). In practice the only requirement for a coupled prediction is that the interface DOFs linked to components be linearly related to the component DOFs $q_{j_{int}} = [c_j][q_j]$. The assumption that the components are disjoint assures that this is always possible. The observation matrices c_j are Boolean matrices for compatible meshes and involve interpolation otherwise.

Because of the duality between force and displacement (reciprocity assumption), forces applied by the interface(s) on the components are described by an input shape matrix which is the transpose of the output shape matrix describing the motion of interface DOFs linked to components based on component DOFs. Reduced component models must thus be accurate for all those inputs. CMS methods achieve this objective by keeping all the associated constraint or attachment modes.

Considering that the motion of the interface DOFs linked to components is imposed by the components, the coupled system (closed-loop response) is simply obtained adding the dynamic stiffness of the components and interfaces. For a case with two components and an interface with no internal DOFs, this results in a model coupled by the dynamic stiffness of the interface

$$\left(\begin{bmatrix} Z_1 & 0 \\ 0 & Z_2 \end{bmatrix} + \begin{bmatrix} c_1^T & 0 \\ 0 & c_2^T \end{bmatrix} [Z_{\text{int}}] \begin{bmatrix} c_1 & 0 \\ 0 & c_2 \end{bmatrix} \right) \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix} = [b] \{u(s)\} \quad (6.96)$$

The traditional CMS perspective is to have the dimension of the interface(s) go to zero. This can be seen as a special case of coupling with an interface stiffness

$$\left(\begin{bmatrix} Z_1 & 0 \\ 0 & Z_2 \end{bmatrix} + \begin{bmatrix} c_1^T & 0 \\ 0 & c_2^T \end{bmatrix} \frac{\begin{bmatrix} I & -I \\ -I & I \end{bmatrix}}{\epsilon} \begin{bmatrix} c_1 & 0 \\ 0 & c_2 \end{bmatrix} \right) \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix} = [b] \{u(s)\} \quad (6.97)$$

where ϵ tends to zero. The limiting case could clearly be rewritten as a problem with a displacement constraint (generalized kinematic or Dirichlet boundary condition)

$$\begin{bmatrix} Z_1 & 0 \\ 0 & Z_2 \end{bmatrix} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix} = [b] \{u(s)\} \quad \text{with} \quad [c_1 \quad -c_2] \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix} = 0 \quad (6.98)$$

Most CMS methods state the problem this way and spend a lot of energy finding an explicit method to eliminate the constraint. The *SDT* encourages you to use `fe_coor` which eliminates the constraint numerically and thus leaves much more freedom on how you reduce the component models.

In particular, this allows a reduction of the number of possible interface deformations [39]. But this reduction should be done with caution to prevent locking (excessive stiffening of the interface).

6.2.7 Reduction for parameterized problems

Methods described up to now, have not taken into account the fact that in (6.82) the dynamic stiffness can depend on some variable parameters. To apply model reduction to a variable model, the simplest approach is to retain the low frequency normal modes of the nominal model. This approach is however often very poor even if many modes are retained. Much better results can be obtained by taking some knowledge about the modifications into account [40].

In many cases, modifications affect a few DOFs: $\Delta Z = Z(\alpha) - Z(\alpha_0)$ is a matrix with mostly zeros on the diagonal and/or could be written as an outer product $\Delta Z_{N \times N} = [b_I] \left[\Delta \hat{Z} \right]_{NB \times NB} [b_I]^T$ with NB much smaller than N . An appropriate

reduction basis then combines nominal normal modes and static responses to the loads b_I

$$T = \begin{bmatrix} \phi_{1\dots NR} & [\hat{K}]^{-1} [b_I] \end{bmatrix} \quad (6.99)$$

In other cases, you know a typical range of allowed parameter variations. You can combine normal modes are selected representative design points to build a multi-model reduction that is exact at these points

$$T = [\phi_{1\dots NR}(\alpha_1) \quad \phi_{1\dots NR}(\alpha_2) \quad \dots] \quad (6.100)$$

If you do not know the parameter ranges but have only a few parameters, you should consider a model combining modeshapes and modeshape sensitivities [41] (as shown in the [gartup](#) demo)

$$T = \begin{bmatrix} \phi_{1\dots NR}(\alpha_0) & \frac{\partial \phi_{1\dots NR}}{\partial \alpha} & \dots \end{bmatrix} \quad (6.101)$$

For a better discussion of the theoretical background of fixed basis reduction for variable models see Refs. [40] and [41].

6.3 Superelements and CMS

6.3.1 Superelements in a model

A superelement is a model that is included in another global model as an element. In general superelements are reduced: the response at all DOFs is described by a linear combination of shapes characterized by generalized DOFs. The use of multiple superelements to generate system predictions is called Component Mode Synthesis (CMS).

Starting with SDT 6, superelements are stored as '**SE**' entries in the model stack (of the form '**SE**', *SEname*, *SEmodel*) with field detailed in section 6.3.2. Superelements are then referenced by element rows in a group of **SE** elements in the global model. A group of superelements in the **Elt** matrix begins by the header row [**Info** **abs('SE')** 0]. Each superelement is then defined by a row of the form

```
[NameCode N1 Nend BasId Elt1 EltEnd MatId ProId EltId].
```

- **NameCode** is an identifier encoding the superelement name using `fesuper('s_name')`. It is then assumed that the model stack contains an `'SE', name` entry containing the model constituting the superelement. **The encoding uses base2dec and is limited to 8 alphabetic lower case characters and numbers.**
- `[N1 Nend]` and `[Elt1 EltEnd]` are ranges of implicit `NodeId` and `EltId` of the superelement nodes and elements in the global model. That is to say that each node or element of the superelement is identified in the global model by an `Id` that can be different from the original `Id` of the superelement model stored in the stack. For more details see `Node`.
- `BasId` is the basis identifier in the `bas` field of the global model. It allows repositioning of the superelement in the global model.
- `Elt1, EltEnd` give the range of `EltId` used to identify elements constituting the superelement. These numbers are distinct from the superelement identifier itself.
- `MatId, ProId, EltId` are used to associate properties to a given superelement. Superelements support `p_super` property entries. Material information can be used for selection purposes.

The `d.cms` demo illustrates the Component Mode Synthesis based on a superelement element strategy. The model of this example (shown below) is composed by two stiffened plates. CMS here consists in splitting the model into two superelement plates that will be reduced, before computation of the global model modes.

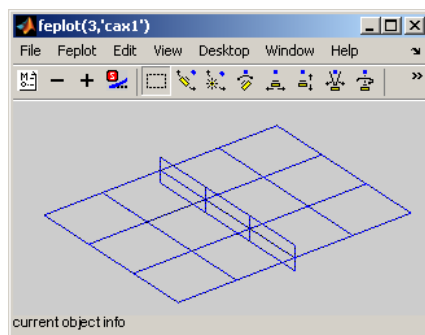


Figure 6.4: CMS example: 2 stiffened plates.

```
cf=demosdt('demoCMSSE feplot'); % get the full model
```

```
fecom('curtab Model')
feutilb('write',cf.mdl); % display the mode in text
```

Examples of superelement use are given in section 6.3.3.

6.3.2 SE data structure reference

The superelement data is stored as a '**SE**',*Name*,**Data** entry of the global model stack. The following entries describe standard fields of the superelement **Data** structure (which is a standard SDT model data structure with possible additional fields).

Opt

Options characterizing the type of superelement as follows:

Opt (1,1)	1 classical superelements, 3 FE update unique superelements (see upcom).
Opt (1,4)	1 for FE update superelement uses non symmetric matrices.
Opt (2,:) :	matrix types for the superelement matrices. Each non zero value on the second row of Opt specifies a matrix stored in the field K{i} (where i is the column number). The value of Opt (2, i) indicates the matrix type of K{i} . For standard types see MatType .
Opt (3,:) :	is used to define the coefficient associated with each of the matrices declared in row 2. An alternative mechanism is to define an element property in the il matrix. If these coefficients are not defined they are assumed to be equal to 1. See p_super for high level handling.

Node

Nominal node matrix. Contains the nodes used by the unique superelement or the nominal generic superelement (see section 7.1). The only restriction in comparison to a standard model **Node** matrix is that it must be sorted by **NodeId** so that the last node has the biggest **NodeId**.

In the element row declaring the superelement (see above) one defines a node range **N1 NEND**. The constraint on node numbers is that the defined range corresponds to the largest node number in the superelement (**NEND-N1+1=max(SE.Node(:,1))**). Not all nodes need to be defined however.

Nodes numbers in the full model are given by
`NodeId=SE.Node(:,1)-max(SE.Node(:,1))+NEND`
`N1` is really only used for coherence checking).

`K{i}`, `Klab{i}`, `DOF`

Superelement matrices. The presence and type of these matrices is declared in the `Opt` field (see above) and should be associated with a label giving the meaning of each matrix.

All matrices must be consistent with the `.DOF` field which is given in internal node numbering. When multiple instances of a superelement are used, node identifiers are shifted.

For multiple instances of a superelement that are in rotated reference frames, the matrices are no longer rotated (as available in SDT 5 with the `.Ref` field) since we have found this strategy to have a negative performance impact in most applications.

`Elt`, `Node`, `il`, `pl`

Initial model retrieval for `unique` superelements. `Elt` field contains the initial model description matrix which allows the construction of a detailed visualization as well as post-processing operations. `Node` contains the nodes used by this model. The `.pl` and `.il` fields store material and element properties for the initial model.

Once the matrices built, `SE.Elt` may be replaced by a display mesh if appropriate.

`TR`

`TR` field contains the definition of a possible projection on a reduction basis. This information is stored in a structure array with fields

- `.DOF` is the model active DOF vector.
- `.def` is the projection matrix. There is as many columns as DOFs in the reduced basis (stored in the `DOF` field of the superelement structure array), and as many row as active DOFs (stored in `TR.DOF`).
- `.adof`, when appropriate, gives a list of DOF labels associated with columns of `TR.def`
- `.data`, when appropriate, gives a list frequencies associated with columns of `TR.def`

6.3.3 An example of SE use for CMS

Following example splits the 2 stiffened plane models into 2 sub models, and defines a new model with those 2 sub models taken as superelements.

First the 2 sub models are built

```
model=demosdt('demo CMS');
SE1.Node=model.Node; SE2.Node=model.Node;
[ind,SE1.Elt]=feutil('FindElt WithNode{x>0|z>0}',model); % sel 1st plate
SE1.Node=feutil('OptimModel',SE1); SE1=feutil('renumber',SE1);
[ind,SE2.Elt]=feutil('FindElt WithNode{x<0|z<0}',model); % sel 2nd plate
SE2.Node=feutil('OptimModel',SE2); SE2=feutil('renumber',SE2);
```

Then mSE model is built including those 2 models as superelements

```
mSE.Node=[];
mSE.Elt=[Inf abs('SE') 0 0 0 0 0 0; % header row for superelements
        fesuper('s_se1') 1 16 0 1 1 100 100 1 ; % SE1
        fesuper('s_se2') 101 116 0 2 2 101 101 2]; % SE2
mSE=stack_set(mSE,'SE','se1',SE1); mSE=stack_set(mSE,'SE','se2',SE2);
feplot(mSE); fecom('promodelinit')
```

This is a low level strategy. `fesuper` provides a set of commands to easily manipulate superelements. In particular the whole example above can be performed by a single call to `fesuper('SelAsSE')` command as shown in the CMS example in section 6.3.3.

In this example one takes a full model split it into two superelements through element selections

```
model=demosdt('demoCMS'); % get the full model
feutil('infoelt',model)
mSE=fesuper('SESelAsSE-dispatch',model, ...
    {'WithNode{x>0|z>0}','WithNode{x<0|z<0}'});
feutil('infoelt',mSE)
[eltid,mSE.Elt]=feutil('eltidfix',mSE);
```

Then the two superelements are stored in the stack of `mSE`. Both of them are reduced using `fe_reduc` (with command option `-SE` for superelement, and `-UseDof` in order to obtain physical DOFs) Craig-Bampton reduction. This operation creates the `.DOF` (reduced DOFs), `.K` (superelement reduced matrices) and `.TR` (reduction basis) fields in the superelement models.

Those operations can be performed with following commands (see `fesuper`)

```

mSE=fesuper(mSE,'setStack','se1','info','EigOpt',[5 50 1e3]);
mSE=fesuper(mSE,'settr','se1','CraigBampton -UseDof');
mSE=fesuper(mSE,'setStack','se2','info','EigOpt',[5 50 1e3]);
mSE=fesuper(mSE,'settr','se2','CraigBampton -UseDof');

```

This is the same as following lower level commands

```

SE1=stack_get(mSE,'SE','se1','getdata');
SE1=stack_set(SE1,'info','EigOpt',[5 50.1 1e3]);
SE1=fe_reduc('CraigBampton -SE -UseDof',SE1);
mSE=stack_set(mSE,'SE','se1',SE1);

```

```

SE2=stack_get(mSE,'SE','se2','getdata');
SE2=stack_set(SE2,'info','EigOpt',[5 50.1 1e3]);
SE2=fe_reduc('CraigBampton -SE -UseDof',SE2);
mSE=stack_set(mSE,'SE','se2',SE2);

```

Then the modes can be computed, using the reduced superelements

```

def=fe_eig(mSE,[5 20 1e3]); % reduced model
dfull=fe_eig(model,[5 20 1e3]); % full model

```

The results of full and reduced models are very close. The frequency error for the first 20 modes is lower than 0.02 %.

`fesuper` provides a set of commands to manipulate superelements. `fesuper('SEAdd')` lets you add a superelement in a model. One can add a model as a unique superelement or repeat it with translations or rotations.

For CMS for example, one has to split a model into sub structure superelement models. It can be performed by the `fesuper SESelAsSE` command. This command can split a model into superelements defined by selections, or can build the model from sub models taken as superelements. The `fesuper SEDispatch` command dispatches the global model constraints (`FixDof`, `mpc`, `rbe3`, `DofSet` and `rigid` elements) into the related superelements and defines `DofSet` (imposed displacements) on the interface DOFs between sub structures.

6.3.4 Obsolete superelement information

The following strategy is now obsolete and should not be used even though it is still tested.

Superelements are stored in global variables whose name is of the form `SEName`. `fe_super` ensures that superelements are correctly interpreted as regular elements

during model assembly, visualization, etc. The superelement *Name* must differ from all function names in your MATLAB `path`. By default these variables are not declared as global in the base workspace. Thus to access them from there you need to use `global SName`.

Reference to the superelements is done using element group headers of the form `[Inf abs('name')]`.

The `fesuper` user interface provides standard access to the different fields (see `fe_super` for a list of those fields). The following sections describe currently implemented commands and associated arguments (see the `commode` help for hints on how to build commands and understand the variants discussed in this help).

Warnings. In the commands superelement names must be followed by a space (in most other cases user interface commands are not sensitive to spaces).

Info *Outputs a summary of current properties of the superelement Name.*

Load, Save Load `FileName` loads superelements (variables with name of the form `SName`) present in the file.

`SaveFileName Name1 Name2 ...` saves superelements `Name1, Name2 ...` in the file.

Note that these commands are really equivalent to `global SName;save FileName SName` and `global SName;load FileName SName`.

Make `elt=fesuper('make Name generic')` takes a unique superelement and makes it generic (see `fe_super` for details on generic superelements). `Opt(1,1)` is set to 2. `SName.DOF` is transformed to a generic DOF form. The output `elt` is a model description matrix for the nominal superelement (header row and one element property row). This model can be used by `femesh` to build structures that use the generic superelement several times (see the `d_cms2` demo).

`make complete` adds zero DOFs to nodes which have less than 3 translations (DOFs `.01` to `.03`) or rotations (DOFs `.04` to `.06`). Having complete superelements is important to be able to rotate them (used for generic superelements with a `Ref` property).

New *New unique superelement declaration using the general format*
`fesuper('New Name',FNode,FEelt)`. If a superelement called *Name* exists it is erased. The `Node` and `Elt` properties are set to those given as arguments. The `Patch` property used by `feplot` for display is initialized.

Set calls of the form `fesuper('Set Name FieldOrCommand', 'Value')` are obsolete and replaced as follows

`ref` field are now replaced by the definition of local bases for each instance of the superelement.

`patch` simply replace the superelement `.Elt` field by another simplified model to be used for viewing once the matrices have been defined.

`ki type fesuper('set Name k i type',Mat)` sets the superelement matrix $K\{i\}$ to `Mat` and its type to `type`. The size of `Mat` must be coherent with the superelement DOF vector. `type` is a positive integer giving the meaning of the considered matrix (see `MatType`).

6.3.5 Sensors and superelements

All sensors, excepted resultant sensor, are supported for superelement models. One can therefore add a sensor with the same way as for a standard model with `fe_case('SensDof')` commands: `fe_case(model, 'SensDof [append, combine] SenType', Name, Sensor)`. `Name` contains the entry name in the stack of the set of sensors where `Sensor` will be added. `Sensor` is a structure of data, a vector, or a matrix, which describes the sensor (or sensors) to be added to `model`. Command option `append` specifies that the `SensId` of latter added sensors is increased if it is the same as a former sensor `SensId`. With `combine` command option, latter sensors take the place of former same `SensId` sensors. See section 4.3 for more details.

Following example defines some sensors in the last `mSE` model

```
model=demosdt('demoCMS'); % get the full model
mSE=fesuper('SESelAsSE-dispatch',model, ...
    {'WithNode{x>0|z>0}';'WithNode{x<0|z<0}'});
[eltid,mSE.Elt]=feutil('eltidfix',mSE);

mSE=fesuper(mSE,'setStack','se1','info','EigOpt',[5 50 1e3]);
mSE=fesuper(mSE,'settr','se1','CraigBampton -UseDof');
mSE=fesuper(mSE,'setStack','se2','info','EigOpt',[5 50 1e3]);
mSE=fesuper(mSE,'settr','se2','CraigBampton -UseDof');

Sensors={ [0,0.0,0.75,0.0,0.0,1.0,0.0]; % Id,x,y,z,nx,ny,nz
    [0,10,0.0,0.0,1.0]; % Id,NodeId,nx,ny,nz
    [29.01]}; % DOF
for j1=1:length(Sensors);
    mSE=fe_case(mSE,'SensDof append trans','output',Sensors{j1});
```

```

end
mSE=fe_case(mSE,'SensDof append stress','output',[111,22,0.0,1.0,0.0]);
fe_case('SensMatch') command is the same as for standard models
mSE=fe_case(mSE,'SensMatch Radius2','output');

```

Use `fe_case('SensSE')` to build the observation matrix on the reduced basis

```
Sens=fe_case(mSE,'SensSE','output');
```

For resultant sensors, standard procedure does not work at this time. If the resultant sensor only relates to a specific superelement in the global model, it is however possible to define it. The strategy consists in defining the resultant sensor in the superelement model. Then one can build the observation matrix associated to this sensor, come back to the implicit nodes in the global model, and define a general sensor in the global model with the observation matrix. This strategy is described in following example.

One begins by defining resultant sensor in the related superelement

```

SE=stack_get(mSE,'SE','se2','GetData'); % get superelement
Sensor=struct('ID',0, ...
    'EltSel','WithNode{x<-0.5}'); % left part of the plate
Sensor.SurfSel='x==-0.5'; % middle line of the plate
Sensor.dir=[1.0 0.0 0.0]; % x direction
Sensor.type='resultant'; % type = resultant
SE=fe_case(SE,'SensDof append resultant',...
    'output',Sensor); % add resultant sensor to SE

```

Then one can build the associated observation matrix

```

SE=fe_case(SE,'SensMatch radius .6','output'); % SensMatch
Sens=fe_case(SE,'Sens','output'); % Build observation

```

Then one can convert the `SE` observation matrix to a `mSE` observation matrix, by renumbering DOF (this step is not necessary here since the use of `fesuperSESelAsSE` command assures that implicit numbering is the same as explicit numbering)

```

cEGI=feutil('findelt eltname SE:se2',mSE);
% implicit nodes of SE in mSE
i1=SE.Node(:,1)-max(SE.Node(:,1))+mSE.Elt(cEGI,3);
% renumber DOF to fit with the global model node numbers:
NNode=sparse(SE.Node(:,1),1,i1);
Sens.DOF=full(NNode(fix(Sens.DOF)))+rem(Sens.DOF,1);

```

Finally, one can add the resultant sensor as a general sensor

```
mSE=fe_case(mSE,'SensDof append general','output',Sens);
```

One can define a load from a sensor observation as following, and compute FRFs:

```
mSE=fe_case(mSE,'DofLoad SensDofSE','in','output:2') % from 2nd output sensor
```

```
def=fe_eig(mSE,[5 20 1e3]); % reduced model
```

```
nor2xf(def,mSE,'acc iipplot'); ci=iipplot;
```

6.4 Model parameterization

6.4.1 Parametric models, zCoef

Different major applications use families of structural models. *Update problems*, where a comparison with experimental results is used to update the mass and stiffness parameters of some elements or element groups that were not correctly modeled initially. *Structural design problems*, where component properties or shapes are optimized to achieve better performance. *Non-linear problems* where the properties of elements change as a function of operating conditions and/or frequency (viscoelastic behavior, geometrical non-linearity, etc.).

A *family of models* is defined (see [40] for more details) as a group of models of the general second order form (5.1) where the matrices composing the dynamic stiffness depend on a number of *design parameters* p

$$[Z(p, s)] = [M(p)s^2 + C(p)s + K(p)] \quad (6.102)$$

Moduli, beam section properties, plate thickness, frequency dependent damping, node locations, or component orientation for articulated systems are typical p parameters. The dependence on p parameters is often very non-linear. It is thus often desirable to use a model description in terms of other parameters α (which depend non-linearly on the p) to describe the evolution from the initial model as a linear combination

$$[Z(p, s)] = \sum_{j=1}^{NB} \alpha_j(p) [Z_{j\alpha}(s)] \quad (6.103)$$

with each $[Z_{j\alpha}(s)]$ having constant mass, damping and stiffness properties.

Plates give a good example of p and α parameters. If p represents the plate thickness, one defines three α parameters: t for the membrane properties, t^3 for the bending properties, and t^2 for coupling effects.

p parameters linked to elastic properties (plate thickness, beam section properties, frequency dependent damping parameters, etc.) usually lead to low numbers of α parameters so that the α should be used. In other cases (p parameters representing node positions, configuration dependent properties, etc.) the approach is impractical and p should be used directly.

par

SDT handles parametric models where various areas of the model are associated with a scalar coefficient weighting the model matrices (stiffness, mass, damping, ...). The first step is to define a set of parameters, which is used to decompose the full model matrix in a linear combination.

The elements are grouped in non overlapping sets, indexed m , and using the fact that element stiffness depend linearly on the considered moduli, one can represent the dynamic stiffness matrix of the parameterized structure as a linear combination of constant matrices

$$[Z(G_m, s)] = s^2 [M] + \sum_m p_m [K_m] \quad (6.104)$$

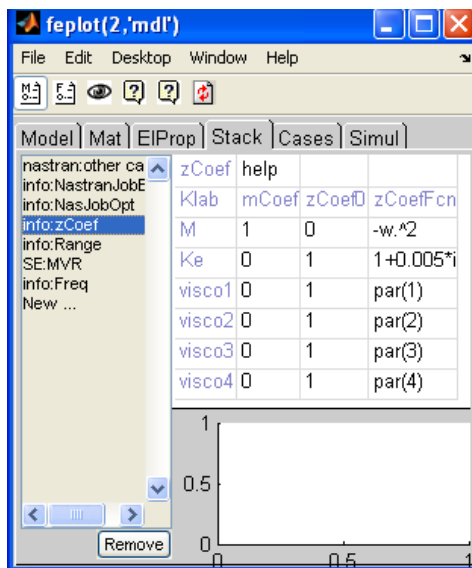
Parameters are case stack entries defined by using `fe_case par` commands (which are identical to `upcom Par` commands for an `upcom` superelement).

A parameter entry defines a element selection and a type of varying matrix. Thus

```
model=demosdt('demoubeam');
model=fe_case(model,'par k 1 .1 10','Top','withnode {z>1}');
fecom('proviewon');fecom('curtabCase','Top') % highlight the area
```

zcoef

The weighting coefficients in (6.104) are defined formally using the `cf.Stack{'info','zCoef'}` cell array viewed in the figure and detailed below.



The columns of the cell array, which can be modified with the `feplot` interface, give

- the matrix labels `Klab` which must coincide with the defined parameters
- the values of coefficients in (6.104) for the nominal mass (typically `mCoef=[1 0 0 ...]`)
- the real valued coefficients `zCoef0` in (6.104) for the nominal stiffness K_0
- the values or strings `zCoefFcn` to be evaluated to obtain the coefficients for the dynamic stiffness (6.104).

Given a model with defined parameters/matrices, `model=fe_def('zcoef-default',model)` defines default parameters.

`zcoef=fe_def('zcoef',model)` returns weighing coefficients for a range of values using the frequencies (see `Freq`) and design point stack entries

Frequencies are stored in the model using a call of the form `model=stack_set(model,'info','Fr`. Design points (temperatures, optimization points, ...) are stored as rows of the `'info','Range'` entry, see `fevisco Range` for generation.

When computing a response, `fe_def zCoef` starts by putting frequencies in a local variable `w` (which by convention is always in rd/s), and the current design point (row of `'info','Range'` entry or row of its `.val` field if it exists) in a local variable `par`.

`zCoef2:end,4` is then evaluated to generate weighting coefficients `zCoef` giving the weighting needed to assemble the dynamic stiffness matrix (6.104). For example in a parametric analysis, where the coefficient `par(1)` stored in the first column of `Range`. One defines the ratio of current stiffness to nominal $Kv_{current} = par(1) * Kv(nominal)$ as follows

```
% external to fe2xf
zCoef={'Klab','mCoef','zCoef0','zCoefFcn';
      'M'   1      0      '-w.^2';
      'Ke'  0      1      '1+i*fe_def('DefEta',[1]);
      'Kv'  0      1      'par(1)'};
model=struct('K',{cell(1,3)});
model=stack_set(model,'info','zCoef',zCoef);
model=stack_set(model,'info','Range',...
    struct('val',[1;2;3],'lab',{'par'}));

%Within fe2xf
w=[1:10]*2*pi; % frequencies in rad/s
Range=stack_get(model,'info','Range','getdata');
for jPar=1:size(Range.val,1)
    Range.jPar=jPar;zCoef=fe2xf('zcoef',model,w,Range);
    disp(zCoef)
    % some work gets done here ...
end
```

6.4.2 Reduced parametric models

As for nominal models, parameterized models can be reduced by projection on a constant reduction basis T leading to input/output models of the form

$$\begin{aligned} [T^T Z(p, s) T] \{q_R\} &= [T^T b] \{u(s)\} \\ \{y(s)\} &= [cT] \{q_R\} \end{aligned} \quad (6.105)$$

or, using the α parameters,

$$\begin{aligned} \sum_{j=1}^{NB} \alpha_j(p) [T^T \Delta Z_{j\alpha}(s) T] \{q_R\} &= [T^T b] \{u(s)\} \\ \{y(s)\} &= [cT] \{q_R\} \end{aligned} \quad (6.106)$$

6.4.3 `upcom` parameterization for full order models

Although superelements can deal with arbitrary models of the form (6.103), the `upcom` interface is designed to allow easier parameterization of models. This interface stores a long list of mass M^e and stiffness K^e matrices associated to each element and provides, through the `assemble` command, a fast algorithm to assemble the full order matrices as weighted sums of the form

$$[M(p)] = \sum_{j=1}^{NE} \alpha_k(p) [M_k^e] \quad [K(p)] = \sum_{j=1}^{NE} \beta_k(p) [K_k^e] \quad (6.107)$$

where the nominal model corresponds to $\alpha_k(p) = \beta_k(p) = 1$.

The basic parameterizations are mass p_i and stiffness p_j coefficients associated to element selections e_i, e_j leading to coefficients

$$\begin{aligned} \alpha_k, \beta_k &= 1 && \text{for } k \notin e_i \\ \alpha_k &= p_i && \text{for } k \in e_i \\ \beta_k &= p_j && \text{for } k \in e_j \end{aligned} \quad (6.108)$$

Only one stiffness and one mass parameter can be associated with each element. The element selections e_i and e_j are defined using `upcom Par` commands. In some `upcom` commands, one can combine changes in multiple parameters by defining a matrix `dirp` giving the p_i, p_j coefficients in the currently declared list of parameters.

Typically each element is only associated to a single mass and stiffness matrix. In particular problems, where the dependence of the element matrices on the design parameter of interest is non-linear and yet not too complicated more than one submatrix can be used for each element.

In practice, the only supported application is related to plate/shell thickness. If p represents the plate thickness, one defines three α, β parameters: t for the membrane properties, t^3 for the bending properties, and t^2 for coupling effects. This decomposition into element submatrices is implemented by specific element functions, `q4up` and `q8up`, which build element submatrices by calling `quad4` and `quadb`. Triangles are supported through the use of degenerate `quad4` elements.

Element matrix computations are performed before variable parameters are declared. In cases where thickness variations are desired, it is thus important to declare which group of plate/shell elements may have a variable thickness so that submatrices will be separated during the call to `fe_mk`. This is done using a call of the form `upcom('set nominal t GroupID', FEnode, FEel0, pl, il)`.

6.4.4 Getting started with `upcom`

Basic operation of the `upcom` interface is demonstrated in `gartup`.

The first step is the selection of a file for the superelement storage using `upcom('load FileName')`. If the file already exists, existing fields of `Up` are loaded. Otherwise, the file is created.

If the results are not already saved in the file, one then computes mass and stiffness element matrices (and store them in the file) using

```
upcom('setnominal',FEnode,FEelt,pl,il)
```

which calls `fe_mk`. You can of course eliminate some DOFs (for fixed boundary conditions) using a call of the form

```
upcom('setnominal',FEnode,FEelt,pl,il,[],adof)
```

At any time, `upcom info` will printout the current state of the model: dimensions of full/reduced model (or a message if one or the other is not defined)

```
'Up' superelement (stored in '/tmp/tp425896.mat')
```

```
Model Up.Elt with 90 element(s) in 2 group(s)
```

```
Group 1 :    73 quad4  MatId 1 ProId 3
```

```
Group 6 :    17 q4up  MatId 1 ProId 4
```

```
Full order (816 DOFs, 90 elts, 124 (sub)-matrices, 144 nodes)
```

```
Reduced model undefined
```

```
No declared parameters
```

In most practical applications, the coefficients of various elements are not independent. The `upcom par` commands provide ways to relate element coefficients to a small set of design variables. Once parameters defined, you can easily set parameters with the `parcoef` command (which computes the coefficient associated to each element (sub-)matrix) and compute the response using the `upcom compute` commands. For example

```
upcom('load GartUp');  
upcom('ParReset')  
upcom('ParAdd k','Tail','group3');  
upcom('ParAdd t','Constrained Layer','group6');  
upcom('ParCoef',[1.2 1.1]);  
upcom('info')
```

```
cf=upcom('plotelt')
cf.def(1)=upcom('computemode full 6 20 1e3 11')
fecom('scd.3');
```

6.4.5 Reduction for variable models

The `upcom` interface allows the simultaneous use of a full and a reduced order model. For any model in a considered family, the full and reduced models can give estimates of all the *qualities* (static responses, modal frequencies, modeshapes, or damped system responses). The reduced model estimate is however much less numerically expensive, so that it should be considered in iterative schemes.

The selection of the reduction basis T is essential to the accuracy of a reduced family of models. The simplest approach, where low frequency normal modes of the nominal model are retained, very often gives poor predictions. For other bases see the discussion in section 6.2.7.

A typical application (see the `gartup` demo), would take a basis combining modes and modeshape sensitivities, orthogonalize it with respect to the nominal mass and stiffness (doing it with `fe_norm` ensures that all retained vectors are independent), and project the model

```
upcom('parcoef',[1 1]);
[fsen,mdsen,mode,freq] = upcom('sens mode full',eye(2),7:20);
[m,k]=upcom('assemble');T = fe_norm([mdsen mode],m,k);
upcom('par red',[T])
```

In the `gartup` demo, the time needed to predict the first 20 modes is divided by 10 for the reduced model. For larger models, the ratio is even greater which really shows how much model reduction can help in reducing computational times.

Note that the projected model corresponds to the currently declared variable parameters (and in general the projection basis is computed based on knowledge of those parameters). If parameters are redefined using `Par` commands, you must thus project the model again.

6.4.6 Predictions of the response using `upcom`

The `upcom` interface provides optimized code for the computation, at any design point, of modes (`ComputeMode` command), modeshape sensitivities (`SensMode`), frequency response functions using a modal model (`ComputeModal`) or by directly inverting the dynamic stiffness (`ComputeFRF`). All predictions can be made based on

either the full or reduced order model. The default model can be changed using `upcom('OptModel[0,1]')` or by appending `full` or `reduced` to the main command. Thus

```
upcom('ParCoef',[1 1]);
[md1,f1] = upcom('compute mode full 105 20 1e3');
[md2,f2] = upcom('compute mode reduced');
```

would be typical calls for a full (with a specification of the `fe_eig` options in the command rather than using the `Opt` command) and reduced model.

Warning: unlike `fe_eig`, `upcom` typically returns frequencies in Hz (rather than rd/s) as the default unit option is `11` (for rd/s use `upcom('optunit22')`)

Given modes you could compute FRFs using

```
IIxh = nor2xf(freq,0.01,mode'*b,c*mode,IIw*2*pi);
```

but this does not include a static correction for the inputs described by `b`. You should thus compute the FRF using (which returns modes as optional output arguments)

```
[IIxh,mode,freq] = upcom('compute modal full 105 20',b,c,IIw);
```

This approach to compute the FRF is based on modal truncation with static correction (see section 6.2.3). For a few frequency points or for exact full order results, you can also compute the response of the full order model using

```
IIxh = upcom('compute FRF',b,c,IIw);
```

In FE model update applications, you may often want to compute modal frequencies and shape sensitivities to variations of the parameters. Standard sensitivities are returned by the `upcom sens` command (see the *Reference* section for more details).

6.5 Finite element model updating

While the `upcom` interface now provides a flexible environment that is designed for finite element updating problems, integrated methodologies for model updating are not stabilized. As a result, the *SDT* currently only intends to provide an efficient platform for developing model updating methodologies. This platform has been successfully used, by *SDTools* and others, for updating industrial models, but the details of parameter selection and optimization strategies are currently only provided through consulting services.

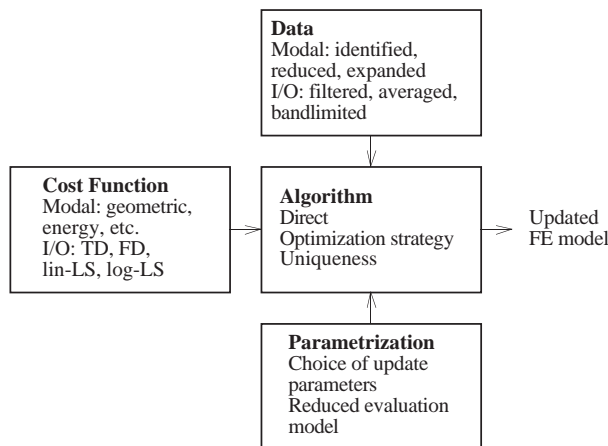


Figure 6.5: FE updating process.

The objective of finite element updating is to estimate certain design parameters (physical properties of the model) based on comparisons of test and analysis results. All the criteria discussed in section 3.2 can be used for updating.

The correlation tools provided by `fe_sens` and `fe_exp` are among the best existing on the market and major correlation criteria can easily be implemented. With *SDT* you can thus easily implement most of the existing error localization algorithms. No mechanism is however implemented to automatically translate the results of this localization into a set of parameters to be updated. Furthermore, the updating algorithms provided are very basic.

6.5.1 Error localization/parameter selection

The choice of design parameters to be updated is central to FE update problems. Update parameters should be chosen based on the knowledge that they have not been determined accurately from initial component tests. Whenever possible, the actual values of parameters should be determined using refined measurements of the component properties as the identifiability of the parameters is then clear. If such refined characterizations are not possible, the comparison of measured and predicted responses of the overall system provide a way to assess the probable value of a restricted set of parameters.

Discrepancies are always expected between the model and test results. Parameter updates made based on experimentally measured quantities should thus be limited

to parameters that have an impact on the model that is large enough to be clearly distinguished from the expected residual error. Such parameters typically are associated to connections and localized masses.

In practice with industrial models, the FE model is initially divided into zones with one mass/stiffness parameter associated with each zone. The `feutil FindElt` commands can greatly help zone definition.

Visualizing the strain/kinetic energy distribution of modeshapes is a typical way to analyze zones where modifications will significantly affect the response. The `gartup` demo shows how the strain energy of modeshapes and displacement residuals can be used in different phases of the error localization process.

6.5.2 Update based on frequencies

As illustrated in `demo_fe`, once a set of update parameters chosen, you should verify that the proper range is set (see `min` and `max` values in section 6.4.4), make sure that `Up.copt` options are appropriately set to allow the computation of modes and sensitivities (see `upcom copt` commands), and define a sensor configuration matrix `sens` using `fe_sens`.

With test results typically stored in poles `IIpo` and residues `IIres` (see section 2.3), the update based on frequencies is then simply obtained by a call of the form

```
i2=1:8; % indices of poles used for the update
[coef,md1,f1] = up_freq('basic',IIpo(i2,:),IIres(i2,:).',sens);
```

The result is obtained by a sensitivity method with automated matching of test and analysis modes using the MAC criterion. A non-linear optimization based solution can be found using `up_ifreq` but computational costs tend to prevent actual use of this approach. Using reduced order models (see section 6.4.5 and start use with `upcom('opt model 1')`) can alleviate some of the difficulties but the sensitivity based method (`up_freq`) is clearly better.

6.5.3 Update based on FRF

An update algorithm based on a non-linear optimization of the Log-Least-Squares cost comparing FRFs is also provided with `up_ixf`. The call to `up_ixf` takes the form

```
coef = up_ixf('basic',b,c,IIw,IIxf,indw)
```

Using `up_min` for the optimization you will have messages such as

```
Step size: 1.953e-03
      Cost      Parameter jumps ...
3.9341e-01  -9.83e+00  4.05e+00
```

which indicate reductions in the step size (`Up.copt(1,7)`) and values of the cost and update parameters at different stages of the optimization. With `Up.copt(1,2)` set to `11` you can follow the evolution of predictions of the first FRF in the considered set. The final result here is shown in the figure where the improvement linked to the update is clear.

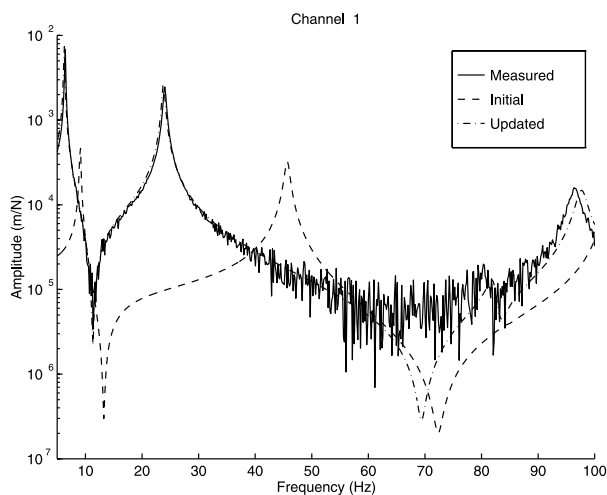


Figure 6.6: Updated FRF.

This algorithm is not very good and you are encouraged to use it as a basis for further study.

6.6 Handling models with piezoelectric materials

6.6.1 Theoretical elements

For piezo-electric shell elements, electric DOF correspond to the difference of potential on the electrodes while the corresponding load is the charge.

For volume elements, one defines multiple point constraints to enforce equal potential on nodes linked by a single electrode and sets one of the electrodes to zero potential (see section 6.6.3 for an example and section 6.1.5 for constitutive law information). During assembly the constraints are eliminated and the resulting model has electrical DOFs that correspond to differences of potential and loads to charge.

One thus generally considers models of the form

$$\begin{bmatrix} Z_{CC}(s) & Z_{CV} \\ Z_{VC} & Z_{VV} \end{bmatrix} \begin{Bmatrix} q_{mech} \\ V \end{Bmatrix} = \begin{Bmatrix} F_{mech} \\ Q \end{Bmatrix} \quad (6.109)$$

Open circuit (sensor), `data(j1,2)=1`, configurations correspond to cases where the charge remains zero and a potential is created on the electrodes due to mechanical deformations. It is then desirable to condense the electrical DOFs and define a sensor to measure the potential. A piezoelectric actuator driven using a charge amplifier would correspond to this configuration (but actuators are normally voltage driven).

Closed circuit, `data(j1,2)=0`, configurations correspond to cases where the potential is forced to zero (the electrical circuit is shorted). This corresponds to a case where the potential (electrical DOF) is fixed and the charge corresponds to the resulting force associated with this boundary condition. A piezoelectric **voltage driven actuator** is in this configuration.

`model=p_piezo('Assemble',model)` implements a manipulation of the model to eliminate the electrode DOFs and define the voltage sensors, for the open circuit case, and charge sensor and potential actuator, for the closed circuit case, as `SensDof` and `DofLoad` entries in the revised model stack. The calls are illustrated in the examples below.

The `'info','Electrode'` entry is a structure with fields

`.data` rows `NodeId IsOpen` gives the electrode nodes and for each one `1` if the circuit is open (sensor), and `0` if it is closed (actuator).

`.def .DOF`, `.lab_in` **only needed** when combining multiple electrodes into a single input. The `.lab_in` is a cell array of strings, you should end the string with `V` so that it shows Q for associated charge sensors.

Each column gives the weighting coefficients associated with each electrode. Thus `def=[1;0;1]` corresponds to a single equal input on electrodes 1 and 3. Note that it does not make sense to combine electrical DOFs that are of mixed nature (actuator/sensor).

The `.DOF` field should contain `NodeId+.21` since the potential corresponds to DOF `.21`.

The `.lab_in` field can be used to provide labels associated with each actuator/sensor defined as a column of `def`. You should end the label with `V` so that the collocated sensor ends with a `Q` label.

`.cta .lab` (optional) can be used to combine electrodes into sensors / actuators. Each row of `.cta` defines a sensor (with matching `.lab`). Each column corresponds to an electrode declared in the `.data` field. You cannot combine open and closed circuit electrodes. It is possible to use both a `.cta` and a `.def` field.

6.6.2 Sample plate with 4 piezo patches

This example was generated in collaboration with the ULB Active Structures Laboratory. It corresponds to a cantilevered composite plate with 4 piezo patches modeled using the `p_piezo Shell` formulation.

The following generates the 4 transfers between voltage input on the pairs of piezo patches and total charge output on the same pairs.

```
cf=ULB_plate_pzt('model feplot') % creates the model
% define the electrode state
ULB_plate_pzt('cantileverclosed',cf); %sdtweb ULB_plate_pzt('cantilever')
% combine electrodes 2 by 2
cf.Stack{'Electrodes'}.def=[1 -1 0 0;0 0 1 -1]';
cf.Stack{'Electrodes'}.lab_in={'UaV';'UbV'};
cf.Stack{'Electrodes'}.data(3:4,2)=0; % All in voltage actuation mode
cf.mdl=stack_set(cf.mdl,'info','DefaultZeta',.01);

[sys,TR]=fe2ss('free 5 10 0 -dterm',cf.mdl);

qbode(sys,linspace(30,300,512)*2*pi,'iplot "Closed"');
```



```

iicom submagpha

% Display response at a particular frequency
cf.Stack{'info','Freq'}=1;cf.def(1)=fe_simul('dfrf',cf);

% Other configuration with combined voltage input and combined voltage output
cf.Stack{'Electrodes'}.def=[1 -1 0 0]'; % Actuator
cf.Stack{'Electrodes'}.lab_in={'UaV'};
cf.Stack{'Electrodes'}.cta=[0 0 1 -1]'; % Sensor
cf.Stack{'Electrodes'}.lab={'UbV'};
cf.Stack{'Electrodes'}.data(3:4,2)=1; % short circuit sensor

[sys2,TR2]=fe2ss('free 5 10 0 -dterm',cf.mdl);
qbode(sys2,linspace(30,300,512)*2*pi,'iipplot "Mixed"');
cf.Stack{'info','Freq'}=1;cf.def(2)=fe_simul('dfrf',cf);
fecom(';show2def;scaleequal;scc-3000;view3')
% Note the slight difference associated to change to OC in sensor

```

One can look at the level of piezoelectric coupling by comparing the modal frequencies with the electrical circuits open or closed. One can see that modal frequencies are shifted down in the closed circuit case (where the difference of potential is set to 0).

```

model=ULB_plate_pzt('model') % creates the model
mo1=ULB_plate_pzt('cantileveropen',model);d1=fe_eig(mo1,[5 20 1e3]);
mo2=ULB_plate_pzt('cantileverclosed',model);d2=fe_eig(mo2,[5 20 1e3]);

r1=[d1.data(1:end)./d2.data(1:end)];
figure(1);plot(r1,'+');axis tight
xlabel('Mode number');ylabel('F_{OC}/F_{CC}');

```

6.6.3 Sample piezo with volume elements

SDT supports piezoelectric constitutive laws for all 3D volume elements (see [hexa8](#)). You just need to define a piezoelectric material as detailed in [p_piezo](#). A series of commands are implemented in [p_piezo](#) to help you handle standard electrode definition steps.

An automated procedure to define nodes whose potential is linked by a common electrode is implemented in [p_piezo](#) and illustrated in the [MFC_actuator](#) demo.

The generic call gives a name to the associated electrode and a selection for the associated nodes

```
[model,InputDOF]=p_piezo('ElectrodeMPC Name',model,FindNodeSel);
```

6.7 Viscoelastic modeling tools

The viscoelastic modeling tools are not part of the base SDT but licensed on an industrial basis only. Their documentation can be found at <http://www.sdtools.com/pdf/visc.pdf>.

6.8 SDT Rotor

Work on the integration of cyclic symmetry capabilities into a complete SDT ROTOR package is under progress. Their documentation can be found at <http://www.sdtools.com/pdf/rotor.pdf>.

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This chapter gives a detailed description of the formats used for variables and data structures. This information is grouped here and hypertext reference is given in the HTML version of the manual.

7.1 Nodes

7.1.1 Node matrix

Nodes are characterized using the convention of Universal files. `model.Node` and `FEnode` are node matrices. A node matrix has seven columns. Each row of gives

```
NodeId PID DID GID x y z
```

where `NodeId` are node numbers (positive integers with no constraint on order or continuity), `PID` and `DID` are coordinate system numbers for position and displacement respectively (zero or any positive integer), `GID` is a node group number (zero or any positive integer), and `x y z` are the coordinates. For cylindrical coordinate systems, coordinates represent `r teta z` (radius, angle in degrees, and z axis value). For spherical coordinates systems, they represent `r teta phi` (radius, angle from vertical axis in degrees, azimuth in degrees). For local coordinate system support see section 7.1.2.

A simple line of 10 nodes along the x axis could be simply generated by the command

```
node = [[1:10]' zeros(10,3) linspace(0,1,10)']*[1 0 0];
```

For other examples take a look at the finite element related demonstrations (see section 4.2) and the mesh handling utility `femesh`.

The **only restriction** applied to the `NodeId` is that they should be positive integers. The earlier limit of `round((2^31-1)/100) ≈ 21e6` is no longer applicable.

In many cases, you will want to access particular nodes by their number. The standard approach is to create a reindexing vector called `NNode`. Thus the commands

```
NNode=[];NNode(node(:,1))=1:size(node,1);  
Indices_of_Nodes = NNode(List_of_NodeId)
```

gives you a simple mechanism to determine the indices in the `node` matrix of a set of nodes with identifiers `List_of_NodeId`. The `futil FindNode` commands provide tools for more complex selection of nodes in a large list.

7.1.2 Coordinate system handling

Local coordinate systems are stored in a `model.bas` field (see `NodeBas`). Columns 2 and 3 of `model.Node` define respectively coordinate system numbers for position and displacement.

Use of local coordinate systems is illustrated in section 3.1.1 where a local basis is defined for test results.

`feplot`, `fe_mk`, `rigid`, ... now support local coordinates. `feutil` does when the model is described by a data structure with the `.bas` field. `femesh` assumes you are using global coordinate system obtained with

```
[FNode,bas] = basis(model.Node,model.bas)
```

To write your own scripts using local coordinate systems, it is useful to know the following calls:

`[node,bas,NNode]=feutil('getnodebas',model)` returns the nodes in global coordinate system, the bases `bas` with recursive definitions resolved and the reindexing vector `NNode`.

To obtain, the local to global transformation matrix (meaning $\{q_{global}\} = [c_{GL}]\{q_{local}\}$) use

```
cGL=basis('trans 1',model.bas,model.Node,model.DOF)
```

7.2 Model description matrices

A *model description matrix* describes the model elements. `model.Elt` and `FEelt` are, for example, model description matrices. The declaration of a finite element model is done through the use of element groups stacked as rows of a model description matrix `elt` and separated by header rows whose first element is `Inf` in Matlab or `zc%inf` in Scilab and the following are the ASCII values for the name of the element. In the following, Matlab notation is used. Don't forget to replace `Inf` by `zc%inf` in Scilab.

For example a model described by

```
elt = [Inf abs('beam1')                0 0
       1  2  11  12  5                0 0 0
       2  3  11  12  5                0 0 0
       Inf abs('mass1')                0 102
       2  1e2 1e2 1e2  5e-5 5e-5 5e-5  0 ];
```

has 2 groups. The first group contains 2 `beam1` elements between nodes 1-2 and 2-3 with material property 11, section property 12, and bending plane containing node 5. The second group contains a concentrated mass on node 2.

Note how columns unused for a given type element are filled with zeros. The 102 declared for the mass corresponds to an element group identification number `EGID`.

You can find more realistic examples of model description matrices in the demonstrations (see section 4.2).

The general format for **header rows** is

```
[Inf abs('ElementName') 0 opt ]
```

The `Inf` that mark the element row and the `0` that mark the end of the element name are **required** (the `0` may only be omitted if the name ends with the last column of `elt`).

For multiplatform compatibility, **element names** should only contain lower case letters and numbers. In any case never include blanks, slashes, ... in the element name. Element names reserved for supported elements are listed in the element reference chapter 8 (or `doc('eltfun')` from the command line).

Users can define new elements by creating functions (`.m` or `.mex` in Matlab, `.sci` in Scilab) files with the element name. Specifications on how to create element functions are given in section 7.16.

Element group options `opt` can follow the zero that marks the end of the element name. `opt(1)`, if used, should be the element group identification number `EGID`. In the example, the group of `mass1` elements is this associated to the `EGID` 102. The default element group identification number is its order in the group declaration. Negative `EGID` are ignored in FEM analyzes (display only, test information, ...).

Between group headers, each row describes an element of the type corresponding to the previous header (first header row above the considered row).

The general format for **element rows** is

```
[NodeNumbers MatId ProId EltId OtherInfo]
```

where

- `NodeNumbers` are positive integers which must match a unique `NodeId` identifier in the first column of the node matrix.

- **MatId** and **ProId** are material and element property identification numbers. They should be positive integers matching a unique identifier in the first column of the material **pl** and element **il** property declaration matrices.
- **EltId** are positive integers uniquely identifying each element. See **feutil** **EltId** for a way to return the vector and verify/fix identifiers.
- **OtherInfo** can for example be the node number of a reference node (**beam1** element). These columns can be used to store arbitrary element dependent information. Typical applications would be node dependent plate thickness, offsets, etc.

Note that the position of **MatId**, **ProId** and **EltId** in the element rows are returned by calls of the form `ind=elem0('prop')` (`elem0` is a generic element name, it can be `bar1`, `hexa8`, ...).

Element property rows are used for assembly by `fe_mk`, display by `feplot`, model building by `femesh`, ...

7.3 Material property matrices and stack entries

This section describes the low level format for material properties. The actual formats are described under `m_` functions `m_elastic`, `m_piezo`, ... For Graphical edition and standard scripts see section 4.2.1.

A material is normally defined as a row in the *material property matrix* **pl**. Such rows give a declaration of the general form `[MatId Type Prop]` with

MatId	a positive integer identifying a particular material property.
Type	a positive real number built using calls of the form <code>fe_mat('m_elastic','SI',subtype)</code> , the <code>subtype</code> integer is described in <code>m_</code> functions.
Prop	as many properties (real numbers) as needed (see <code>fe_mat</code> , <code>m_elastic</code> for details).

Additional information can be stored as an entry of type `'mat'` in the model stack which has data stored in a structure with at least fields

<code>.name</code>	Description of material.
<code>.pl</code>	a single value giving the <code>MatId</code> of the corresponding row in the <code>model.pl</code> matrix or row of values. Resolution of the true <code>.pl</code> value is done by <code>pl=fe_mat('getpl',model)</code> . The property value in <code>.pl</code> should be <code>-1</code> for interpolation in <code>GetPl</code> , <code>-2</code> for interpolation using the table at each integration point, <code>-3</code> for direct use of a <code>FieldAtNode</code> value as constitutive value.
<code>.unit</code>	a two character string describing the unit system (see <code>fe_mat Convert</code> and <code>Unit</code> commands).
<code>.type</code>	the name of the material function handling this particular type of material (for example <code>m_elastic</code>).
<code>.field</code>	can be a structure allowing the interpolation of a value called <code>field</code> based on the given table. Thus <code>mat.E=struct('X',[-10;20], 'Xlab',{{'T'}}, 'Y',[10 20]*1e6)</code> will interpolate value <code>E</code> based on field <code>T</code> . The positions of interpolated variables within the <code>pl</code> row are given by <code>list=feval(mat.type, 'propertyunittype cell', subtype)</code> .

7.4 Element property matrices and stack entries

This section describes the low level format for element properties. The actual formats are described under `p_` functions `p_shell`, `p_solid`, `p_beam`, `p_spring`. For Graphical edition and standard scripts see section 4.2.1.

An element property is normally defined as a row in the *element property matrix* `il`. Such rows give a declaration of the general form `[ProId Type Prop]` with

<code>ProId</code>	a positive integer identifying a particular element property.
<code>Type</code>	a positive real number built using calls of the form <code>fe_mat('p_beam','SI',1)</code> , the <code>subtype</code> integer is described in the <code>p_</code> functions.
<code>Prop</code>	as many properties (real numbers) as needed (see <code>fe_mat</code> , <code>p_solid</code> for details).

Additional information can be stored as an entry of type `'pro'` in the model stack which has data stored in a structure with fields

<code>.name</code>	description of property.
<code>.il</code>	a single value giving the <code>ProId</code> of the corresponding row in the <code>il</code> matrix or row of values Resolution of the true <code>.il</code> value is done by <code>il=fe_mat('getil',model)</code> . The property value in <code>.il</code> should be <code>-1</code> for interpolation in <code>GetIl</code> , <code>-2</code> for interpolation using the table at each integration point, <code>-3</code> for direct use of a <code>FieldAtNode</code> value as constitutive value.
<code>.unit</code>	a two character string describing the unit system (see the <code>fe_mat Convert</code> and <code>Unit</code> commands)
<code>.type</code>	the name of the property function handling this particular type of element properties (for example <code>p_beam</code>)
<code>.NLdata</code>	used to stored non-linear property information
<code>.MAP</code>	specifications of a field at node, see section 7.13
<code>.gstate</code>	specifications of a field at integration points, see section 7.13
<code>.field</code>	can be a structure allowing the interpolation of a value called <code>field</code> based on the given table. Thus <code>pro.A=struct('X',[-10;20],'Xlab',{{'x'}},'Y',[10 20]*1e6)</code> will interpolate value <code>A</code> based on field <code>x</code> . The positions of interpolated variables within the <code>il</code> row are given by <code>list=feval(pro.type, 'propertyunittypetype cell', subtype)</code> .

The handling of a particular type of constants should be fully contained in the `p_*` function. The meaning of various constants should be defined in the help and TEX documentation. The subtype mechanism can be used to define several behaviors of the same class. The generation of the `integ` and `constit` vectors should be performed through a `BuildConstit` call that is the same for a full family of element shapes. The generation of `EltConst` should similarly be identical for an element family.

7.5 DOF definition vector

The meaning of each Degree of Freedom (DOF) is handled through DOF definition vectors typically stored in `.DOF` fields (and columns of `.dof` in test cases where a DOF specifies an input/output location). All informations defined at DOFs (deformations, matrices, ...) should always be stored with the corresponding DOF definition vector. The `fe_c` function supports all standard DOF manipulations (extraction, conversion to label, ...)

Nodal DOFs are described as a single number of the form `NodeId.DofId` where `DofId` is an integer between `01` and `99`. For example DOF 1 of node 23 is described by `23.01`. By convention

- DOFs `01` to `06` are, in the following order u, v, w (displacements along the global coordinate axes) and $\theta_u, \theta_v, \theta_w$ (rotations along the same directions)
- DOFs `07` to `12` are, in the following order $-u, -v, -w$ (displacements along the reversed global coordinate axes) and $-\theta_u, -\theta_v, -\theta_w$ (rotations along the same directions). This convention is used in test applications where measurements are often made in those directions and not corrected for the sign change. It should not be used for finite element related functions which may not all support this convention.

While these are the only mandatory conventions, other typical DOFs are `.19` pressure, `.20` temperature, `.21` voltage, `.22` magnetic field.

In a small shell model, all six DOFs (translations and rotations) of each node would be retained and could be stacked sequentially node by node. The DOF definition vector `mdof` and corresponding displacement or load vectors would thus take the form

$$\text{mdof} = \begin{bmatrix} 1.01 \\ 1.02 \\ 1.03 \\ 1.04 \\ 1.05 \\ 1.06 \\ \vdots \end{bmatrix}, \mathbf{q} = \begin{bmatrix} u_1 & u_2 & & \\ v_1 & v_2 & & \\ w_1 & w_2 & & \\ \theta_{u1} & \theta_{u2} & \cdots & \\ \theta_{v1} & \theta_{v2} & & \\ \theta_{w1} & \theta_{w2} & & \\ \vdots & & \ddots & \end{bmatrix} \text{ and } \mathbf{F} = \begin{bmatrix} F_{u1} & F_{u2} & & \\ F_{v1} & F_{v2} & & \\ F_{w1} & F_{w2} & & \\ M_{u1} & M_{u2} & \cdots & \\ M_{v1} & M_{v2} & & \\ M_{w1} & M_{w2} & & \\ \vdots & & \ddots & \end{bmatrix}$$

Typical vectors and matrices associated to a DOF definition vector are

- **modes** resulting from the use of `fe_eig` or read from FE code results (see `nasread`, `ufread`).
- **input and output shape matrices** which describe how forces are applied and sensors are placed (see `fe_c`, `fe_load`, `bc` page 152).
- **system matrices** : mass, stiffness, etc. assembled by `fe_mk`.
- **FRF** test data. If the position of sensors is known, it can be used to animate experimental deformations (see `feplot` , `xfopt`, and `fe_sens`).

Note that, in Matlab version, the functions `fe_eig` and `fe_mk`, for models with more

than 1000 DOFs, renumber DOF internally so that you may not need to optimize DOF numbering yourself. In such cases though, `mdof` will not be ordered sequentially as shown above.

Element DOFs are described as a single number of the form `-EltId.DofId` where `DofId` is an integer between `001` and `999`. For example DOF 1 of the element with ID `23001` is described by `-23001.001`. Element DOFs are typically only used by superelements (see section 6.3). Due to the use of integer routines for indexing operations, you cannot define element DOFs for elements with and `EltId` larger than 2 147 484.

7.6 FEM model structure

Finite element simulations are best handled using standard data structures supported by *OpenFEM*. The two main data structures are `model` which contains information needed to specify a FEM problem, and `DEF` which stores a solution.

Finite element models are described by their topology (nodes, elements and possibly coordinate systems), their properties (material and element). Computations performed with a model are further characterized by a case as illustrated in section 4.2.3 and detailed in section 7.7.

Data structures describing finite element models have the following standardized fields, where only nodes and elements are always needed.

<code>.bas</code>	local coordinate system definitions.
<code>.cta</code>	sensor observation matrix. Used by <code>fe_sens</code> .
<code>.copt</code>	solver options. For use by <code>upcom</code> . This field is likely to disappear in favor of defaults in <code>sdtdef</code> .
<code>.DOF</code>	DOF definition vector for the matrices of the model. Boundary conditions can be imposed using cases.
<code>.Elt</code>	elements. This field is mandatory .
<code>.file</code>	Storage file name. Used by <code>upcom</code> .
<code>.il</code>	element property description matrix. Can also be stored as <code>'pro'</code> entries in the <code>Stack</code> .
<code>.K{i}</code>	cell array of constant matrices for description of model as a linear combination. Indices <code>i</code> match definitions in <code>.Opt(2,:)</code> and <code>.Opt(3,:)</code> . Should be associated with a <code>.Klab</code> field giving a string definition of each matrix. See details in the <code>fe_super</code> reference.
<code>.mind</code>	element matrix indices. Used by <code>upcom</code> .
<code>.Node</code>	nodes. This field is mandatory .
<code>.Opt</code>	options characterizing models that are to be used as superelements.
<code>.pl</code>	material property description matrix. Can also be stored as <code>'mat'</code> entries in the <code>Stack</code> .
<code>.Patch</code>	Patch face matrix. See <code>fe_super</code> .
<code>.Stack</code>	A cell array containing optional properties further characterizing a finite element model. See <code>stack_get</code> for how to handle the stack and the next section for a list of standardized entries.
<code>.TR</code>	projection matrix. See <code>fe_super</code> .
<code>.unit</code>	main model unit system (see <code>fe_mat Convert</code> for a list of supported unit systems and the associated two letter codes). Specifying this field let you perform conversion from materials defined in <code>US</code> system unit from the GUI.
<code>.wd</code>	working directory

Obsolete fields are `.Ref` Generic coordinate transformation specification, `.tdof` test DOF field (now in `SensDof` entries).

7.7 FEM stack and case entries

Various information are stored in the `model.Stack` field. If you use a `SDT handle` referring to a `feplot` figure, modification of the model and case entries is often easier using `cf.Stack` calls (see `feplot`).

Currently supported entry types in the stack are

<code>case</code>	defines a case : boundary conditions, loading, ...
<code>curve</code>	curve to be used for simulations (see fe_curve).
<code>info</code>	non standard information used by solvers or meshing procedures (see below).
<code>info,map</code>	used to define a normal MAP, see feutil GetNormal for format
<code>mat</code>	defines a material entry.
<code>pro</code>	defines an element property entry.
<code>SE</code>	defines a superelement entry.
<code>sel</code>	defines a element selection.
<code>seln</code>	defines a node selection. Typically a structure with fields <code>.ID</code> giving the reference number and <code>.data</code> giving either node numbers or a node selection command.
<code>set</code>	defines a set that is a structure with fields <code>.ID</code> (a reference number of the set), <code>.data</code> (defines the data) and <code>.type</code> (nature of the set: NodeId , EltId , FaceId , EdgeId or DOF). <code>.data</code> contains NodeId for nodes, EltId for elements, two columns giving EltId and face/edge number (as detailed in integrules) for faces and edges, DOF values for DOF sets. Sets are often used to define loaded surfaces. The feutil AddSet commands let you define a set from a selection. For FaceId sets, a third column can be added to specify subgroups within the set and a <code>.NodeCon</code> sparse matrix can be used to specify nodes (rows) connected to each subgroup (column).

Currently reserved names for `info` entries are

<code>DefaultZeta</code>	value to be used as default modal damping ratio (viscous damping). The default loss factor if needed is taken to be twice that value.
<code>EigOpt</code>	gives real eigenvalue solver options (see <code>fe_eig</code>).
<code>FluidEta</code>	Default loss factor for use in vibroacoustic fluid computations.
<code>Freq</code>	Frequencies given as a structure with field <code>.data</code> with frequency values and <code>.ID</code> a integer identifier. A <code>.unit</code> field can specify <code>rad/s,Hz,rev/mn,RPM</code> . <code>f=fe_def('DefFreq',model)</code> is used to obtain the frequency vector in Hz.
<code>NewNodeFrom</code>	integer giving the next <code>NodeId</code> to be used when adding nodes to the model (used by some commands of <code>feutil</code>).
<code>Omega</code>	rotation vector used for rotating machinery computations (see <code>fe_cyclic</code>) can be specified as a structure for unit selection. For example <code>r1=struct('data',250,'unit','RPM');f_hz=fe_def('deffreq'</code>
<code>OrigNumbering</code>	original node numbering (associated with <code>feutil Renumber</code> command). Two <code>int32</code> columns giving original and new node numbers.
<code>StressCritFcn</code>	string to be evaluated for a specific stress criterion, see <code>fe_stress</code> .
<code>Rayleigh</code>	defines a Rayleigh damping entry.
<code>MifDes</code>	defines the list of desired response output (see <code>fe2xf</code>).
<code>NasJobOpt</code>	structure with options to be used for automated job runs by the NASTRAN job handler.
<code>NastranJobEdit</code>	cell array giving a list of job editing commands to be used through a <code>naswrite EditBulk</code> call.
<code>TimeOpt</code>	gives time solver options (see <code>fe_time</code>).

A `case` defines finite element boundary conditions, applied loads, physical parameters, ... The associated information is stored in a `case` data structure with fields

<code>Case.Stack</code>	list of boundary conditions, constraints, parametric design point, and loading cases that need to be considered. A table of accepted entries is given under <code>fe_case</code> . Each row gives <code>{Type,Name,data}</code> .
<code>Case.T</code>	basis of subspace verifying fixed boundary conditions and constraints.
<code>Case.DOF</code>	DOF definition vector describing the columns of <code>T</code> , the rows of <code>T</code> are described by the <code>.DOF</code> field of the model.

The various cases are then stored in the `.Stack` field of the model data structure

(this is done by a call to `fe_case`). If you use a *SDT handle* referring to a `feplot` figure, modification of the case entries is often easier using `cf.CStack` calls (see `feplot`).

7.8 FEM result data structure

Deformations resulting from finite element computations (`fe_eig`, `fe_load`, ...) are described by `def` structures with fields

<code>.def</code>	deformations (<i>NDOF</i> by <i>NDef</i> matrix)
<code>.DOF</code>	DOF definition vector
<code>.data</code>	(optional) (<i>NDef</i> by <i>Ninfo</i> vector or matrix) characterizing the content of each deformation (frequency, time step, ...)
<code>.fun</code>	function description [Model Analysis Field FieldType Format NDV]. This is based on the UNV 55 format detailed below. Typically field with [<code>0 fe_curve('TypeAnalysis')</code>]. This field is needed for proper automated display setup.
<code>.lab</code>	(optional) cell array of strings characterizing the content of each deformation (columns of <code>.def</code>). For large arrays, the use of a <code>.LabFcn</code> is preferable.
<code>.ImWrite</code>	(optional) can be used to control automated multiple figure generation, see <code>iicom ImWrite</code> .
<code>.LabFcn</code>	callback for label generation see <code>fecom LabFcn</code>
<code>.Legend</code>	data for legend generation, see <code>fecom Legend</code>
<code>.label</code>	(optional) string describing the content
<code>.DofLab</code>	optional cell array of strings specifying a label for each DOF. This is used for display in <code>iipplot</code> .
<code>.scale</code>	field used by <code>feplot</code> to store scaling information.

The `.fun` field is a numeric row with values (a typical value for static responses is `def.fun=[0 1 0]`)

Model (0 Unknown, 1 Structural, 2 Heat Transfer, 3 Fluid Flow)

Analysis see list with `fe_curve('TypeAnalysis')`

Field see list with 0: Unknown (or general SDT), 1: Scalar, 2: Tx Ty Tz, 3: Tx Ty

Tz Rx Ry Rz, 4: Sxx Sxy Syy Sxz Syz Szz, 5: Sxx Syx Szx Sxy Syy Szy Sxz
Syz Szz

FieldType see list with `fe_curve('typefield')`

Format 0 default, 2 Real, 5 Complex

NDV Number Of Data Values Per Node (0 for variable number)

SDT provides a number of utilities to manipulate deformation structures. In particular you should use

- `def=fe_def('subdef',def,ind)` extracts some deformations (columns of `def.def`). You can select based on the data field, for example with `ind=def.data(:,1)>100`.
- `def=fe_def('AppendDef',def,def1)` combines two sets of deformations
- `def=fe_def('SubDof',def,DOF)` extracts some DOF (rows of `def.def`). To select based on DOF indices, use `def=fe_def('SubDofInd',def,ind)`.
- `def=feutilb('placeindof',DOF,def)` is similar but `DOF` may be larger than `def.DOF`.
- `fe_def('SubDofInd-Cell',def,ind_dof,ind_def)` return clean display of deformation as a cell array.

7.9 Curves and data sets

Curves are used to specify inputs (for time or frequency domain simulation) and store results from simulations. They can be stored as entries `{'curve', Name, data}` in the model stack or in the case of inputs in the `load.curve` cell array. Curves can be displayed using the `iiplot` interface.

A curve can be used to define a time (or frequency) dependent load $\{F\} = [B] \{u\}$. $[B]$ defines the spatial distribution of the load on DOFs and its unit is the same as F . $[B]$ is defined by a `DOFLoad` entry in the Case. $\{u\}$ defines the time (or frequency) dependency as a unitless curve. There should be as many curves as columns in the

matrix of a given load `def`. If a single curve is defined for a multi-load entry, it will affect all the loads of this entry.

As an illustration, let us consider ways to define a time dependent load by defining a `.curve` field in the load data structure. This field may contain a string referring to an existing curve (name is `'input'` here)

```
model=fe_time('demo bar');fe_case(model,'info')

% redefine curve
model=fe_curve(model,'set','input','TestStep 1e-3');
% have load reference that curve
model=fe_case(model,'setcurve','Point load 1','input');

TimeOpt=fe_time('timeopt newmark .25 .5 0 1e-4 100');
model=stack_set(model,'info','TimeOpt',TimeOpt);
def=fe_time(model); feplot(model,def); fecom ColorDataAll
```

or a data structure generated by `fe_curve`

```
model=fe_time('demo bar');fe_case(model,'info')
model=fe_case(model,'remove','fd'); % loads at both ends
data=struct('DOF',[1.01;2.01],'def',1e6*eye(2),...
           'curve',{{'test ricker 10e-4 1',...
                    'test ricker 20e-4 1'}});
model = fe_case(model,'DOFLoad','Point load 1',data);

TimeOpt=fe_time('timeopt newmark .25 .5 0 1e-4 100');
model=stack_set(model,'info','TimeOpt',TimeOpt);
def=fe_time(model); feplot(model,def); fecom ColorDataAll
```

Multi-dim curve

A curve is a `data` structure with fields

<code>.ID</code>	Optional. It can be used to generate automatically vertical lines in <code>iiplot</code> . See <code>ii_plp Call from iiplot</code> for more details.
<code>.X</code>	axis data. A cell array with as many entries as dimensions of <code>.Y</code> . Contents of each cell can be a vector (for example vector of frequencies or time steps), a cell array describing data vectors in <code>.Y</code> (for example response labels) with as many rows as elements in corresponding dimension of <code>.Y</code> . (obsolete) x-axis data.
<code>.Xlab</code>	a cell array giving the meaning of each entry in <code>.X</code> . Each cell can be a string (giving the dimension name) or itself a cell array with columns giving <code>{'name','UnitString',unitcode,'fmt'}</code> . Typical entries are obtained using the <code>fe_curve datatypecell</code> command. Multiple rows can be used to describe multiple columns in the <code>.X</code> entry. <code>fmt</code> , if provided, gives a formatting instruction for example <code>'length=%i m'</code> . If more intricate formatting is needed a callback can be obtained with <code>\zs{'#st3{'}}=sprintf('PK=%.2fkm',r2(j2)*1e-3);'</code> .
<code>.Y</code>	response data. If a matrix rows correspond to <code>.X</code> values and columns are called <i>channels</i> .
<code>.Ylab</code>	describes content of <code>.Y</code> data. It can be a string, a 1x3 unit type cell array, or a number that indicates which dimension (index in <code>.X</code> field cell array) describes the <code>.Y</code> unit.
<code>.name</code>	name of the curve.
<code>.type</code>	<code>'fe_curve'</code> .
<code>.Interp</code>	optional interpolation method. Available interpolations are <code>linear</code> , <code>log</code> and <code>stair</code> .
<code>.Extrap</code>	optional extrapolation method. Available extrapolations are <code>flat</code> , <code>zero</code> and <code>exp</code> .
<code>.PlotInfo</code>	indications for automated plotting, see <code>iiplot PlotInfo</code>
<code>.DimPos</code>	order of dimensions to be plot.

For other use, following type can exist.

Response data

Response data sets correspond to groups of universal files of type `UFF58` that have the same properties (type of measurement, abscissa, units, ...). They are used for identification with `idcom` while the newer curve format is used for simulation results.

They are characterized by the following fields

<code>.w</code>	abscissa values
<code>.xf</code>	response data, one column per response, see section 5.8
<code>.dof</code>	characteristics of individual responses (one row per column in the response data as detailed below)
<code>.fun</code>	general data set options, contain <code>[FunType DFormat NPoints XSpacing Xmin XStep ZValue]</code> as detailed in <code>ufread 58</code> .
<code>.idopt</code>	options used for identification related routines (see <code>idopt</code>)
<code>.header</code>	header (5 text lines with a maximum of 72 characters)
<code>.x</code>	abscissa description (see <code>xfopt('_datatype')</code>)
<code>.yn</code>	numerator description (see <code>xfopt('_datatype')</code>)
<code>.yd</code>	denominator description (see <code>xfopt('_datatype')</code>)
<code>.z</code>	third axis description (see <code>xfopt('_datatype')</code>)
<code>.group</code>	(optional) cell array containing DOF group names
<code>.load</code>	(optional) loading patterns used in the data set

The `.w` and `.xf` fields contain the real data while other fields give more precisions on its nature.

The `.dof` field describes *DOF/channel dependent options* of a MIMO data set. The `dof` field contains one row per response/DOF with the following information (this corresponds to data in line 6 of `ufread 58` except for address)

```
[RespNodeID.RespDOFID ExciNodeID.ExciDOFID Address ...
RespGroupID ExciGroupID FunID LoadCase ZaxisValue]
```

- Standard *DOF definitions* of the form `NodeID.DOFID` are introduced in section 7.5. When considering sensors in general directions (see section 4.3) the `SensId` should match `RespNodeID.RespDOFID`.
- *Addresses* are integer numbers used to identify columns of `xf` matrices. They typically correspond to a measurement number.
- Sensor / actuator *groups* are correspond to the group names given in the `group` field (this is really only supported by `ufread`).
- Other columns are given in the universal format specification but unused in SDT.

The `idopt` field is used to point to identification options used on the data set. These should point to the figure options `ci.IDopt`.

The `Group` field is used to associate a name to the group identification numbers `RespGroupID` `ExciGroupID` defined in the `.dof` columns 4 and 5. These names are saved by `ufwrite` but currently not used in other parts of the *SDT*.

The `load` field describes *loading cases* by giving addresses of applied loads in odd columns and the corresponding coefficients in even columns. This field is used in test cases with multiple correlated inputs.

Shapes at DOFs

Shapes at DOFs is used to store modeshapes, time responses defined at all nodes, ... and are written to universal file format 55 (response at nodes) by `ufwrite`. The fields used for such datasets are

<code>.po</code>	pole values, time steps, frequency values ... For poles, see <code>ii_pof</code> which allows conversions between the different pole formats.
<code>.res</code>	residues / shapes (one row per shape). Residue format is detailed in section 5.6.
<code>.dof</code>	characteristics of individual responses (follow link for description).
<code>.fun</code>	function characteristics (see <code>UFF58</code>)
<code>.header</code>	header (5 text lines with a maximum of 72 characters)
<code>.idopt</code>	identification options. This is filled when the data structure is obtained as the result of an <code>idcom</code> call.
<code>.label</code>	string describing the content
<code>.lab_in</code>	optional cell array of names for the inputs
<code>.lab_out</code>	optional cell array of names for the outputs
<code>.group</code>	optional cell group names

FEM Result

See section 7.8.

7.10 DOF selection

`fe_c` is the general purpose function for manipulating DOF definition vectors. It is called by many other functions to select subsets of DOFs in large DOF definition vectors. DOF selection is very much related to building an observation matrix `c`, hence the name `fe_c`.

For DOF selection, `fe_c` arguments are the reference DOF vector `mdof` and the DOF selection vector `adof`. `adof` can be a standard DOF definition vector but can also contain wild cards as follows

`NodeId.0` means all the DOFs associated to node `NodeId`
`0.DofId` means `DofId` for all nodes having such a DOF
`-EltN.0` means all the DOFs associated to element `EltId`

Typical examples of DOF selection are

`ind = fe_c(mdof,111.01,'ind');` returns the position in `mdof` of the x translation at node 111. You can thus extract the motion of this DOF from a vector using `mode(ind,:)`. Note that the same result would be obtained using an output shape matrix in the command `fe_c(mdof,111.01)*mode`.

`model = fe_mk(model,'FixDOF','2-D motion',[.03 .04 .05])`

assembles the model but only keeps translations in the xy plane and rotations around the z axis (DOFs `[.01 .02 .06]`). This is used to build a 2-D model starting from 3-D elements.

The `feutil FindNode` commands provides elaborate node selection tools. Thus `femesh('findnode x>0')` returns a vector with the node numbers of all nodes in the standard global variable `FENode` that are such that their x coordinate is positive. These can then be used to select DOFs, as shown in the section on boundary conditions section 7.14. Node selection tools are described in the next section.

7.11 Node selection

`feutil FindNode` supports a number of node selection criteria that are used by many functions. A node selection command is specified by giving a string command (for example `'GroupAll'`, or the equivalent cell array representation described at the end of this section) to be applied on a model (nodes, elements, possibly alternate element set).

Output arguments are the numbers `NodeId` of the selected nodes and the selected nodes `node` as a second optional output argument. The basic commands are

- `[NodeId,node]=feutil(['findnode ...'],model)` or `node=feutil(['getnode ...'],model)`
this command applies the specified node selection command to a `model` structure. For example, `[NodeId,node] = feutil('findnode x==0',model);` selects the nodes in `model.Node` which first coordinate is null.
- `[NodeId,node]=femesh(['findnode ...'])`
this command applies the specified node selection command to the standard global matrices `FENode`, `FEelt`, `FEel0`, ... For example, `[NodeId,node] = femesh('findnode x==0');` selects the node in `FENode` which first coordinate is null.

Accepted selectors are

<code>GID <i>i</i></code>	selects the nodes in the node group <i>i</i> (specified in column 4 of the node matrix). Logical operators are accepted.
<code>Group <i>i</i></code>	selects the nodes linked to elements of group(s) <i>i</i> in the main model. Same as <code>InElt{Group <i>i</i>}</code>
<code>Groupa <i>i</i></code>	selects nodes linked to elements of group(s) <i>i</i> of the alternate model
<code>InElt{<i>sel</i>}</code>	selects nodes linked to elements of the main model that are selected by the element selection command <i>sel</i> .
<code>NodeId ><i>i</i></code>	selects nodes based relation of <code>NodeId</code> to integer <i>i</i> . The logical operator <code>></code> , <code><</code> , <code>>=</code> , <code><=</code> , <code>~=</code> , or <code>==</code> can be omitted (the default is then <code>==</code>).
<code>NotIn{<i>sel</i>}</code>	<code>feutil('findnode 1 2',model)</code> interprets the values as <code>NodeId</code> unless three values are given (then interpreted as <code>x y z</code>). <code>feutil('findnode',model,IdList)</code> should then be used. selects nodes not linked to elements of the main model that are selected by the element selection command <i>sel</i> .
<code>Plane == <i>i nx ny nz</i></code>	selects nodes on the plane containing the node number <i>i</i> and orthogonal to the vector <code>[<i>nx ny nz</i>]</code> . Logical operators apply to the oriented half plane. <i>i</i> can be replaced by string <code>o xo yo zo</code> specifying the origin.

<code>rad <=r x y z</code>	selects nodes based on position relative to the sphere specified by radius <code>r</code> and position <code>x y z</code> node or number <code>x</code> (if <code>y</code> and <code>z</code> are not given). The logical operator <code>></code> , <code><</code> , <code>>=</code> , <code><=</code> or <code>==</code> can be omitted (the default is then <code><=</code>).
<code>cyl <=r i nx ny nz z1 z2</code>	selects nodes based on position relative to the cylinder specified by radius <code>r</code> and axis of direction <code>nx ny nz</code> and origin the node <code>i</code> (NodeId <code>i</code> can be replaced by string <code>o xo yo zo</code>). Optional arguments <code>z1</code> and <code>z2</code> define bottom and top boundaries from origin along cylinder axis.
<code>Setname name</code>	finds nodes based on a set defined in the model stack. Note that the name must not contain blanks or be given between double quotes <code>"name"</code> . Set can be a <code>NodeId</code> or even an <code>EltId</code> or <code>FaceId</code> , <code>EdgeId</code> set. <code>"name:con IdList"</code> can be used to select a subset connected to nodes in the <code>IdList</code> .
<code>x>a</code>	selects nodes such that their x coordinate is larger than <code>a</code> . <code>x y z r</code> (where the radius <code>r</code> is taken in the <code>xy</code> plane) and the logical operators <code>></code> , <code><</code> , <code>>=</code> , <code><=</code> , <code>==</code> can be used. Expressions involving other dimensions can be used for the right hand side. For example <code>r>.01*z+10</code> .
<code>x y z</code>	selects nodes with the given position. If a component is set to <code>NaN</code> it is ignored. Thus <code>[0 NaN NaN]</code> is the same as <code>x==0</code> .

Element selectors `EGID`, `EltId`, `EltName`, `MatId` and `ProId` are interpreted as `InElt` selections.

Command option `eps1 value` can be used to give an evaluation tolerance for equality logical operators.

Different selectors can be chained using the logical operations `&` (finds nodes that verify both conditions), `|` (finds nodes that verify one or both conditions). Condition combinations are always evaluated from left to right (parentheses are not accepted).

While the string format is typically more convenient for the user, the reference format for a node selection is really a 4 column cell array :

```
{      Selector      Operator      Data
Logical Selector      Operator      Data
}
```

The first column gives the chaining between different rows, with `Logical` being either `&`, `|` or a bracket (and). The `Selector` is one of the accepted commands for node selection (or element selection if within a bracket). The `operator` is a logical operator `>`, `<`, `>=`, `<=`, `~=`, or `==`. The `data` contains numerical or string values that

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are used to evaluate the operator. Note that the meaning of `~=` and `==` operators is slightly different from base MATLAB operators as they are meant to operate on sets.

The `feutil FindNodeStack` command returns the associated cell array rather than the resulting selection.

7.12 Element selection

`feutil FindElt` supports a number of element selection criteria that are used by many functions. An element selection command is specified by giving a string command (for example `'GroupAll'`) to be applied on a model (nodes, elements, possibly alternate element set).

Basic commands are :

- `[eltind,elt] = feutil('findelt selector',model);`
or `elt = feutil('selelt selector',model);` this command applies the specified element selection command to a `model` structure. For example, `[eltind,selelt] = feutil('findelt eltname bar1',model)` selects the elements in `model.Elt` which type is `bar1`.
- `[eltind,elt] = feutil('findelt selector',model);`
this command applies the specified element selection command to the standard global matrices `FEnode`, `FEelt`, `FEel0`, ... For example, `[eltind,selelt] = feutil('findelt eltname bar1',model)` selects the elements in `FEelt` which type is `bar1`.

Output arguments are `eltind` the selected elements indices in the element description matrix and `selelt` the selected elements.

Accepted selectors are

<code>ConnectedTo</code>	<code>i</code>	finds elements in a group that contains the nodes <code>i</code> . This calls <code>feutil DivideInGroups</code> and thus only operates on groups of elements of a single type.
<code>EGID</code>	<code>i</code>	finds elements with element group identifier <code>i</code> . Operators accepted.
<code>EltId</code>	<code>i</code>	finds elements with identifiers <code>i</code> in <code>FEelt</code> . Operators accepted.
<code>EltInd</code>	<code>i</code>	finds elements with indices <code>i</code> in <code>FEelt</code> . Operators accepted.
<code>EltName</code>	<code>s</code>	finds elements with element name <code>s</code> . <code>EltName flui</code> will select all elements with name starting with <code>flui</code> . <code>EltName ~ = flui</code> will select all elements with name not starting with <code>flui</code> . One can select superelements from their name using <code>EltName SE:SEName</code> .
<code>Facing</code>	<code>> cos</code> <code>x y z</code>	finds topologically 2-D elements whose normal projected on the direction from the element CG to <code>x y z</code> has a value superior to <code>cos</code> . Inequality operations are accepted.
<code>Group</code>	<code>i</code>	finds elements in group(s) <code>i</code> . Operators accepted.
<code>InNode</code>	<code>i</code>	finds elements with all nodes in the set <code>i</code> . Nodes numbers in <code>i</code> can be replaced by a string between braces defining a node selection command. For example <code>feutil('FindElt withnode {y>-230 & NodeId>1000}',model)</code> .
<code>MatId</code>	<code>i</code>	finds elements with <code>MatId</code> equal to <code>i</code> . Relational operators are also accepted (<code>MatId =1:3, ...</code>).
<code>ProId</code>	<code>i</code>	finds elements with <code>ProId</code> equal to <code>i</code> . Operators accepted.
<code>SelEdge</code>	<code>type</code>	selects the external edges (lines) of the currently selected elements (any element selected before the <code>SelEdge</code> selector), any further selector is applied on the model resulting from the <code>SelEdge</code> command rather than on the original model. The <code>-KeepAll</code> option skips the internal edge elimination step. Type <code>g</code> retains inter-group edges. <code>m</code> retains inter-material edges. Type <code>p</code> retains inter-property edges. <code>all</code> retains all edges. The <code>MatId</code> for the resulting model identifies the original properties of each side of the edge.
<code>SelFace</code>	<code>type</code>	selects the external faces (surfaces) of the currently selected elements. The face number is stored in the column after <code>EltId</code> to allow set generation. See more details under <code>SelEdge</code> . The <code>-KeepAll</code> option skips the internal face elimination step.
<code>WithNode</code>	<code>i</code>	finds elements with at least one node in the set <code>i</code> . <code>i</code> can be a list of node numbers. Replacements for <code>i</code> are accepted as above.
<code>SetName</code>	<code>s</code>	finds elements in element set named <code>s</code> (see <code>set</code> stack entries). <code>SetName "name:con IdList"</code> can be used to select a subset connected to nodes in the <code>IdList</code> (assuming the <code>.NodeCon</code> field is defined).
<code>WithoutNode</code>	<code>i</code>	finds elements without any of the nodes in the set <code>i</code> . <code>i</code> can be a list of node numbers. Replacements for <code>i</code> are accepted as above.

Different selectors can be chained using the logical operations `&` (finds elements that verify both conditions), `|` (finds elements that verify one or both conditions). `i1=feutil('FindEltGroup 1:3 & with node 1 8',model)` for example. Condition combinations are always evaluated from left to right (parentheses are not accepted).

Command option `eps1 value` can be used to give an evaluation tolerance for equality logical operators.

Numeric values to the command can be given as additional arguments. Thus the command above could also have been written `i1=feutil('findelt group & withnode',model,1:3,[1 8])`.

7.13 Defining fields trough tables, expressions, ...

Finite element fields are used in four main formats

- `def` field at DOFs
- `InfoAtNode` field at nodes of an element group can be built from a `pro.MAP` field and also from an `info,EltOrient` entry a structure with fields `.bas` and `.EltId`.
- `gstate` field at integration points of an element group (can be built from a `pro.gstate` field).
- a field definition structure to be transformed to the other formats using a `elem0('VectFromDir')` command as illustrated below.

The field definition structure has fields

- data.dir** a cell array specifying the value of various fields. Each cell of **data.dir** can give a constant value, a position dependent value defined by a string **FcnName** that is evaluated using **fv(:,jDir)=eval(FcnName)** or **fv(:,jDir)=feval(FcnName,node)** if the first fails. Note that **node** corresponds to nodes of the model in the global coordinate system and you can use the coordinates **x,y,z** for your evaluation.
- data.lab** cell array giving label for each field of an **InfoAtNode** or **gstate** structure.
- data.DOF** a vector defining the DOF associated with each **.dir** entry. The transformation to a vector defined at **model.DOF** is done using **vect=elem0('VectFromDirAtDof',model,data,model.DOF)**.

For example

```
% Analytical expression for a displacement field
femesh('reset');model=femesh('testubeam');
data=struct('dir',{{'ones(size(x))','y','1*x.^3'}}, ...
    'DOF',[.01;.02;.03]);
model.DOF=feutil('GetDOF',model);
def=elem0('VectFromDirAtDof',model,data,model.DOF)

% Orientation field at nodes
data=struct('dir',{{'x./sqrt(x.^2+y.^2)','y./sqrt(x.^2+y.^2)',0}}, ...
    'lab',{{'v1x','v1y','v1z'}}});
pro=struct('il',1,'type','p_solid','MAP',data);
model=stack_set(model,'pro','WithMap',pro);
C1=fe_mkn1('init',model);InfoAtNode=C1.GroupInfo{7}
```

7.14 Constraint and fixed boundary condition handling

7.14.1 Theory and basic example

`rigid` links, `FixDof`, `MPC` entries, symmetry conditions, continuity constraints in CMS applications, ... all lead to problems of the form

$$\begin{aligned} [Ms^2 + Cs + K] \{q(s)\} &= [b] \{u(s)\} \\ \{y(s)\} &= [c] \{q(s)\} \\ [c_{int}] \{q(s)\} &= 0 \end{aligned} \quad (7.1)$$

The linear constraints $[c_{int}] \{q(s)\} = 0$ can be integrated into the problem using Lagrange multipliers or constraint elimination. Elimination is done by building a basis T for the kernel of the constraint equations, that is such that

$$\text{range}([T]_{N \times (N-NC)}) = \ker([c_{int}]_{NS \times N}) \quad (7.2)$$

Solving problem

$$\begin{aligned} [T^T M T s^2 + T^T C T s + T^T K T] \{q_R(s)\} &= [T^T b] \{u(s)\} \\ \{y(s)\} &= [c T] \{q_R(s)\} \end{aligned}$$

is then strictly equivalent to solving (7.1).

The basis T is generated using `[Case,model.DOF]=fe_case(model,'gett')` where `Case.T` gives the T basis and `Case.DOF` describes the active or master DOFs (associated with the columns of T) while `model.DOF` describes the full list of DOFs.

The assembly of unconstrained M , ... or constrained $T^T M T$ matrices can be controlled with appropriate options in `fe_mkn1`, `fe_load`, ... Typically a `NoT` string is added to the command.

For the two bay truss example, can be written as follows :

```
femesh('reset');
model2 = femesh('test 2bay');
model2=fe_case(model, ...           % defines a new case
  'FixDof','2-D motion',[.03 .04 .05]', ... % 2-D motion
  'FixDof','Clamp edge',[1 2]');           % clamp edge
Case=fe_case('gett',model2) % Notice the size of T and
fe_c(Case.DOF)                % display the list of active DOFs
model2 = fe_mkn1(model2)
```

```

% Now reassemble unconstrained matrices and verify the equality
% of projected matrices
[m,k,mdof]=fe_mknl(model2,'NoT');

norm(full(Case.T'*m*Case.T-model2.K{1}))
norm(full(Case.T'*k*Case.T-model2.K{2}))

```

7.14.2 Local coordinates

In the presence of local coordinate systems (non zero value of `DID` in node column 3), the `Case.cGL` matrix built during the `gett` command, gives a local to global coordinate transformation

$$\{q_{global}\} = [cGL] \{q_{local}\}$$

Master DOFs (DOFs in `Case.DOF`) are defined in the local coordinate system. As a result, M is expected to be defined in the global response system while the projected matrix $T^T M T$ is defined in local coordinates. `mpc` constraints are defined using the local basis.

7.14.3 Enforced displacement

For a `DofSet` entry, one defines the enforced motion in `Case.TIn` and associated DOFs in `Case.DofIn`. The DOFs specified in `Case.DofIn` are then fixed in `Case.T`.

7.14.4 Low level examples

A number of low level commands (`femesh GetDof`, `FindNode`, ...) and functions `fe_c` can be used to operate similar manipulations to what `fe_case GetT` does, but things become rapidly complex. For example

```

femesh('reset'); model = femesh('test 2bay');
[m,k,mdof]=fe_mknl(model)

i1 = femesh('findnode x==0');
adof1 = fe_c(mdof,i1,'dof',1); % clamp edge
adof2 = fe_c(mdof,[.03 .04 .05]','dof',1); % 2-D motion

```



```

adof = fe_c(mdof,[adof1;adof2], 'dof',2);

ind = fe_c(model.DOF,adof,'ind');
mdof=mdof(ind); tmt=m(ind,ind); tkt=k(ind,ind);

```

Handling multiple point constraints (rigid links, ...) really requires to build a basis T for the constraint kernel. For rigid links the obsolete `rigid` function supports some constraint handling. The following illustrates restitution of a constrained solution on all DOFs

```

% Example of a plate with a rigid edge
model=femesh('testquad4 divide 10 10');femesh(model)

% select the rigid edge and set its properties
femesh(';selet group1 & seledge & innode {x==0};addsel');
femesh('setgroup2 name rigid');
FEelt(femesh('findelt group2'),3)=123456;
FEelt(femesh('findelt group2'),4)=0;
model=femesh;

% Assemble
model.DOF=feutil('getdof',model);% full list of DOFs
[tmt,tkt,mdof] = fe_mknl(model); % assemble constrained matrices
Case=fe_case(model,'gett');      % Obtain the transformation matrix

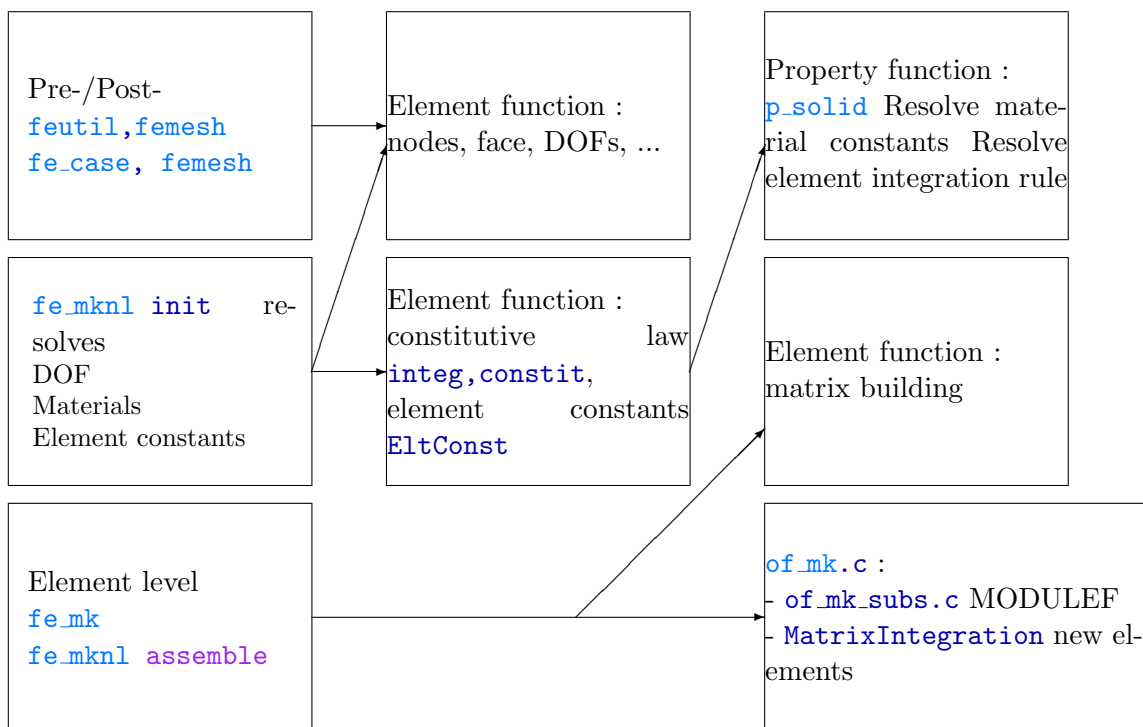
[md1,f1]=fe_eig(tmt,tkt,[5 10 1e3]); % compute modes on master DOF

def=struct('def',Case.T*md1,'DOF',model.DOF) % display on all DOFs
feplot(model,def); fecom(';view3;ch7')

```

7.15 Internal data structure reference

7.15.1 Element functions and C functionality



In *OpenFEM*, elements are defined by element functions. Element functions provide different pieces of information like geometry, degrees of freedom, model matrices, ...

OpenFEM functions like the preprocessor `femesh`, the model assembler `fe_mk` or the post-processor `feplot` call element functions for data about elements.

For example, in the assembly step, `fe_mk` analyzes all the groups of elements. For each group, `fe_mk` gets its element type (`bar1`, `hexa8`, ...) and then calls the associated element function.

First of all, `fe_mk` calls the element function to know what is the right call form to compute the elementary matrices (`eCall=elem0('matcall')` or `eCall=elem0('call')`, see section 7.16.6 for details). `eCall` is a string. Generally, `eCall` is a call to the element function. Then for each element, `fe_mk` executes `eCall` in order to compute the elementary matrices.

This automated work asks for a likeness of the element functions, in particular for the calls and the outputs of these functions. Next section gives information about element function writing.

7.15.2 Standard names in assembly routines

<code>cEGI</code>	vector of element property row indices of the current element group (without the group header)
<code>constit</code>	real (<code>double</code>) valued constitutive information. The <code>constit</code> for each group is stored in <code>Case.GroupInfo{jGroup,4};</code> .
<code>def.def</code>	vector of deformation at DOFs. This is used for non-linear, stress or energy computation calls that need displacement information.
<code>EGID</code>	Element Group Identifier of the current element group (different from <code>jGroup</code> if an EGID is declared).
<code>elt</code>	model description matrix. The element property row of the current element is given by <code>elt(cEGI(jElt),:)</code> which should appear in the calling format <code>eCall</code> of your element function.
<code>ElemF</code>	name of element function or name of superelement
<code>ElemP</code>	parent name (used by <code>femesh</code> in particular to allow property inheritance)
<code>gstate</code>	real (<code>double</code>) valued element state information.
<code>integ</code>	<code>int32</code> valued constitutive information.
<code>jElt</code>	number of the current element in <code>cEGI</code>
<code>jGroup</code>	number of the current element group (order in the element matrix). <code>[EGroup,nGroup]=getegroup(elt);</code> finds the number of groups and group start indices.
<code>nodeE</code>	nodes of the current element. In the compiled functions, <code>NodeId</code> is stored in column 4, followed by the values at each node given in the <code>InfoAtNode</code> . The position of known columns is identified by the <code>InfoAtNode.lab</code> labels (the associated integer code is found with <code>comstr('lab',-32)</code>). Of particular interest are <ul style="list-style-type: none">• <code>v1x</code> (first vector of material orientation, which is assumed to be followed by <code>v1y,v1z</code> and for 3D orientation <code>v2x,y,z</code>), see stack entry <code>info,EltOrient</code>• <code>v3x,v3y,v3z</code> for normal maps• <code>T</code> is used for temperature (stack entry <code>info,RefTemp</code>)

NNode node identification reindexing vector. `NNode(ID)` gives the row index (in the `node` matrix) of the nodes with identification numbers `ID`. You may use this to extract nodes in the `node` matrix using something like `node(NNode(elt(cEGI(jElt), [1 2])), :)` which will extract the two nodes with numbers given in columns 1 and 2 of the current element row (an error occurs if one of those nodes is not in `node`). This can be built using `NNode=sparse(node(:,1), 1, 1:size(node,1))`.

pointers one column per element in the current group gives.

7.15.3 Case.GroupInfo cell array

The meaning of the columns of `GroupInfo` is as follows

`DofPos` `Pointers` `Integ` `Constit` `gstate` `ElMap` `InfoAtNode` `EltConst`

DofPos `int32` matrix whose columns give the DOF positions in the full matrix of the associated elements. Numbering is C style (starting at 0) and -1 is used to indicate a fixed DOF.

pointers `int32` matrix whose columns describe information each element of the group. `Pointers` has one column per element giving
`[OutSize1 OutSize2 u3 NdNRule MatDes IntegOffset
ConstitOffset StateOffset u9 u10]`
`Outsize1` size of element matrix (for elements issued from MODULEF), zero otherwise.
`MatDes` type of desired output. See the `fe_mk MatType` section for a current list.
`IntegOffset` gives the starting index (first element is 0) of integer options for the current element in `integ`.
`ConstitOffset` gives the starting index (first element is 0) of real options for the current element in `constit`.

integ **int32** matrix storing integer values used to describe the element formulation of the group. Meaning depends on the problem formulation and should be documented in the property function (`p_solid BuildConstit` for example).
The nominal content of an **integ** column (as return by the element `integinfo` call) is
`MatId, ProId, NDofPerElt, NNodePerElt, IntegRuleType`
where `integrules(ElemP, IntegRuleType)` is supposed to return the appropriate integration rule.

constit **double** matrix storing integer values used to describe the element formulation of the group. Meaning depends on element family and should be documented in the element property function (`p_solid BuildConstit` for example).

gstate a curve with field `.Y` describing the internal state of each element in the group. Typical dimensions stress, integration points, elements so that `.Y` has size $N_{strain} \times N_w \times N_{Elt}$. The labels in `.X{1}` can be used to find positions in the `.Y` matrix. The `.X{2}` should contain the gauss point locations within the reference element. Automated generation of initial states is discussed in section 7.13.
Users are of course free to add any appropriate value for their own elements, a typical application is the storage of internal variables. For an example of **gstate** initialization see `fe_stress thermal`.
the old format with a **double** matrix with one column per element is still supported but will be phased out.

ElMap **int32** element map matrix used to distinguish between internal and external element DOF numbering (for example : `hexa8` uses all x DOF, then all y ... as internal numbering while the external numbering is done using all DOFs at node 1, then node 2, ...). The element matrix in external sort is given by `k_ext=ke(ElMap)`. `EltConst.VectMap` gives similar reordering information for vectors (loads, ...).

InfoAtNode a structure with `.NodePos (int32)` with as many columns as elements in the group giving column positions in a `.data` field. The each row in `.data` corresponds to a field that should be described by a cell array of string in `.lab` used to identify fields in assembly, see `nodeE`. Automated generation is discussed in section 7.13. Initialization for a given element type is done the `GroupInit` phase.
Obsolete format : **double** matrix whose **rows** describe information at element nodes (as many columns as nodes in the model).

EltConst **struct** used to store element formulation information (integration rule, constitutive matrix topology, etc.) Details on this data structure are given in section 7.15.4.

7.15.4 Element constants data structure

The `EltConst` data structure is used in most newer generation elements implemented in `of_mk.c`. It contains geometric and integration rule properties. The shape information is generated by calls to `integrules`. The formulation information is generated `p_function const` calls (see `p_solid`, `p_heat`, ...).

<code>.N</code>	$nw \times Nnode$ shape functions at integration points
<code>.Nr</code>	$nw \times Nnode$ derivative of shape function with respect to the first reference coordinate r
<code>.Ns</code>	$nw \times Nnode$ derivative of shape function with respect to the second reference coordinate s
<code>.Nt</code>	$nw \times Nnode$ derivative of shape function with respect to the second reference coordinate t
<code>.NDN</code>	$Nshape \times nw(1 + Ndim)$ memory allocation to store the shape functions and their derivatives with respect to physical coordinates $[N N, x N, y N, z]$. <code>of_mk</code> currently supports the following geometry rules 3 3D volume, 2 2D volume, 23 3D surface, 13 3D line (see <code>integrules BuildNDN</code> for calling formats). Cylindrical and spherical coordinates are not currently supported. In the case of rule 31 (hyperelastic elements), the storage scheme is modified to be $(1 + Ndim) \times Nshape \times nw$ which preserves data locality better.
<code>.jdet</code>	Nw memory allocation to store the determinant of the jacobian matrix at integration points.
<code>.bas</code>	$9 \times Nw$ memory allocation to store local material basis. This is in particular used for 3D surface rules where components 6:9 of each column give the normal.
<code>.Nw</code>	number of integration points (equal to <code>size(EltConst.N,1)</code>)
<code>.Nnode</code>	number of nodes (equal to <code>size(EltConst.N,2)=size(EltConst.NDN,1)</code>)
<code>.xi</code>	$Nnode \times 3$ reference vertex coordinates
<code>.VectMap</code>	index vector giving DOF positions in external sort. This is needed for RHS computations.
<code>.CTable</code>	low level interpolation of constitutive relation based on field values. Storage as a double vector is given by <code>[Ntables CurrentValues (Ntables x 7) tables]</code> with <code>CurrentValues</code> giving <code>[i1 xi si xstartpos Nx nodeEfield constit(pos_Matlab)]</code> . Implementation is provided for <code>m_elastic</code> to account for temperature dependence, <code>fe_mat</code> to generate interpolated properties.

7.16 Creating new elements (advanced tutorial)

In this section one describes the developments needed to integrate a new element function into *OpenFEM*. First, general information about OpenFEM work is given. Then the writing of a new element function is described. And at last, conventions which must be respected are given.

7.16.1 Generic compiled linear and non-linear elements

To improve the ease of development of new elements, OpenFEM now supports a new category of generic element functions. Matrix assembly, stress and load assembly calls for these elements are fully standardized to allow optimization and generation of new element without recompilation. All the element specific information stored in the `EltConst` data structure.

Second generation volume elements are based on this principle and can be used as examples. These elements also serve as the current basis for non-linear operations.

The adopted logic is to develop families of elements with different topologies. To implement a family, one needs

- shape functions and integration rules. These are independent of the problem posed and grouped systematically in `integrules`.
- topology, formatting, display, test, ... information for each element. This is the content of the element function (see `hexa8`, `tetra4`, ...).
- a procedure to build the `constit` vectors from material data. This is nominally common to all elements of a given family and is used in `integinfo` element call. For example `p_solid('BuildConstit')`.
- a procedure to determine constants based on current element information. This is nominally common to all elements of a given family and is used in `groupinit` phase (see `fe_mk`). The `GroupInit` call is expected to generate an `EltConst` data structure, that will be stored in the last column of `Case.GroupInfo`. For example `hexa8 constants` which calls `p_solid('ConstSolid')`.
- a procedure to build the element matrices, right hand sides, etc. based on existing information. This is compiled in `of_mk MatrixIntegration` and `StressObserve` commands. For testing/development purposes is expected

that for `sdtdef('diag',12)` an `.m` file implementation in `elem0.m` is called instead of the compiled version.

The following sections detail the principle for linear and non-linear elements.

7.16.2 What is done in the element function

Most of the work in defining a generic element is done in the element property function (for initializations) and the compile `of_mk` function. You do still need to define the commands

- `integinfo` to specify what material property function will be called to build `integ`, `constit` and `elmap`. For example, in `hexa8`, the code for this command is

```
if comstr(Cam,'integinfo')
    %constit integ,elmap                                ID,pl,il
    [out,out1,out2]= ...
    p_solid('buildconstit',[varargin{1};24;8],varargin{2},varargin{3});
```

input arguments passed from `fe_mkn1` are `ID` a unique pair of `MatId` and `ProId` in the current element group. `pl` and `il` the material and element property fields in the model. Expected outputs are `constit`, `integ` and `elmap`, see `Case.GroupInfo`. Volume elements `hexa8`, `q4p`, ... are topology holders. They call `p_solid BuildConstit` which in turn calls as another property function as coded in the type (column two of `il` coded with `fe_mat('p_fun','SI',1)`). When another property function is called, it is expected that `constit(1:2)=[-1 TypeM]` to allow propagation of type information to parts of the code that will not analyze `pl`.

- `constants` to specify what element property function will be called to initialize `EltConst` data structure and possibly set the geometry type information in `pointers(4,:)`. For example, in `hexa8`, the code for this command is

```
...
elseif comstr(Cam,'constants')
    integ=varargin{2};constit=varargin{3};
    if nargin>3; [out,ldim]=p_solid('const','hexa8',integ,constit);
    else; p_solid('constsolid','hexa8',[1 1 24 8],[]);return;
    end
    out1=varargin{1};out1(4,:)=ldim; % Tell of_mk('MatrixInt') this is IDIM
    ...
```


input arguments passed from `fe_mkn1` are `pointers,integ,constit` the output arguments are `EltConst` and a modified `pointers` where row 4 is modified to specify a 3D underlying geometry.

If `constit(1:2)=[-1 TypeM]` `p_solid` calls the appropriate property function. For elements that have an internal orientation (shells, beams, etc.) it is expected that orientation maps are built during this command (see `beam1t`, ...). Note, that the `'info','EltOrient'` stack entry can also be used for that purpose.

- standard topology information (commands `node`, `dof`, `prop`, `line`, `patch`, `face`, `edge`, `parent`) see section 7.16.6.

`hexa8` provides a clean example of what needs to be done here.

7.16.3 What is done in the property function

`p_fcn`

Commands specific to `p_*` are associated to the implementation of a particular physical formulation for all topologies.

`BuildConstit`

As shown in section 7.15.1 and detailed under `fe_mkn1` the FEM initialization phase needs to resolve

- constitutive law information from model constants (`elem0 integinfo` call to the element functions, which for all topology holder elements is forwarded to `p_solid BuildConstit`)
- and to fill-in integration constants and other initial state information (using `groupinit` to generate the call and `constant` build the data).

Many aspects of a finite element formulation are independent of the supporting topology. Element property functions are thus expected to deal with topology independent aspects of element constant building for a given family of elements.

Thus the element `integinfo` call usually just transmits arguments to a property function that does most of the work. That means defining the contents of `integ`

and `constit` columns. For example for an acoustic fluid, `constit` columns generated by `p_solid BuildConstit` contain $\begin{bmatrix} \frac{1}{\rho C^2} & \eta & \frac{1}{\rho} \end{bmatrix}$.

Generic elements (`hexa8`, `q4p`, ...) all call `p_solid BuildConstit`. Depending on the property type coded in column 2 of the current material, `p_solid` attempts to call the associated `m_mat` function with a `BuildConstit` command. If that fails, an attempt to call `p_mat` is made (this allows to define a new family of elements through a single `p_fcn` `p_heat` is such an example).

`integ` nominally contains `MatId, ProId, NDofPerElt, NNodePerElt, IntegRuleNumber`.

Const

Similarly, element constant generation of elements that support variable integration rules is performed for an element family. For example, `p_solid const` supports for 3D elastic solids, for 2D elastic solids and 3D acoustic fluid volumes. `p_heat` supports 2D and 3D element constant building for the heat equation.

Generic elements (`hexa8`, `q4p`, ...) all use the call `[EltConst, NDNDim] = p_solid('Const', ElemF, integ, constit)`.

User extensibility requires that the user be able to bypass the normal operation of `p_solid const`. This can be achieved by setting `constit(1)=-1` and coding a property type in the second value (for example `constit(1)=fe_mat('p_heat', 'SI', 1)`). The proper function is then called with the same arguments as `p_solid`.

`*_fcn` Expected commands common to both `p_*` and `m_*` functions are the following

Subtype

With no argument returns a cell array of strings associated with each subtype (maximum is 9). With a string input, it returns the numeric value of the subtype. With a numeric input, returns the string value of the subtype. See `m_elastic` for the reference implementation.

database

Returns a structure with reference materials or properties of this type. Additional strings can be used to give the user more freedom to build properties.

dbval

Mostly the same as `database` but replaces or appends rows in `model.il` (for element properties) or `model.pl` (for material properties).

PropertyUnitType

`il=p_function('PropertyUnitType',SubType)` returns for each subtype the units of each value in the property row (column of pl).

This mechanism is used to automate unit conversions in `fe_mat Convert`.

`[list,repeat]=p_function('PropertyUnitTypeCell',SubType)` returns a cell array describing the content of each column, the units and possibly a longer description of the variable. When properties can be repeated a variable number of times, use the `repeat` (example in `p_shell` for composites). This mechanism is used to generate graphical editors for properties.

Cell arrays describing each subtype give

- a label. This should be always the same to allow name based manipulations and should not contain any character that cannot be used in field names.
- a conversion value. Lists of units are given using `fe_mat('convertSITM')`. If the unit is within that list, the conversion value is the row number. If the unit is the ratio of two units in the list this is obtained using a non integer conversion value. Thus `9.004` corresponds to kg/m (9 is kg and 4 is m).
- a string describing the unit

7.16.4 Compiled element families in of_mk

`of_mk` is the C function used to handle all compiled element level computations. Integration rules and shape derivatives are also supported as detailed in `BuildNDN`.

Generic multi-physic linear elements

This element family supports a fairly general definition of linear multi-physic elements whose element integration strategy is fully described by an `EltConst` data structure. `hexa8` and `p_solid` serve as a prototype element function. Element matrix

and load computations are implemented in the `of.mk.c MatrixIntegration` command with `StrategyType=1`, stress computations in the `of.mk.c StressObserve` command.

```
EltConst=hexa8('constants', [], [1 1 24 8], []);
integrules('texstrain',EltConst)
EltConst=integrules('stressrule',EltConst);
integrules('texstress',EltConst)
```

Elements of this family are standard element functions (see section 7.16) and the element functions must thus return `node`, `prop`, `dof`, `line`, `patch`, `edge`, `face`, and `parent` values. The specificity is that all information needed to integrate the element is stored in an `EltConst` data structure that is initialized during the `fe_mkn1 GroupInit` phase.

For DOF definitions, the family uses an internal DOF sort where each field is given at all nodes sequentially `1x2x...8x1y...8y...` while the more classical sort by node `1x1y...2x...` is still used for external access (internal and external DOF sorting are discussed in section 7.16.6).

Each linear element matrix type is represented in the form of a sum over a set of integration points

$$k^{(e)} = \sum_{ji,jj} \sum_{jw} \left[\{B_{ji}\} D_{ji \ jk}(w(jw)) \{B_{jj}\}^T \right] J(w(jw)) W((jw)) \quad (7.3)$$

where the jacobian of the transformation from physical `xyz` to element `rst` coordinates is stored in `EltConst.jdet(jw)` and the weighting associated with the integration rule is stored in `EltConst.w(jw,4)`.

The relation between the `Case.GroupInfo constit` columns and the D_{ij} constitutive law matrix is defined by the cell array `EltConst.ConstitTopology` entries. For example, the strain energy of a acoustic pressure formulation (`p_solid ConstFluid`) is given by

```
constit(:,j1)=[1/rho/C2; eta ; 1/rho]
```

$$\text{EltConst.MatrixTopology}\{1\} = \begin{bmatrix} 3 & 0 & 0 \\ 0 & 3 & 0 \\ 0 & 0 & 3 \end{bmatrix} \quad D = \begin{bmatrix} 1/\rho & 0 & 0 \\ 0 & 1/\rho & 0 \\ 0 & 0 & 1/\rho \end{bmatrix}$$

The integration rule for a given element is thus characterized by the strain observation matrix $B_{ji}(r, s, t)$ which relates a given strain component ϵ_{ji} and the nodal displacements. The generic linear element family assumes that the generalized strain components are linear functions of the shape functions and their derivatives in euclidian coordinates (xyz rather than rst).

The first step of the element matrix evaluation is the evaluation of the `EltConst.NDN` matrix whose first Nw columns store shape functions, Nw next their derivatives with respect to x , then y and z for 3D elements

$$[NDN]_{Nnode \times Nw(Ndims+1)} = \left[[N(r, s, t)] \begin{bmatrix} \frac{\partial N}{\partial x} \\ \frac{\partial N}{\partial y} \\ \frac{\partial N}{\partial z} \end{bmatrix} \right] \quad (7.4)$$

To improve speed the `EltConst.NDN` and associated `EltConst.jdet` fields are pre-allocated and reused for the assembly of element groups.

For each strain vector type, one defines an `int32` matrix

`EltConst.StrainDefinition{jType}` with each row describing `row`, `NDNBloc`, `DOF`, `NwStart`, `NwTot` giving the strain component number (these can be repeated since a given strain component can combine more than one field), the block column in NDN (block 1 is N , 4 is $\partial N/\partial z$), the field number, and the starting integration point associated with this strain component and the number of integration points needed to assemble the matrix. The default for `NwStart` `NwTot` is `1`, `Nw` but this formalism allows for differentiation of the integration strategies for various fields. The figure below illustrates this construction for classical mechanical strains.

$$\begin{array}{c}
 \left. \begin{array}{l} \epsilon_x \\ \epsilon_y \\ \epsilon_z \\ \gamma_{yz} \\ \gamma_{zx} \\ \gamma_{xy} \end{array} \right\} = \begin{bmatrix} N,x & 0 & 0 \\ 0 & N,y & 0 \\ 0 & 0 & N,z \\ 0 & N,z & N,y \\ N,z & 0 & N,x \\ N,y & N,x & 0 \end{bmatrix} \left. \begin{array}{l} u \\ v \\ w \end{array} \right\}
 \end{array}
 \quad
 \begin{array}{l}
 \text{EltConst.StrainDefinition}\{1\} = \begin{bmatrix} 1 & 2 & 1 & 1 & 8 \\ 2 & 3 & 2 & 1 & 8 \\ 3 & 4 & 3 & 1 & 8 \\ 4 & 4 & 2 & 1 & 8 \\ 4 & 3 & 3 & 1 & 8 \\ 5 & 4 & 1 & 1 & 8 \\ 5 & 2 & 3 & 1 & 8 \\ 6 & 3 & 1 & 1 & 8 \\ 6 & 2 & 2 & 1 & 8 \end{bmatrix} \\
 \\
 [NDN]_{Nnode \times Nw(Ndims+1)} = \left[[N(r, s, t)] \begin{bmatrix} \frac{\partial N}{\partial x} \\ \frac{\partial N}{\partial y} \\ \frac{\partial N}{\partial z} \end{bmatrix} \right] \sum_{j_w=1}^8
 \end{array}$$

To help you check the validity of a given rule, you should fill the

`EltConst.StrainLabels{jType}` and `EltConst.DofLabels` fields and use the `integrules('texstrain', EltConst)` command to generate a LATEX printout of the rule you just generated.

The `.StrainDefinition` and `.ConstitTopology` information is combined automatically in `integrules` to generate `.MatrixIntegration` (`integrules MatrixRule` command) and `.StressRule` fields (`integrules StressRule` command). These tables once filed properly allow an automated integration of the element level matrix and stress computations in OpenFEM.

Phases in of_mk.c matrix integration

The core of element computations is the `matrixintegration` command that computes and assembles a group of elements.

After a number of inits, one enters the loop over elements.

The `nodeE` matrix, containing *field at element nodes*, is filled with information at the element nodes as columns. The first 3 columns are positions. Column 4 is reserved for node numbers in case a callback to MATLAB makes use of the information. The following columns are based on the `InfoAtNode` structure whos indexing strategy is compatible with both continuous and discontinuous fields at each node. See `sdtweb elem0('get_nodeE')` for details.

Initialization of `InfoAtNode` is performed with `fe_mkn1('Init -gstate')` calls. The `m_elastic AtNodeGState` command is an illustration of init used to interpolate material properties in volume elements.

The `defe` vector/matrix contains the values at the current element DOF of the provided deformation(s).

Generic RHS computations

Right hand side (load) computations can either be performed once (fixed set of loads) through `fe_load` which deals with multiple loads, or during an iterative process where a single RHS is assembled by `fe_mkn1` into the second column of the state argument `dc.def(:,2)` along with the matrices when requiring the stiffness with `MatDes=1` or `MatDes=5` (in the second case, the forces are assumed following if implemented).

There are many classical forms of RHS, one thus lists here forms that are implemented in `of_mk.c MatrixIntegration`. Computations of these rules, requires that the `EltConst.VectMap` field be defined. Each row of `EltConst.RhsDefinition` specifies the procedure to be used for integration.

Two main strategies are supported where the fields needed for the integration of

loads are stored either as columns of `dc.def` (for fields that can be defined on DOFs of the model) or as `nodeE` columns.

Currently the only accepted format for rows of `EltConst.RhsDefinition` is

```
101(1) InfoAtNode1(2) InStep(3) NDNOff1(4) FDof1(5) NDNCol(6)
NormalComp(7) w1(8) nwStep(9)
```

Where `InfoAtNode1` gives the first row index in storing the field to be integrated in `InfoAtNode`. `InStep` gives the index step (3 for a 3 dimensional vector field), `NDNOff1` gives the block offset in the NDN matrix (zero for the nominal shape function). `FDof1` gives the offset in force DOFs for the current integration. `NDNCol`. If larger than `-1`, the normal component `NormalComp` designs a row number in `EltConst.bas`, which is used as a weighting coefficient. `tt w1` gives the index of the first gauss point to be used (in C order starting at 0). `nwStep` gives the number of gauss points in the rule being used.

- volume forces not proportional to density

$$\int_{\Omega_0} f_v(x).du(x) = \{F_v\}_k = \sum_{j_w} (\{N_k(j_w)\} \{N_j(j_w)\} f_v(x_j)) J(j_w)W(j_w) \quad (7.5)$$

are thus described by

```
opt.RhsDefinition=int32( ...
  101 0 3 0      0 0 -1   rule+[-1 0];
  101 1 3 0      1 0 -1   rule+[-1 0];
  101 2 3 0      2 0 -1   rule+[-1 0]);
```

for 3D solids (see `p_solid`).

Similarly, normal pressure is integrated as 3 volume forces over 3D surface elements with normal component weighting

$$\begin{aligned} F_m &= \int_{\partial\Omega_0} p(x)n_m(x).dv(x) \\ &= \sum_{j_w} (\{N_k(j_w)\} \{N_j(j_w)\} p(x_j)n_m) J(j_w)W(j_w) \end{aligned} \quad (7.6)$$

- inertia forces (volume forces proportional to density)

$$F = \int_{\Omega_0} \rho(x)f_v(x).dv(x) \quad (7.7)$$

- stress forces (will be documented later)

Large transformation linear elasticity

`Elastic3DNL` fully anisotropic elastic elements in geometrically non-linear mechanics problems. Element matrix are implemented in the `of_mk.c MatrixIntegration` command with `StrategyType=2` for the linear tangent matrix (`MatType=5`). Other computations are performed using generic elements (section 7.16.4) (mass `MatType=2`). This formulation family has been tested for the prediction of vibration responses under static pre-load.

Stress post-processing is implemented using the underlying linear element.

Hyperelasticity

Simultaneous element matrix and right hand side computations are implemented in the `of_mk.c MatrixIntegration` command with `StrategyType=3` for the linear tangent matrix (`MatType=5`). In this case (and only this case!!), the `EltConst.NDN` matrix is built as follow:

for $1 \leq jw \leq Nw$

$$[NDN]_{(Ndims+1) \times Nnode(Nw)} = [[NDN]^{jw}] \quad (7.8)$$

with

$$[NDN]_{(Ndims+1) \times Nnode}^{jw} = \begin{bmatrix} [N(r, s, t)]_{jw} \\ \left[\frac{\partial N}{\partial x} \right]_{jw} \\ \left[\frac{\partial N}{\partial y} \right]_{jw} \\ \left[\frac{\partial N}{\partial z} \right]_{jw} \end{bmatrix} \quad (7.9)$$

This implementation corresponds to `case 31` of `NDNSwitch` function in `of_mk_pre.c`. The purpose is to use C-BLAS functions in element matrix and right hand side computations implemented in the same file (function `Mecha3DintegH`) to improve speed.

Other computations are performed using generic elements (section 7.16.4) (mass `MatType=2`). This formulation family has been tested for the `RivlinCube` test.

Stress post-processing is not yet implemented for hyperelastic media.

7.16.5 Non-linear iterations, what is done in `of_mk`

Non linear problems are characterized by the need to perform iterations with multiple assemblies of matrices and right hand sides (RHS). To optimize the performance, the nominal strategy for non-linear operations is to

- perform an initialization (standard `of_mkn1 init` call)
- define a deformation data structure `dc` with two columns giving respectively the current state and the non linear RHS.

At a given iteration, one resets the RHS and performs a single `fe_mkn1` call that returns the current non-linear matrix and replaces the RHS by its current value (note that `fe_mkn1` actually modifies the input argument `dc` which is not a normal MATLAB behavior but is needed here for performance)

```
% at init allocate DC structure
dc=struct('DOF',model.DOF,'def',zeros(length(model.DOF),2);
% ... some NL iteration mechanism here
dc.def(:,2)=0; % reset RHS at each iteration
k=fe_mkn1('assemble not',model,Case,dc,5); % assemble K and RHS
```

Most of the work for generic elements is done within the `of_mk MatrixIntegration` command that is called by `fe_mkn1`. Each call to the command performs matrix and RHS assembly for a full group of elements. Three strategies are currently implemented

- **Linear** multiphysic elements of arbitrary forms, see section 7.16.4
- **Elastic3DNL** general elastic elements for large, see section 7.16.4 transformation,
- **Hyperelastic** elements for large transformation problems. see section 7.16.4. These elements have been tested through the **RivlinCube** example.

7.16.6 Element function command reference

Nominally you should write topology independent element families, if hard coding is needed you can however develop new element functions.

In Matlab version, a typical element function is an `.m` or `.mex` file that is in your MATLAB path. In Scilab version, a typical element function is an `.sci` or `.mex` file that is loaded into Scilab memory (see `getf` in Scilab on-line help).

The name of the function/file corresponds to the name of the element (thus the element `bar1` is implemented through the `bar1.m` file)

General element information

To build a new element take `q4p.m` or `q4p.sci` as an example.

As for all Matlab or Scilab functions, the header is composed of a function syntax declaration and a help section. The following example is written for Matlab. For Scilab version, don't forget to replace `%` by `//`. In this example, the name of the created element is `elem0`.

For element functions the nominal format is

```
function [out,out1,out2]=elem0(CAM,varargin);
%elem0 help section
```

The element function should then contain a section for standard calls which let other functions know how the element behaves.

```
if isstr(CAM) %standard calls with a string command

[CAM,Cam]=comstr(CAM,1); % remove blanks
if comstr(Cam,'integinfo')
% some code needed here
out= constit; % real parameter describing the constitutive law
out1=integ; % integer (int32) parameters for the element
out2=elmap;

elseif comstr(Cam,'matcall')
out=elem0('call');
out1=1; % SymFlag
elseif comstr(Cam,'call'); out = ['AssemblyCall'];
elseif comstr(Cam,'rhscall'); out = ['RightHandSideCall'];
elseif comstr(Cam,'scall'); out = ['StressComputationCall'];
elseif comstr(Cam,'node'); out = [NodeIndices];
elseif comstr(Cam,'prop'); out = [PropertyIndices];
elseif comstr(Cam,'dof'); out = [ GenericDOF ];
```

```

elseif comstr(Cam,'patch');
                    out = [ GenericPatchMatrixForPlotting ];
elseif comstr(Cam,'edge');    out = [ GenericEdgeMatrix ];
elseif comstr(Cam,'face');    out = [ GenericFaceMatrix ];
elseif comstr(Cam,'sci_face'); out = [ SciFaceMatrix ];
elseif comstr(Cam,'parent');  out = ['ParentName'];
elseif comstr(Cam,'test')
    % typically one will place here a series of basic tests
end
return
end % of standard calls with string command

```

The expected outputs to these calls are detailed below.

`call,matcall`

Format string for element matrix computation call. Element functions must be able to give `fe_mk` the proper format to call them (note that superelements take precedence over element functions with the same name, so avoid calling a superelement `beam1`, etc.).

`matcall` is similar to `call` but used by `fe_mkn1`. Some elements directly call the `of_mk` mex function thus avoiding significant loss of time in the element function. If your element is not directly supported by `fe_mkn1` use `matcall=elem0('call')`.

The format of the call is left to the user and determined by `fe_mk` by executing the command `eCall=elem0('call')`. The default for the string `eCall` should be (see any of the existing element functions for an example)

```

[k1,m1]=elem0(nodeE,elt(cEGI(jElt),:),...
              pointers(:,jElt),integ,constit,elmap);

```

To define other proper calling formats, you need to use the names of a number of variables that are internal to `fe_mk`. `fe_mk` variables used as *output arguments of element functions* are

<code>k1</code>	element matrix (must always be returned, for <code>opt(1)==0</code> it should be the stiffness, otherwise it is expected to be the type of matrix given by <code>opt(1)</code>)
<code>m1</code>	element mass matrix (optional, returned for <code>opt(1)==0</code> , see below)

```
[ElemF,opt,ElemP]=
zrfeutil('getelemf',elt(EGroup(jGroup),:),jGroup)
```

returns, for a given header row, the element function name `ElemF`, options `opt`, and parent name `ElemP`.

`fe_mk` and `fe_mkn1` variables that can be used as *input arguments to element function* are listed in section 7.15.2.

`dof`, `dofcall`

Generic DOF definition vector. For user defined elements, the vector returned by `elem0('dof')` follows the usual DOF definition vector format (`NodeId.DofId` or `-1.DofId`) but is generic in the sense that node numbers indicate positions in the element row (rather than actual node numbers) and `-1` replaces the element identifier (if applicable).

For example the `bar1` element uses the 3 translations at 2 nodes whose number are given in position 1 and 2 of the element row. The generic DOF definition vector is thus `[1.01;1.02;1.03;2.01;2.01;2.03]`.

A `dofcall` command may be defined to bypass generic `dof` calls. In particular, this is used to implement elements where the number of DOFs depends on the element properties. The command should always return `out=elem0('dofcall');`. The actual DOF building call is performed in `p_solid('BuildDof')` which will call user `p_*.m` functions if needed.

Elements may use different DOF sorting for their internal computations.

`edge`, `face`, `patch`, `line`, `sci_face`

`face` is a matrix where each row describes the positions in the element row of nodes of the oriented face of a volume (conventions for the orientation are described under `integrules`). If some faces have fewer nodes, the last node should be repeated as needed. `feutil` can consider face sets with orientation conventions from other software.

`edge` is a matrix where each row describes the node positions of the oriented edge of a volume or a surface. If some edges have fewer nodes, the last node should be repeated as needed.

`line` (obsolete) is a vector describes the way the element will be displayed in the line mode (wire frame). The vector is generic in the sense that node numbers represent positions in the element row rather than actual node numbers. Zeros can be used to create a discontinuous line. `line` is now typically generated using information provided by `patch`.

`patch`. In MATLAB version, surface representations of elements are based on the use of MATLAB `patch` objects. Each row of the generic patch matrix gives the indices nodes. These are generic in the sense that node numbers represent positions in the element row rather than actual node numbers.

For example the `tetra4` solid element has four nodes in positions `1:4`. Its generic patch matrix is `[1 2 3;2 3 4;3 4 1;4 1 2]`. Note that you should not skip nodes but simply repeat some of them if various faces have different node counts.

`sci_face` is the equivalent of `patch` for use in the SCILAB implementation of *Open-FEM*. The difference between `patch` and `sci_face` is that, in SCILAB, a face must be described with 3 or 4 nodes. That means that, for a two nodes element, the last node must be repeated (in generality, `sci_face = [1 2 2];`). For a more than 4 nodes per face element, faces must be cut in subfaces. The most important thing is to not create new nodes by the cutting of a face and to use all nodes. For example, 9 nodes quadrilateral can be cut as follows :

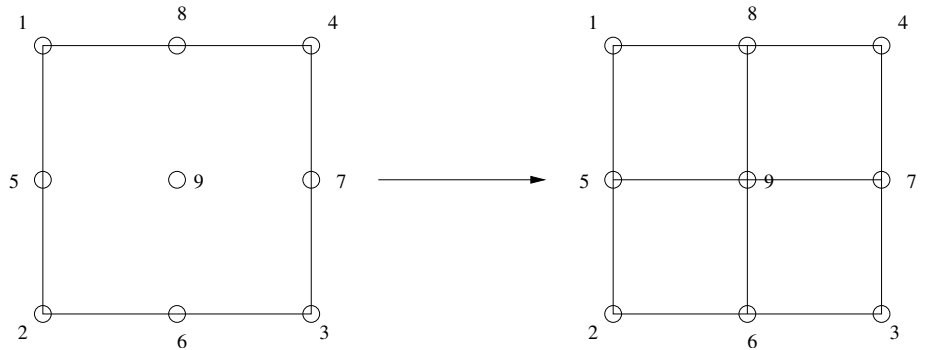


Figure 7.1: Lower order patch representation of a 9 node quadrilateral

but a 8 nodes quadrilaterals cannot be cut by this way. It can be cut as follows:

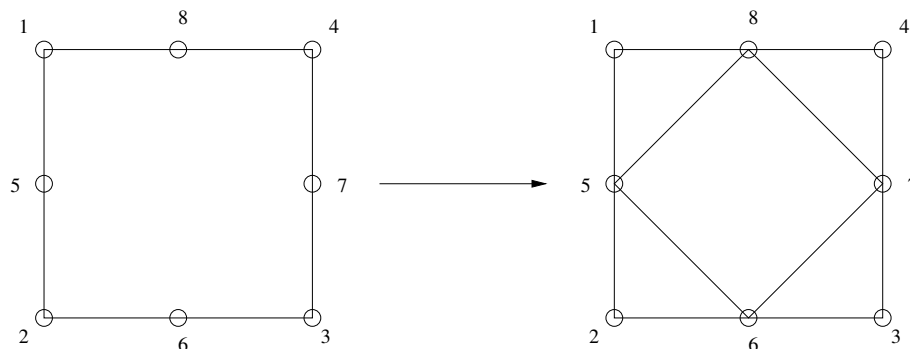


Figure 7.2: Lower order patch representation of a 8 node quadrilateral

`integinfo`, `BuildConstit`

`integinfo`, `BuildConstit` are commands to resolve constants in elements and `p_function` respectively.

`[constit,integ,emap]=elem0('integinfo',[MatId ProId],pl,il,model,Case)` is supposed to search `pl` and `il` for rows corresponding to `MatId` and `ProId` and return a real vector `constit` describing the element constitutive law and an integer vector `integ`.

`ElMap` is used to build the full matrix of an element which initially only gives it lower or upper triangular part. If a structure is return, `fe_mkn1` can do some group wise processing (typically initialization of internal states).

In most elements, one uses `[constit,integ,emap]=p_solid('buildconstit',[varargin{1};Ndof;Nnode],varargin{2:end})` since `p_solid` passes calls to other element property functions when needed.

`emap` can also be used to pass structures and callbacks back to `fe_mkn1`.

`node`

Vector of indices giving the position of nodes numbers in the element row. In general this vector should be `[1:n]` where `n` is the number of nodes used by the element.

prop

Vector of indices giving the position of `MatId`, `ProId` and `EltId` in the element row. In general this vector should be `n+[1 2 3]` where `n` is the number of nodes used by the element. If the element does not use any of these identifiers the index value should be zero (but this is poor practice).

parent

Parent element name. If your element is similar to a standard element (`beam1`, `tria3`, `quad4`, `hexa8`, etc.), declaring a parent allows the inheritance of properties. In particular you will be able to use functions, such as `fe_load` or parts of `femesh`, which only recognize standard elements.

rhscall

`rhscall` is a string that will be evaluated by `fe_load` when computing right hand side loads (volume and surface loads). Like `call` or `matcall`, the format of the call is determined by `fe_load` by executing the command `eCall=elem0('call')`. The default for the string `eCall` should be :

```
be=elem0(nodeE,elt(cEGI(jElt),:),pointers(:,jElt),...
              integ,constit,elmap,estate);
```

The output argument `be` is the right hand side load. The inputs arguments are the same as those for `matcall` and `call`.

Matrix, load and stress computations

The calls with one input are followed by a section on element matrix assembly. For these calls the element function is expected to return an element DOF definition vector `idof` and an element matrix `k`. The type of this matrix is given in `opt(1)`. If `opt(1)==0`, both a stiffness `k` and a mass matrix `m` should be returned. See the `fe_mk MatType` section for a current list.

Take a look at `bar1` which is a very simple example of element function.

A typical element assembly section is as follows :

```

% elem0 matrix assembly section

% figure out what the input arguments are
node=CAM;  elt=varargin{1};
point=varargin{2};  integ=varargin{3};
constit=varargin{4};  elmap=varargin{5};
typ=point(5);

% outputs are [k,m] for opt(1)==0
%             [mat] for other opt(1)
switch point(5)
case 0
    [out,out1] = ... % place stiffness in out and mass in out1
case 1
    out= ... % compute stiffness
case 2
    out= ... % compute mass
case 100
    out= ... % compute right hand side
case 200
    out= ... % compute stress ...
otherwise
    error('Not a supported matrix type');
end

```

Distributed load computations (surface and volume) are handled by `fe_load`. Stress computations are handled by `fe_stress`.

There is currently no automated mechanism to allow users to integrate such computations for their own elements without modifying `fe_load` and `fe_stress`, but this will appear later since it is an obvious maintenance requirement.

The mechanism that will be used will be similar to that used for matrix assembly. The element function will be required to provide calling formats when called with `elem0('fsurf')` for surface loads, `elem0('fvol')` for volume loads, and `elem0('stress')` for stresses. `fe_load` and `fe_stress` will then evaluate these calls for each element.

7.17 Variable names and programming rules (syntax)

The following rules are used in programming SDT and OpenFEM as it makes reading the source code easier.

<code>carg</code>	index of current argument. For functions with variable number of inputs, one seeks the next argument with <code>NewArg=varargin{carg};carg=carg+1;</code>
<code>CAM,Cam</code>	string command to be interpreted. <code>Cam</code> is the lower case version of <code>CAM</code> .
<code>j1,j2,j3 ...</code>	loop indices.
<code>jGroup,jElt,jW</code>	indices for element groups, elements, integration points. For code samples use <code>help('getegroup')</code>
<code>i,j</code>	unit imaginary $\sqrt{-1}$. <code>i,j</code> should never be used as indices to avoid any problem overloading their default value.
<code>i1,i2,i3 ...</code>	integer values intermediate variables
<code>r1,r2,r3 ...</code>	real valued variables or structures
<code>ind,in2,in3 ...</code>	vectors of indices, <code>cind</code> is used to store the complement of <code>ind</code> when applicable.
<code>out,out1,out2</code>	output variables.
...	

The following names are also used throughout the toolbox functions

<code>node,FNode</code>	nodes
<code>NNode</code>	reindexing vector verifies <code>NodeInd=NNode(NodeId)</code> . Can be built using <code>NNode=sparse(node(:,1),1,1:size(node,1))</code> .
<code>RunOpt</code>	run options a structure used to store options that are used in a command, these should be

7.18 Legacy information

This section gives data that is no longer used but is important enough not to be deleted.

7.18.1 Legacy 2D elements

These elements support isotropic and 2-D anisotropic materials declared with a material entry described in `m_elastic`. Element property declarations are `p_solid` subtype 2 entries

```
[ProId fe_mat('p_solid','SI',2) f N 0]
```

Where

`f` Formulation : 0 plane stress, 1 plane strain, 2 axisymmetric.
`N` Fourier coefficient for axisymmetric formulations
`Integ` set to zero to select this family of elements.

The xy plane is used with displacement DOFs `.01` and `.02` given at each node. Element matrix calls are implemented using `.c` files called by `of_mk_subs.c` and handled by the element function itself, while load computations are handled by `fe_load`. For integration rules, see section 7.18.2. The following elements are supported

- `q4p` (`plane stress/strain`) uses the `et*2q1d` routines for plane stress and plane strain.
- `q4p` (`axisymmetric`) uses the `et*aq1d` routines for axisymmetry. The radial u_r and axial u_z displacement are bilinear functions over the element.
- `q5p` (`plane stress/strain`) uses the `et*5noe` routines for axisymmetry.
There are five nodes for this incompressible quadrilateral element, four nodes at the vertices and one at the intersection of the two diagonals.
- `q8p` uses the `et*2q2c` routines for plane stress and plane strain and `et*aq2c` for axisymmetry.
- `q9a` is a plane axisymmetric element with Fourier support. It uses the `e*aq2c` routines to generate matrices.
- `t3p` uses the `et*2p1d` routines for plane stress and plane strain and `et*ap1d` routines for axisymmetry.

The displacement (u,v) are assumed to be linear functions of (x,y) (*Linear Triangular Element*), thus the strain are constant (*Constant Strain Triangle*).

- `t6p` uses the `et*2p2c` routines for plane stress and plane strain and `et*ap2c` routines for axisymmetry.

7.18.2 Rules for elements in `of_mk_subs`

hexa8, hexa20

The `hexa8` and `hexa20` elements are the standard 8 node 24 DOF and 20 node 60 DOF brick elements.

The `hexa8` element uses the `et*3q1d` routines.

`hexa8` volumes are integrated at 8 Gauss points

$$\omega_i = \frac{1}{8} \text{ for } i = 1, 4$$

b_i for $i = 1, 4$ as below, with $z = \alpha_1$

b_i for $i = 4, 8$ as below, with $z = \alpha_2$

`hexa8` surfaces are integrated using a 4 point rule

$$\omega_i = \frac{1}{4} \text{ for } i = 1, 4$$

$b_1 = (\alpha_1, \alpha_1)$, $b_2 = (\alpha_2, \alpha_1)$, $b_3 = (\alpha_2, \alpha_2)$ and $b_4 = (\alpha_1, \alpha_2)$

with $\alpha_1 = \frac{1}{2} - \frac{1}{2\sqrt{3}} = 0.2113249$ and $\alpha_2 = \frac{1}{2} + \frac{1}{2\sqrt{3}} = 0.7886751$.

The `hexa20` element uses the `et*3q2c` routines.

`hexa20` volumes are integrated at 27 Gauss points $\omega_l = w_i w_j w_k$ for $i, j, k = 1, 3$

with

$$w_1 = w_3 = \frac{5}{18} \text{ and } w_2 = \frac{8}{18} \text{ } b_l = (\alpha_i, \alpha_j, \alpha_k) \text{ for } i, j, k = 1, 3$$

with

$$\alpha_1 = \frac{1 - \sqrt{\frac{3}{5}}}{2}, \alpha_2 = 0.5 \text{ and } \alpha_3 = \frac{1 + \sqrt{\frac{3}{5}}}{2}$$

$$\alpha_1 = \frac{1 - \sqrt{\frac{3}{5}}}{2}, \alpha_2 = 0.5 \text{ and}$$

`hexa20` surfaces are integrated at 9 Gauss points $\omega_k = w_i w_j$ for $i, j = 1, 3$ with

w_i as above and $b_k = (\alpha_i, \alpha_j)$ for $i, j = 1, 3$

$$\text{with } \alpha_1 = \frac{1 - \sqrt{\frac{3}{5}}}{2}, \alpha_2 = 0.5 \text{ and } \alpha_3 = \frac{1 + \sqrt{\frac{3}{5}}}{2}.$$

penta6, **penta15**

The **penta6** and **penta15** elements are the standard 6 node 18 DOF and 15 node 45 DOF pentahedral elements. A derivation of these elements can be found in [42].

The **penta6** element uses the **et*3r1d** routines.

penta6 volumes are integrated at 6 Gauss points

Points b_k	x	y	z
1	a	a	c
2	b	a	c
3	a	b	c
4	a	a	d
5	b	a	d
6	a	b	d

with $a = \frac{1}{6} = .16667$, $b = \frac{4}{6} = .66667$, $c = \frac{1}{2} - \frac{1}{2\sqrt{3}} = .21132$, $d = \frac{1}{2} + \frac{1}{2\sqrt{3}} = .78868$

penta6 surfaces are integrated at 3 Gauss points for a triangular face (see **tetra4**) and 4 Gauss points for a quadrangular face (see **hexa8**).

penta15 volumes are integrated at 21 Gauss points with the 21 points formula

$$a = \frac{9-2\sqrt{15}}{21}, b = \frac{9+2\sqrt{15}}{21},$$

$$c = \frac{6+\sqrt{15}}{21}, d = \frac{6-\sqrt{15}}{21},$$

$$e = 0.5(1 - \sqrt{\frac{3}{5}}),$$

$$f = 0.5 \text{ and } g = 0.5(1 + \sqrt{\frac{3}{5}})$$

$$\alpha = \frac{155-\sqrt{15}}{2400}, \beta = \frac{5}{18},$$

$$\gamma = \frac{155+\sqrt{15}}{2400}, \delta = \frac{9}{80} \text{ and } \epsilon = \frac{8}{18}.$$

Positions and weights of the 21 Gauss point are

Points b_k	x	y	z	weight ω_k
1	d	d	e	$\alpha.\beta$
2	b	d	e	$\alpha.\beta$
3	d	b	e	$\alpha.\beta$
4	c	a	e	$\gamma.\beta$
5	c	c	e	$\gamma.\beta$
6	a	c	e	$\gamma.\beta$
7	$\frac{1}{3}$	$\frac{1}{3}$	e	$\delta.\beta$
8	d	d	f	$\alpha.\epsilon$
9	b	d	f	$\alpha.\epsilon$
10	d	b	f	$\alpha.\epsilon$
11	c	a	f	$\gamma.\epsilon$
12	c	c	f	$\gamma.\epsilon$
13	a	c	f	$\gamma.\epsilon$
14	$\frac{1}{3}$	$\frac{1}{3}$	f	$\delta.\epsilon$
15	d	d	g	$\alpha.\beta$
16	b	d	g	$\alpha.\beta$
17	d	b	g	$\alpha.\beta$
18	c	a	g	$\gamma.\beta$
19	c	c	g	$\gamma.\beta$
20	a	c	g	$\gamma.\beta$
21	$\frac{1}{3}$	$\frac{1}{3}$	g	$\delta.\beta$

[penta15](#) surfaces are integrated at 7 Gauss points for a triangular face (see [tetra10](#)) and 9 Gauss points for a quadrangular face (see [hexa20](#)).

[tetra4](#), [tetra10](#)

The [tetra4](#) element is the standard 4 node 12 DOF trilinear isoparametric solid element. [tetra10](#) is the corresponding second order element.

You should be aware that this element can perform very badly (for poor aspect ratio, particular loading conditions, etc.) and that higher order elements should be used instead.

The [tetra4](#) element uses the [et*3p1d](#) routines.

[tetra4](#) volumes are integrated at the 4 vertices $\omega_i = \frac{1}{4}$ for $i = 1, 4$ and $b_i = S_i$ the i -th element vertex.

[tetra4](#) surfaces are integrated at the 3 vertices with $\omega_i = \frac{1}{3}$ for $i = 1, 3$ and $b_i = S_i$

the i -th vertex of the actual face

The `tetra10` element is second order and uses the `et*3p2c` routines.

`tetra10` volumes are integrated at 15 Gauss points

Points b_k	λ_1	λ_2	λ_3	λ_4	weight ω_k
1	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{1}{4}$	$\frac{8}{405}$
2	b	a	a	a	α
3	a	b	a	a	α
4	a	a	b	a	α
5	a	a	a	b	α
6	d	c	c	c	β
7	c	d	c	c	β
8	c	c	d	c	β
9	c	c	c	d	β
10	e	e	f	f	γ
11	f	e	e	f	γ
12	f	f	e	e	γ
13	e	f	f	e	γ
14	e	f	e	f	γ
15	f	e	f	e	γ

with $a = \frac{7-\sqrt{15}}{34} = 0.0919711$, $b = \frac{13+3\sqrt{15}}{34} = 0.7240868$, $c = \frac{7+\sqrt{15}}{34} = 0.3197936$,
 $d = \frac{13-3\sqrt{15}}{34} = 0.0406191$, $e = \frac{10-2\sqrt{15}}{40} = 0.0563508$, $f = \frac{10+2\sqrt{15}}{40} = 0.4436492$

and $\alpha = \frac{2665+14\sqrt{15}}{226800}$, $\beta = \frac{2665-14\sqrt{15}}{226800}$ et $\gamma = \frac{5}{567}$

λ_j for $j = 1, 4$ are barycentric coefficients for each vertex S_j :

$b_k = \sum_{j=1,4} \lambda_j S_j$ for $k = 1, 15$

`tetra10` surfaces are integrated using a 7 point rule

Points b_k	λ_1	λ_2	λ_3	weight ω_k
1	c	d	c	α
2	d	c	c	α
3	c	c	d	α
4	b	b	a	β
5	a	b	b	β
6	b	a	b	β
7	$\frac{1}{3}$	$\frac{1}{3}$	$\frac{1}{3}$	γ

with $\gamma = \frac{9}{80} = 0.11250$, $\alpha = \frac{155-\sqrt{15}}{2400} = 0.06296959$, $\beta = \frac{155+\sqrt{15}}{2400} = 0.066197075$
and $a = \frac{9-2\sqrt{15}}{21} = 0.05961587$, $b = \frac{6+\sqrt{15}}{21} = 0.47014206$, $c = \frac{6-\sqrt{15}}{21} = 0.10128651$,
 $d = \frac{9+2\sqrt{15}}{21} = 0.797427$

λ_j for $j = 1, 3$ are barycentric coefficients for each surface vertex S_j :

$$b_k = \sum_{j=1,3} \lambda_j S_j \text{ for } k = 1, 7$$

q4p (plane stress/strain)

The displacement (u,v) are bilinear functions over the element.

For surfaces, **q4p** uses numerical integration at the corner nodes with $\omega_i = \frac{1}{4}$ and $b_i = S_i$ for $i = 1, 4$.

For edges, **q4p** uses numerical integration at each corner node with $\omega_i = \frac{1}{2}$ and $b_i = S_i$ for $i = 1, 2$.

q4p (axisymmetric)

For surfaces, **q4p** uses a 4 point rule with

- $\omega_i = \frac{1}{4}$ for $i = 1, 4$
- $b_1 = (\alpha_1, \alpha_1)$, $b_2 = (\alpha_2, \alpha_1)$, $b_3 = (\alpha_2, \alpha_2)$, $b_4 = (\alpha_1, \alpha_2)$
with $\alpha_1 = \frac{1}{2} - \frac{1}{2\sqrt{3}} = 0.2113249$ and $\alpha_2 = \frac{1}{2} + \frac{1}{2\sqrt{3}} = 0.7886751$

For edges, **q4p** uses a 2 point rule with

- $\omega_i = \frac{1}{2}$ for $i = 1, 2$
- $b_1 = \alpha_1$ and $b_2 = \alpha_2$ the 2 gauss points of the edge.

q5p (plane stress/strain)

For surfaces, **q5p** uses a 5 point rule with $b_i = S_i$ for $i = 1, 4$ the corner nodes and b_5 the node 5.

For edges, **q5p** uses a 1 point rule with $\omega = \frac{1}{2}$ and b the midside node.

q8p (plane stress/strain)

For surfaces, **q8p** uses a 9 point rule with

- $\omega_k = w_i w_j$ for $i, j = 1, 3$ with $w_1 = w_3 = \frac{5}{18}$ et $w_2 = \frac{8}{18}$
- $b_k = (\alpha_i, \alpha_j)$ for $i, j = 1, 3$ with $\alpha_1 = \frac{1-\sqrt{\frac{3}{5}}}{2}$, $\alpha_2 = 0.5$ and $\alpha_3 = \frac{1+\sqrt{\frac{3}{5}}}{2}$

For edges, **q8p** uses a 3 point rule with

- $\omega_1 = \omega_2 = \frac{1}{6}$ and $\omega_3 = \frac{4}{6}$
- $b_i = S_i$ for $i = 1, 2$ corner nodes of the edge et b_3 the midside.

q8p (axisymmetric)

For surfaces, **q8p** uses a 9 point rule with

- $\omega_k = w_i w_j$ for $i, j = 1, 3$
with $w_1 = w_3 = \frac{5}{18}$ and $w_2 = \frac{8}{18}$
- $b_k = (\alpha_i, \alpha_j)$ for $i, j = 1, 3$
with $\alpha_1 = \frac{1-\sqrt{\frac{3}{5}}}{2}$, $\alpha_2 = 0.5$ and $\alpha_3 = \frac{1+\sqrt{\frac{3}{5}}}{2}$

For edges, **q8p** uses a 3 point rule with

- $\omega_1 = \omega_3 = \frac{5}{18}$, $\omega_2 = \frac{8}{18}$
- $b_1 = \frac{1-\sqrt{\frac{3}{5}}}{2} = 0.1127015$, $b_2 = 0.5$ and $b_3 = \frac{1+\sqrt{\frac{3}{5}}}{2} = 0.8872985$

t3p (plane stress/strain)

For surfaces, **t3p** uses a 3 point rule at the vertices with $\omega_i = \frac{1}{3}$ and $b_i = S_i$.

For edges, **t3p** uses a 2 point rule at the vertices with $\omega_i = \frac{1}{2}$ and $b_i = S_i$.

t3p (axisymmetric)

For surfaces, **t3p** uses a 1 point rule at the barycenter ($b_1 = G$) with $\omega_1 = \frac{1}{2}$.

For edges, **t3p** uses a 2 point rule at the vertices with $\omega_i = \frac{1}{2}$ and $b_1 = \frac{1}{2} - \frac{2}{2\sqrt{3}}$ and $b_2 = \frac{1}{2} + \frac{2}{2\sqrt{3}}$.

t6p (plane stress/strain)

For surfaces, **t6p** uses a 3 point rule with

- $\omega_i = \frac{1}{3}$ for $i = 1, 6$
- $b_i = S_{i+3, i+4}$ the three midside nodes.

For edges, **t6p** uses a 3 point rule

- $\omega_1 = \omega_2 = \frac{1}{6}$ and $\omega_3 = \frac{4}{6}$
- $b_i = S_i, i = 1, 2$ the i -th vertex of the actual edge and $b_3 = S_{i, i+1}$ the midside.

t6p (axisymmetric)

For surfaces, **t6p** uses a 7 point rule

Points b_k	λ_1	λ_2	λ_3	weight ω_k
1	$\frac{1}{3}$	$\frac{1}{3}$	$\frac{1}{3}$	a
2	α	β	β	b
3	β	β	α	b
4	β	α	β	b
5	γ	γ	δ	c
6	δ	γ	γ	c
7	γ	δ	γ	c

with :

$$a = \frac{9}{80} = 0.11250, \quad b = \frac{155 + \sqrt{15}}{2400} = 0.066197075 \text{ and}$$

$$c = \frac{155 - \sqrt{15}}{2400} = 0.06296959$$

$$\alpha = \frac{9 - 2\sqrt{15}}{21} = 0.05961587, \quad \beta = \frac{6 + \sqrt{15}}{21} = 0.47014206$$

$$\gamma = \frac{6 - \sqrt{15}}{21} = 0.10128651, \quad \delta = \frac{9 + 2\sqrt{15}}{21} = 0.797427$$

λ_j for $j = 1, 3$ are barycentric coefficients for each vertex S_j :

$$b_k = \sum_{j=1,3} \lambda_j S_j \text{ for } k = 1, 7$$

For edges, **t6p** uses a 3 point rule with $\omega_1 = \omega_3 = \frac{5}{18}$, $\omega_2 = \frac{8}{18}$

$$b_1 = \frac{1 - \sqrt{\frac{3}{5}}}{2} = 0.1127015, \quad b_2 = 0.5 \text{ and } b_3 = \frac{1 + \sqrt{\frac{3}{5}}}{2} = 0.8872985$$

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Element functions supported by *OpenFEM* are listed below. The rule is to have element families (2D and 3D) with families of formulations selected through element properties and implemented for all standard shapes

3-D VOLUME ELEMENT SHAPES	
hexa8	8-node 24-DOF brick
hexa20	20-node 60-DOF brick
hexa27	27-node 81-DOF brick
penta6	6-node 18-DOF pentahedron
penta15	15-node 45-DOF pentahedron
tetra4	4-node 12-DOF tetrahedron
tetra10	10-node 30-DOF tetrahedron

2-D VOLUME ELEMENT SHAPES	
q4p	4-node quadrangle
q5p	5-node quadrangle
q8p	8-node quadrangle
q9a	9-node quadrangle
t3p	3-node 6-DOF triangle
t6p	6-node 12-DOF triangle

Supported problem formulations are listed in section 6.1, in particular one considers 2D and 3D elasticity, acoustics, hyperelasticity, fluid/structure coupling, piezoelectric volumes, ...

Other elements, non generic elements, are listed below

3-D PLATE/SHELL ELEMENTS	
dktp	3-node 9-DOF discrete Kirchoff plate
mitc4	4-node 20-DOF shell
quadb	quadrilateral 4-node 20/24-DOF plate/shell
quad9	(display only)
quadb	quadrilateral 8-node 40/48-DOF plate/shell
tria3	3-node 15/18-DOF thin plate/shell element
tria6	6-node 36DOF thin plate/shell element

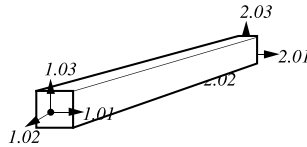
OTHER ELEMENTS	
<code>bar1</code>	standard 2-node 6-DOF bar
<code>beam1</code>	standard 2-node 12-DOF Bernoulli-Euler beam
<code>beam1t</code>	pretensioned 2-node 12-DOF Bernoulli-Euler beam
<code>beam3</code>	(display only)
<code>celas</code>	scalar springs and penalized rigid links
<code>mass1</code>	concentrated mass/inertia element
<code>mass2</code>	concentrated mass/inertia element with offset
<code>rigid</code>	handling of linearized rigid links

UTILITY ELEMENTS	
<code>fe_super</code>	element function for general superelement support
<code>integrules</code>	FEM integration rule support
<code>fsc</code>	fluid/structure coupling capabilities

bar1

Purpose Element function for a 6 DOF traction-compression bar element.

Description The [bar1](#) element corresponds to the standard linear interpolation for axial traction-compression. The element DOFs are the standard translations at the two end nodes (DOFs [.01](#) to [.03](#)).



In a model description matrix, *element property rows* for [bar1](#) elements follow the standard format (see section 7.16).

```
[n1 n2 MatID ProID EltID]
```

Isotropic elastic materials are the only supported (see [m_elastic](#)).

For supported element properties see [p_beam](#). Currently, [bar1](#) only uses the element area [A](#) with the format

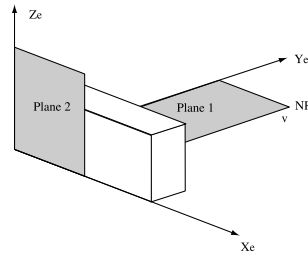
```
[ProID Type 0 0 0 A]
```

See also [m_elastic](#), [p_beam](#), [fe_mk](#), [feplot](#)

beam1, beam1t

Purpose Element functions for a 12 DOF beam element. `beam1t` is a 2 node beam with pretension available for non-linear cable statics and dynamics.

Description `beam1`



In a model description matrix, *element property* rows for `beam1` elements follow the format

```
[n1 n2 MatID ProID nr 0 0 EltID p1 p2 x1 y1 z1 x2 y2 z2]
```

where

<code>n1,n2</code>	node numbers of the nodes connected
<code>MatID</code>	material property identification number
<code>ProID</code>	element section property identification number
<code>nr 0 0</code>	number of node not in the beam direction defining bending plane 1 in this case $\{v\}$ is the vector going from <code>n1</code> to <code>nr</code> . If <code>nr</code> is undefined it is assumed to be located at position $[1.5 \ 1.5 \ 1.5]$.
<code>vx vy vz</code>	alternate method for defining the bending plane 1 by giving the components of a vector in the plane but not collinear to the beam axis. If <code>vy</code> and <code>vz</code> are zero, vx must not be an integer . <code>MAP=beam1t('map',model)</code> returns a normal vector MAP giving the vector used for bending plane 1. This can be used to check your model.
<code>p1,p2</code>	pin flags. These give a list of DOFs to be released (condensed before assembly). For example, 456 will release all rotation degrees of freedom. Note that the DOFS are defined in the local element coordinate system.
<code>x1,...</code>	optional components in global coordinate system of offset vector at node 1 (default is no offset)
<code>x2,...</code>	optional components of offset vector at node 2

Isotropic elastic materials are the only supported (see [m_elastic](#)). [p_beam](#) describes the section property format and associated formulations.

Failure to define orientations is a typical error with beam models. In the following example, the definition of bending plane 1 using a vector is illustrated.

```
cf=fepplot(femesh('test2bay'));
% Map is in very variable direction due to undefined nr
% This is only ok for sections invariant by rotation
beam1t('map',cf.mdl);fecom('view3');

% Now define generator for bending plane 1
i1=feutil('findelt elname beam1',cf.mdl); % element row index
cf.mdl.Elt(i1,5:7)=ones(size(i1))*[-.1 .9 0]; % vx vy vz
beam1t('map',cf.mdl);fecom('view2');
```

beam1t

This element has an internal state stored in a [InfoAtNode](#) structure where each column of `Case.GroupInfo{7}.data` gives the local basis, element length and tension [`bas(:);L;ten`]. Initial tension can be defined using a `.MAP` field in the element property.

This is a simple example showing how to impose a pre-tension :

```
model=femesh('TestBeam1 divide 10');
model=fe_case(model,'FixDof','clamp',[1;2;.04;.02;.01;.05]);
model.Elt=feutil('SetGroup 1 name beam1t',model);
d1=fe_eig(model,[5 10]);
model=feutil('setpro 112',model,'MAP', ...
    struct('dir',{{'1.5e6'}},'lab',{{'ten'}}));
d2=fe_eig(model,[5 10]);

figure(1);plot([d2.data./d1.data-1]);
xlabel('Mode index');ylabel('Frequency shift');
```

Strains in a non-linear Bernoulli Euler section are given by

$$\epsilon_{11} = \left(\frac{\partial u}{\partial x} + \frac{1}{2} \left(\frac{\partial w_0}{\partial x} \right)^2 \right) - z \frac{\partial^2 w_0}{\partial x^2} \quad (8.1)$$

See also [p_beam](#), [m_elastic](#), [fe_mk](#), [fepplot](#)

celas,cbush

Purpose element function for scalar springs and penalized rigid links

Description `celas`

In an model description matrix a group of `celas` elements starts with a header row `[Inf abs('celas') 0 ...]` followed by element property rows following the format

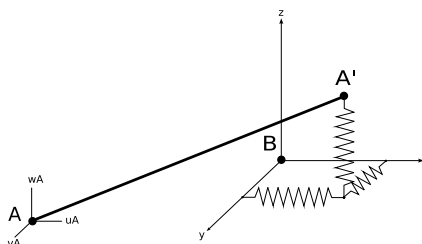
```
[n1 n2 DofID1 DofID2 ProID EltID Kv Mv Cv Bv]
```

with

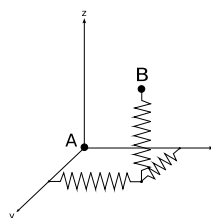
n1,n2 node numbers of the nodes connected. Grounded springs are obtained by setting **n1** or **n2** to 0.

DofID Identification of selected DOFs.

For *rigid links*, the first node defines the rigid body motion of the other extremity slave node. Motion between the slave node and the second node is then penalized. **DofID** (positive) defines which DOFs of the slave node are connected by the constraint. Thus `[1 2 123 0 0 0 1e14]` will only impose the penalization of node translations 2 by motion of node 1, while `[1 2 123456 0 0 0 1e14]` will also penalize the difference in rotations.



For *scalar springs*, **DofID1** (negative) defines which DOFs of node 1 are connected to which of node 2. **DofID2** can be used to specify different DOFs on the 2 nodes. For example `[1 2 -123 231 0 0 1e14]` connects DOFs 1.01 to 2.02, etc.



ProID Optional property identification number (see format below)

Kv Optional stiffness value used as a weighting associated with the constraint. If **Kv** is zero (or not given), the default value in the element property declaration is used. If this is still zero, **Kv** is set to `1e14`.

p_spring properties for `celas` elements take the form `[ProID type KvDefault m c eta S]`

Below is the example of a 2D beam on elastic supports.

```
model=femesh('Testbeam1 divide 10');
model=fe_case(model,'FixDof','2D',[.01;.02;.04]);
model.Elt(end+1,1:6)=[Inf abs('celas')]; % spring supports
model.Elt(end+[1:2],1:7)=[1 0 -13 0 0 0 1e5;2 0 -13 0 0 0 1e5];
def=fe_eig(model,[5 10 0]); feplot(model,def);
```

cbush

The element property row is defined by

```
[n1 n2 MatId ProId EltId x1 x2 x3 CID S OCID S1 S2 S3]
```

The orientation of the spring can be specified, by using distinct **n1,n2**, giving components **x1,x2,x3** of an orientation vector (**x1** should not be an integer if **x2** and **x3** are zero), a node number as **NodeIdRef,0,0**, the specification of a coordinate system **CID**.

The spring/damper is located at a position interpolated between **n1** and **n2** using **S**, such that $x_i = S n_1 + (1 - S) n_2$. The midpoint is used by default, that-is-to-say **S** is taken at 0.5 if left to zero. To use other locations, specify a non-zero **OCID** and an offset **S1,S2,S3**.

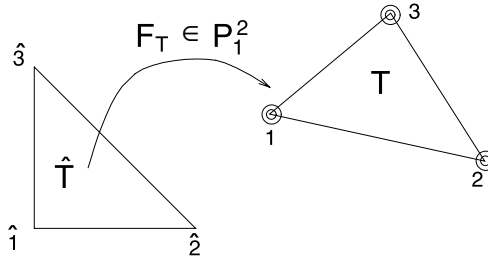
It is possible to set **n2** to 0 to define a grounded **cbush**, or to define a local **cbush** using **n1=n2**.

See also

[p-spring](#), [rigid](#)

Purpose 2-D 9-DOF Discrete Kirchhoff triangle

Description



In a model description matrix, **element property rows** for [dktp](#) elements follow the standard format

`[n1 n2 n3 MatID ProID EltID Theta]`

giving the node identification numbers **ni**, material **MatID**, property **ProID**. Other **optional** information is **EltID** the element identifier, **Theta** the angle between material x axis and element x axis (currently unused)

The elements support isotropic materials declared with a material entry described in [m_elastic](#). Element property declarations follow the format described in [p_shell](#).

The [dktp](#) element uses the `et*dktp` routines.

There are three vertices nodes for this triangular Kirchhoff plate element and the normal deflection $W(x, y)$ is cubic along each edge.

We start with a 6-node triangular element with a total $D.O.F = 21$:

- five degrees of freedom at corner nodes :

$$W(x, y), \frac{\partial W}{\partial x}, \frac{\partial W}{\partial y}, \theta_x, \theta_y \text{ (deflection } W \text{ and rotations } \theta)$$

- two degrees of freedom θ_x and θ_y at mid side nodes.

Then, we impose no transverse shear deformation $\gamma_{xz} = 0$ and $\gamma_{yz} = 0$ at selected nodes to reduce the total $DOF = 21 - 6 * 2 = 9$:

- three degrees of freedom at each of the vertices of the triangle.

$$W(x, y), \theta_x = \left(\frac{\partial W}{\partial x}\right), \theta_y = \left(\frac{\partial W}{\partial y}\right)$$

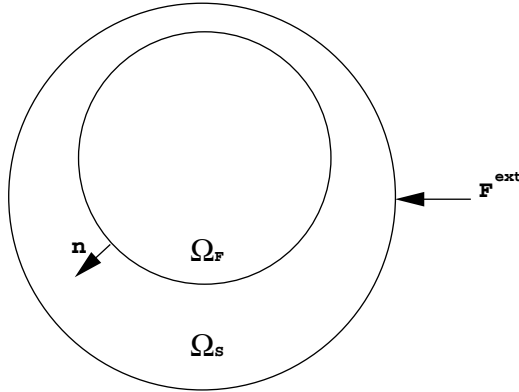
The coordinates of the reference element's vertices are $\hat{S}_1(0., 0.)$, $\hat{S}_2(1., 0.)$ and $\hat{S}_3(0., 1.)$.

Surfaces are integrated using a 3 point rule $\omega_k = \frac{1}{3}$ and b_k mid side node.

See also [fe_mat](#), [m_elastic](#), [p_shell](#), [fe_mk](#), [feplot](#)

Purpose Fluid structure/coupling with non-linear follower pressure support.

Description Elasto-acoustic coupling is used to model structures containing a compressible, non-weighting fluid, with or without a free surface.



The FE formulation for this type of problem can be written as [43]

$$s^2 \begin{bmatrix} M & 0 \\ C^T & K_p \end{bmatrix} \begin{Bmatrix} q \\ p \end{Bmatrix} + \begin{bmatrix} K(s) & -C \\ 0 & F \end{bmatrix} \begin{Bmatrix} q \\ p \end{Bmatrix} = \begin{Bmatrix} F^{ext} \\ 0 \end{Bmatrix} \quad (8.2)$$

with q the displacements of the structure, p the pressure variations in the fluid and F^{ext} the external load applied to the structure, where

$$\begin{aligned} \int_{\Omega_S} \sigma_{ij}(u) \epsilon_{ij}(\delta u) dx &\Rightarrow \delta q^T K q \\ \int_{\Omega_S} \rho_S u \cdot \delta u dx &\Rightarrow \delta q^T M q \\ \frac{1}{\rho_F} \int_{\Omega_F} \nabla p \nabla \delta p dx &\Rightarrow \delta p^T F p \\ \frac{1}{\rho_F c^2} \int_{\Omega_F} p \delta p dx &\Rightarrow \delta p^T K_p p \\ \int_{\Sigma} p \delta u \cdot n dx &\Rightarrow \delta q^T C p \end{aligned} \quad (8.3)$$

To assemble fluid/structure coupling matrix you should declare a set of surface elements (any topology) with property `p_solid('dbval 1 fsc')`. The C matrix (solid forces induced by pressure field) is assembled with the stiffness (matrix type

1), while the C^T matrix (fluid pressure due to normal velocity of solid) is assembled with the mass (matrix type 2).

Some formulations, consider a surface impedance proportional to the pressure. This matrix can be computed by defining a group of surface elements with an acoustic material (see `m.elastic 2`) and a standard surface integration rule (`p.solid('dbval 1 d2 -3')`). This results in a mass given by

$$\delta p^T K_{pp} = \frac{1}{\rho_F c^2} \int_{\Omega_F} \delta p p dx \quad (8.4)$$

Follower force One uses the identity

$$n dS = \frac{\partial \underline{x}}{\partial r} \wedge \frac{\partial \underline{x}}{\partial s} dr ds, \quad (8.5)$$

where (r, s) designate local coordinates of the face (assumed such that the normal is outgoing). Work of the pressure is thus:

$$\delta W_p = - \int_{r,s} \Pi \left(\frac{\partial \underline{x}}{\partial r} \wedge \frac{\partial \underline{x}}{\partial s} \right) \cdot \delta \underline{v} dr ds. \quad (8.6)$$

On thus must add the non-linear stiffness term:

$$- d\delta W_p = \int_{r,s} \Pi \left(\frac{\partial d\underline{u}}{\partial r} \wedge \frac{\partial \underline{x}}{\partial s} + \frac{\partial \underline{x}}{\partial r} \wedge \frac{\partial d\underline{u}}{\partial s} \right) \cdot \delta \underline{v} dr ds. \quad (8.7)$$

Using $\frac{\partial \underline{x}}{\partial r} = \{x_{1,r} \ x_{2,r} \ x_{3,r}\}^T$ (idem for s), and also

$$[Axr] = \begin{pmatrix} 0 & -x_{,r3} & x_{,r2} \\ x_{,r3} & 0 & -x_{,r1} \\ -x_{,r2} & x_{,r1} & 0 \end{pmatrix}, \quad [Axs] = \begin{pmatrix} 0 & -x_{,s3} & x_{,s2} \\ x_{,s3} & 0 & -x_{,s1} \\ -x_{,s2} & x_{,s1} & 0 \end{pmatrix},$$

this results in

$$\left(\frac{\partial d\underline{x}}{\partial r} \wedge \frac{\partial \underline{x}}{\partial s} + \frac{\partial \underline{x}}{\partial r} \wedge \frac{\partial d\underline{x}}{\partial s} \right) \cdot \delta \underline{v} = \{\delta q_{ik}\}^T \{N_k\} (Axr_{ij} \{N_{l,s}\}^T - Axs_{ij} \{N_{l,r}\}^T) \{dq_j\}. \quad (8.8)$$

Tests : `fsc3 testsimple` and `fsc3 test`.

In the RivlinCube test , the pressure on each free face is given by

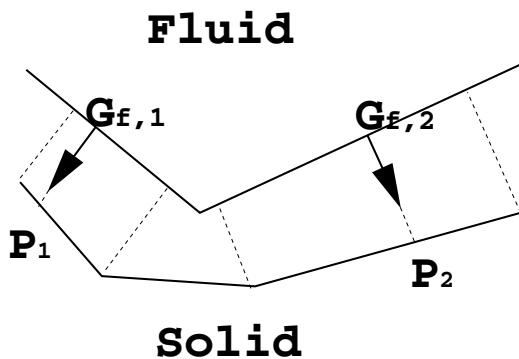
$$\begin{aligned} \Pi_1 &= -\frac{1+\lambda_1}{(1+\lambda_2)(1+\lambda_3)} \Sigma_{11} \quad \text{on face } (x_1 = l_1) \\ \Pi_2 &= -\frac{1+\lambda_2}{(1+\lambda_1)(1+\lambda_3)} \Sigma_{22} \quad \text{on face } (x_2 = l_2) \\ \Pi_3 &= -\frac{1+\lambda_3}{(1+\lambda_1)(1+\lambda_2)} \Sigma_{33} \quad \text{on face } (x_3 = l_3). \end{aligned}$$

Non-conform SDT supports non conforming element for fluid/structure coupling terms corresponding to the structure are computed using the classical elements of the SDT, and terms corresponding to the fluid are computed using the fluid elements (see [flui4](#)).

The coupling term C is computed using fluid/structure coupling elements ([fsc](#) elements).

Only one integration point on each element (the centre of gravity) is used to evaluate C .

When structural and fluid meshes do not match at boundaries, pairing of elements needs to be done. The pairing procedure can be described for each element. For each fluid element F_i , one takes the center of gravity $G_{f,i}$ (see figure), and searches the solid element S_i which is in front of the center of gravity, in the direction of the normal to the fluid element F_i . The projection of $G_{f,i}$ on the solid element, P_i , belongs to S_i , and one computes the reference coordinate r and s of P_i in S_i (if S_i is a quad4, $-1 < r < 1$ and $-1 < s < 1$). Thus one knows the weights that have to be associated to each node of S_i . The coupling term will associate the DOFs of F_i to the DOFs of S_i , with the corresponding weights.



See also [flui4, m_elastic](#)

hexa8, penta6, tetra4, and other 3D volumes ---

Purpose Topology holders for 3D volume elements.

Description The [hexa8](#) [hexa20](#) [hexa27](#), [penta6](#) [penta15](#) [tetra4](#) and [tetra10](#) elements are standard topology reference for 3D volume FEM problems.

In a model description matrix, **element property rows** for [hexa8](#) and [hexa20](#) elements follow the standard format with no element property used. The generic format for an element containing i nodes is `[n1 ... ni MatID ProId EltId]`. For example, the [hexa8](#) format is `[n1 n2 n3 n4 n5 n6 n7 n8 MatID ProId EltId]`.

These elements only define topologies, the nature of the problem to be solved should be specified using a property entry, see section 6.1 for supported problems and [p_solid](#), [p_heat](#), ... for formats.

Integration rules for various topologies are described under [integrules](#). Vertex coordinates of the reference element can be found using an [integrules](#) command containing the name of the element such as `r1=integrules('q4p');r1.xi`.

Backward compatibility note : if no element property entry is defined, or with a [p_solid](#) entry with the integration rule set to zero, the element defaults to the historical 3D mechanic elements described in section 7.18.2.

See also [fe_mat](#), [m_elastic](#), [fe_mk](#), [feplot](#)

.

integrules

Purpose Command function for FEM integration rule support.

Description This function groups integration rule manipulation utilities used by various elements. The following calls generate the reference `EltConst` data structure (see section 7.15.4).

Gauss

This command supports the definition of Gauss points and associated weights. It is called with `integrules('Gauss Topology',RuleNumber)`. Supported topologies are `1d` (line), `q2d` (2D quadrangle), `t2d` (2D triangle), `t3d` (3D tetrahedron), `p3d` (3D prism), `h3d` (3D hexahedron). `integrules('Gauss q2d')` will list available 2D quadrangle rules.

- `Integ -3` is always the default rule for the order of the element.
- `-2` a rule at nodes.
- `-1` the rule at center.

```
[ -3] [ 0x1 double] 'element dependent default'  
[ -2] [ 0x1 double] 'node'  
[ -1] [ 1x4 double] 'center'  
[102] [ 4x4 double] 'gefdyn 2x2'  
[  2] [ 4x4 double] 'standard 2x2'  
[109] [ 9x4 double] 'Q4WT'  
[103] [ 9x4 double] 'gefdyn 3x3'  
[104] [16x4 double] 'gefdyn 4x4'  
[  9] [ 9x4 double] '9 point'  
[  3] [ 9x4 double] 'standard 3x3'  
[  2] [ 4x4 double] 'standard 2x2'  
[ 13] [13x4 double] '2x2 and 3x3'
```

bar1,beam1,beam3

For integration rule selection, these elements use the 1D rules which list you can find using `integrules('Gauss1d')`.

Geometric orientation convention for segment is $\bullet (1) \rightarrow (2)$

One can show the edge using `elt_name edge` (e.g. `beam1 edge`).

`t3p, t6p`

Vertex coordinates of the reference element can be found using `r1=integrules('tria3')`

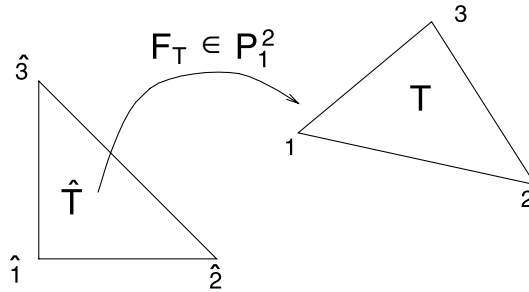


Figure 8.1: `t3p` reference element.

Vertex coordinates of the reference element can be found using `r1=integrules('tria6')`

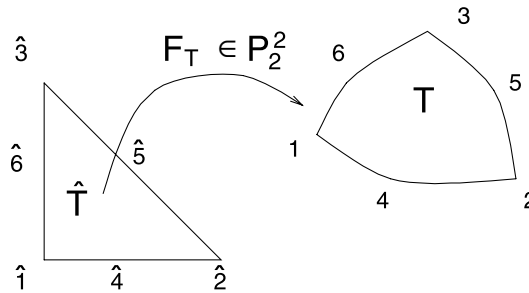


Figure 8.2: `t6p` reference element.

For integration rule selection, these elements use the 2D triangle rules which list you can find using `integrules('Gausst2d')`.

Geometric orientation convention for triangle is to number anti-clockwise in the two-dimensional case (in the three-dimensional case, there is no orientation).

- edge [1]: (1) → (2) (nodes 4, 5,... if there are supplementary nodes)
- edge [2]: (2) → (3) (...)
- edge [3]: (3) → (1) (...)

One can show the edges or faces using `elt_name edge` or `elt_name face` (e.g. `t3p edge`).

q4p, q5p, q8p

Vertex coordinates of the reference element can be found using `r1=integrules('quad4');r1.xi`

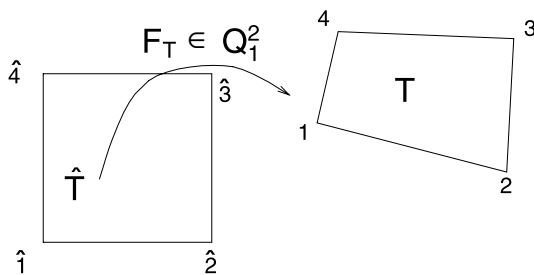


Figure 8.3: q4p reference element.

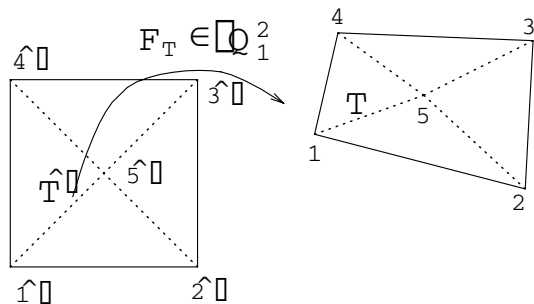


Figure 8.4: q5p reference element.

Vertex coordinates of the reference element can be found using the `r1=integrules('quadb');r1`

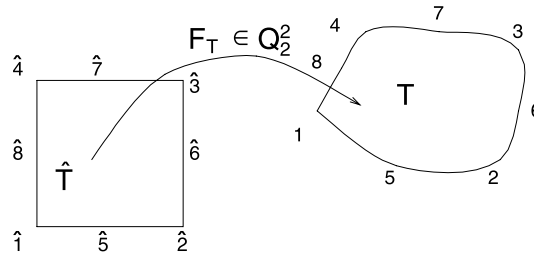


Figure 8.5: `q8p` reference element.

For integration rule selection, these elements use the 2D quadrangle rules which list you can find using `integrules('Gaussq2d')`.

Geometric orientation convention for quadrilateral is to number anti-clockwise (same remark as for the triangle)

- edge [1]: (1) → (2) (nodes 5, 6, ...)
- edge [2]: (2) → (3) (...)
- edge [3]: (3) → (4)
- edge [4]: (4) → (1)

One can show the edges or faces using `elt_name edge` or `elt_name face` (e.g. `q4p edge`).

`tetra4, tetra10`

3D tetrahedron geometries with linear and quadratic shape functions. Vertex coordinates of the reference element can be found using `r1=integrules('tetra4');r1.xi` (or command `'tetra10'`).

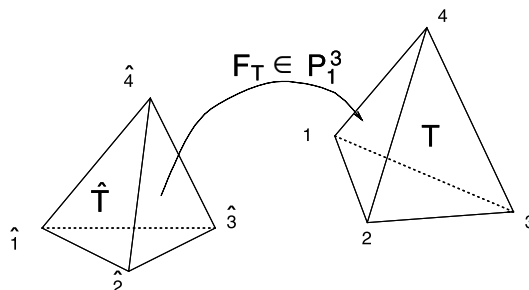


Figure 8.6: `tetra4` reference element.

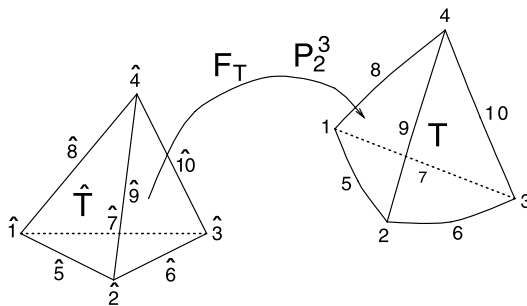


Figure 8.7: `tetra10` reference element.

For integration rule selection, these elements use the 3D pentahedron rules which list you can find using `integrules('Gausst3d')`.

Geometric orientation convention for tetrahedron is to have trihedral $(\vec{12}, \vec{13}, \vec{14})$ direct (\vec{ij} designates the vector from point i to point j).

- edge [1]: (1) \rightarrow (2) (nodes 5, ...)
- edge [2]: (2) \rightarrow (3) (...)
- edge [3]: (3) \rightarrow (1)
- edge [4]: (1) \rightarrow (4)
- edge [5]: (2) \rightarrow (4)
- edge [6]: (3) \rightarrow (4) (nodes ..., p)

All faces, seen from the exterior, are described anti-clockwise:

- face [1]: (1) (3) (2) (nodes $p+1$, ...)
- face [2]: (1) (4) (3) (...)
- face [3]: (1) (2) (4)
- face [4]: (2) (3) (4)

One can show the edges or faces using `elt_name edge` or `elt_name face` (e.g. `tetra10 face`).

`penta6`, `penta15`

3D prism geometries with linear and quadratic shape functions. Vertex coordinates of the reference element can be found using `r1=integrules('penta6');``r1.xi` (or command `'penta15'`).

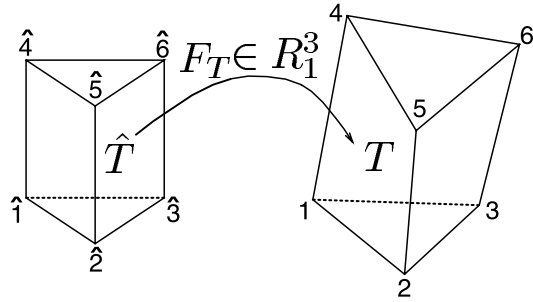


Figure 8.8: [penta6](#) reference element.

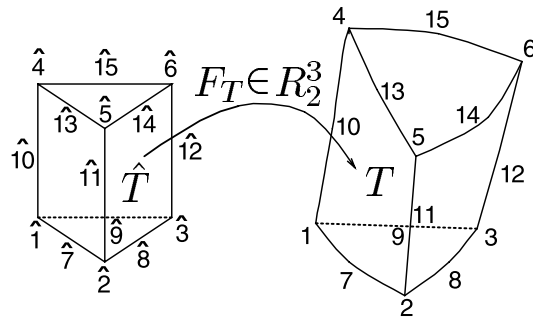


Figure 8.9: [penta15](#) reference element.

For integration rule selection, these elements use the 3D pentahedron rules which list you can find using `integrules('Gaussp3d')`.

Geometric orientation convention for pentahedron is to have trihedral $(\vec{12}, \vec{13}, \vec{14})$ direct

- edge [1]: (1) → (2) (nodes 7, ...)
- edge [2]: (2) → (3) (...)
- edge [3]: (3) → (1)
- edge [4]: (1) → (4)
- edge [5]: (2) → (5)
- edge [6]: (3) → (6)
- edge [7]: (4) → (5)
- edge [8]: (5) → (6)
- edge [9]: (6) → (4) (nodes ..., p)

All faces, seen from the exterior, are described anti-clockwise.

- face [1] : (1) (3) (2) (nodes p+1, ...)
- face [2] : (1) (4) (6) (3)
- face [3] : (1) (2) (5) (4)
- face [4] : (4) (5) (6)
- face [5] : (2) (3) (5) (6)

One can show the edges or faces using `elt_name edge` or `elt_name face` (e.g.

penta15 face).

hexa8, hexa20, hexa21, hexa27

3D brick geometries, using linear [hexa8](#), and quadratic shape functions. Vertex coordinates of the reference element can be found using `r1=integrules('hexa8');r1.xi` (or command `'hexa20'`, `'hexa27'`).

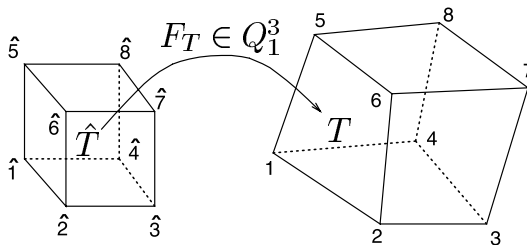


Figure 8.10: [hexa8](#) reference topology.

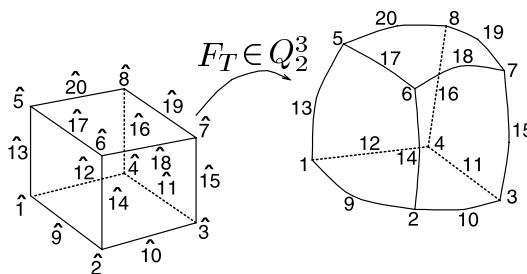


Figure 8.11: [hexa20](#) reference topology.

For integration rule selection, these elements use the 3D hexahedron rules which list you can find using `integrules('Gauss3d')`.

Geometric orientation convention for hexahedron is to have trihedral ($\vec{12}, \vec{14}, \vec{15}$) direct

- edge [1]: (1) → (2) (nodes 9, ...)
- edge [2]: (2) → (3) (...)
- edge [3]: (3) → (4)
- edge [4]: (4) → (1)
- edge [5]: (1) → (5)
- edge [6]: (2) → (6)
- edge [7]: (3) → (7)
- edge [8]: (4) → (8)
- edge [9]: (5) → (6)
- edge [10]: (6) → (7)
- edge [11]: (7) → (8)
- edge [12]: (8) → (5) (nodes ..., p)

All faces, seen from the exterior, are described anti-clockwise.

- face [1] : (1) (4) (3) (2) (nodes p+1, ...)
- face [2] : (1) (5) (8) (4)
- face [3] : (1) (2) (6) (5)
- face [4] : (5) (6) (7) (8)
- face [5] : (2) (3) (7) (6)
- face [6] : (3) (4) (8) (7)

One can show the edges or faces using `elt_name edge` or `elt_name face` (e.g. `hexa8 face`).

BuildNDN

The commands are extremely low level utilities to fill the `.NDN` field for a given set of nodes. The calling format is `of_mk('BuildNDN',type,rule,nodeE)` where `type` is an `int32` that specifies the rule to be used : 2 for 2D, 3 for 3D, 31 for 3D with xyz sorting of NDN columns, 23 for surface in a 3D model, 13 for a 3D line. A negative value can be used to switch to the `.m` file implementation in `integrules`.

The 23 rule generates a transformation with the first axis along N, r , the second axis orthogonal in the plane tangent to N, r , N, s and the third axis locally normal to the element surface. If a local material orientation is provided in columns 5 to 7 of `nodeE` then the material x axis is defined by projection on the surface. One recalls that columns of `nodeE` are field based on the `InfoAtNode` field and the first three labels should be `'v1x', 'v1y', 'v1z'`.

With the 32 rule if a local material orientation is provided in columns 5 to 7 for x and 8 to 10 for y the spatial derivatives of the shape functions are given in this local frame.

The `rule` structure is described earlier in this section and `node` has three columns that give the positions in the nodes of the current element. The `rule.NDN` and `rule.jdet` fields are modified. They must have the correct size before the call is made or severe crashes can be experienced.

If a `rule.bas` field is defined ($9 \times Nw$), each column is filled to contain the local basis at the integration point for 23 and 13 types. If a `rule.J` field with ($4 \times Nw$), each column is filled to contain the jacobian at the integration point for 23.

```
model=femesh('testhexa8'); nodeE=model.Node(:,5:7);
opt=integrules('hexa8',-1);
nodeE(:,5:10)=0; nodeE(:,7)=1; nodeE(:,8)=1; % xe=z and ye=y
integrules('buildndn',32,opt,nodeE)
```

```
model=femesh('testquad4'); nodeE=model.Node(:,5:7);
opt=integrules('q4p',-1);opt.bas=zeros(9,opt.Nw);opt.J=zeros(4,opt.Nw);
```

integrules

```
nodeE(:,5:10)=0; nodeE(:,5:6)=1; % xe= along [1,1,0]
integrules('buildndn',23,opt,nodeE)
```

See also

[elem0](#)

m_elastic

Purpose Material function for elastic solids and fluids.

Syntax

```
mat= m_elastic('default')
mat= m_elastic('database name')
pl = m_elastic('dbval MatId name');
pl = m_elastic('dbval -unit TM MatId name');
pl = m_elastic('dbval -punit TM MatId name');
```

Description This help starts by describing the main commands of `m_elastic` : `Database` and `Dbval`. Materials formats supported by `m_elastic` are then described.

`[Database,Dbval] [-unit TY] [,MatID]] Name`

A material property function is expected to store a number of standard materials. See section 7.3 for material property interface.

`m_elastic('database Steel')` returns a the data structure describing steel.

`m_elastic('dbval 100 Steel')` only returns the property row.

```
% List of materials in data base
m_elastic info
% examples of row building and conversion
pl=m_elastic([100 fe_mat('m_elastic','SI',1) 210e9 .3 7800], ...
    'dbval 101 aluminum', ...
    'dbval 200 lamina .27 3e9 .4 1200 0 790e9 .3 1780 0');
pl=fe_mat('convert SITM',pl);
pl=m_elastic(pl,'dbval -unit TM 102 steel')
```

Command option `-unit` asks the output to be converted in the desired unit system. Command option `-punit` tells the function that the provided data is in a desired unit system (and generates the corresponding type).

You can generate orthotropic shell properties using the `Dbval 100 lamina VolFrac Ef nu_f rho_f G_f E_m nu_m Rho_m G_m` command which gives fiber and matrix characteristics as illustrated above (the volume fraction is that of fiber).

The default material is steel.

To orient fully anisotropic materials, you can use the following command

```

% Behavior of a material grain assumed orthotropic
C11=168.4e9; C12=121.4e9; C44=75.4e9; % GPa
C=[C11 C12 C12 0 0 0;C12 C11 C12 0 0 0;C12 C12 C11 0 0 0;
   0 0 0 C44 0 0;    0 0 0 0 C44 0;    0 0 0 0 0 C44];

pl=[m_elastic('formulaPlAniso 1',C,basis('bunge',[5.175 1.3071 4.2012]))
    m_elastic('formulaPlAniso 2',C,basis('bunge',[2.9208 1.7377 1.3921]))

```

Subtypes `m_elastic` supports the following material subtypes

1 : standard isotropic

Standard isotropic materials, see section 6.1.1 and section 6.1.2, are described by a row of the form

```
[MatID typ E nu rho G Eta Alpha T0]
```

with `typ` an identifier generated with the `fe_mat('m_elastic','SI',1)` command, E (Young's modulus), ν (Poisson's ratio), ρ (density), G (shear modulus, set to $G = E/2(1 + \nu)$ if equal to zero). η loss factor for hysteretic damping modeling. α thermal expansion coefficient. T_0 reference temperature.

2 : acoustic fluid

Acoustic fluid, see section 6.1.3, are described by a row of the form

```
[MatId typ rho C eta]
```

with `typ` an identifier generated with the `fe_mat('m_elastic','SI',2)` command, ρ (density), C (velocity) and η (loss factor). The bulk modulus is then given by $K = \rho C^2$.

3 : 3-D anisotropic solid

3-D Anisotropic solid, see section 6.1.1, are described by a row of the form

```
[MatId typ Gij rho eta A1 A2 A3 A4 A5 A6 T0]
```

with `typ` an identifier generated with the `fe_mat('m_elastic','SI',3)` command, ρ (density), η (loss factor) and G_{ij} a row containing

```
[G11 G12 G22 G13 G23 G33 G14 G24 G34 G44 ...
 G15 G25 G35 G45 G55 G16 G26 G36 G46 G56 G66]
```

Note that shear is ordered g_{yz}, g_{zx}, g_{xy} which may not be the convention of other software.

4 : 2-D anisotropic solid

2-D Anisotropic solid, see section 6.1.2, are described by a row of the form

```
[MatId typ E11 E12 E22 E13 E23 E33 rho eta a1 a2 a3 T0]
```

with `typ` an identifier generated with the `fe_mat('m_elastic','SI',4)` command, *rho* (density), *eta* (loss factor) and *Eij* elastic constants and *ai* anisotropic thermal expansion coefficients.

5 : shell orthotropic material

shell orthotropic material, see section 6.1.4 corresponding to NASTRAN MAT8, are described by a row of the form

```
[MatId typ E1 E2 nu12 G12 G13 G23 Rho A1 A2 T0 Xt Xc Yt Yc S Eta ...
F12 STRN]
```

with `typ` an identifier generated with the `fe_mat('m_elastic','SI',5)` command, *rho* (density), ... See `m_elastic Dbvallamina` for building.

6 : Orthotropic material

3-D orthotropic material, see section 6.1.1, are described by a set of engineering constants, in a row of the form

```
[MatId typ E1 E2 E3 Nu23 Nu31 Nu12 G23 G31 G12 rho a1 a2 a3 T0 eta]
```

with `typ` an identifier generated with the `fe_mat('m_elastic','SI',6)` command, *Ei* (Young modulus in each direction), *νij* (Poisson ratio), *Gij* (shear modulus), *rho* (density), *ai* (anisotropic thermal expansion coefficient), *T₀* (reference temperature), and *eta* (loss factor). Care must be taken when using these conventions, in particular, it must be noticed that

$$\nu_{ji} = \frac{E_j}{E_i} \nu_{ij} \quad (8.9)$$

See also Section 4.2.1, section 7.3, `fe_mat`, `p_shell`

m_heat

Purpose Material function for heat problem elements.

Syntax

```
mat= m_heat('default')
mat= m_heat('database name')
pl = m_heat('dbval MatId name');
pl = m_heat('dbval -unit TM MatId name');
pl = m_heat('dbval -punit TM MatId name');
```

Description This help starts by describing the main commands of `m_heat` : `Database` and `Dbval`. Materials formats supported by `m_heat` are then described.

`[Database,Dbval] [-unit TY] [,MatID]] Name`

A material property function is expected to store a number of standard materials. See section 7.3 for material property interface.

`m_heat('DataBase Steel')` returns a the data structure describing steel.

`m_heat('DBVal 100 Steel')` only returns the property row.

```
% List of materials in data base
m_heat info
% examples of row building and conversion
pl=m_heat('DBVal 5 steel');
pl=m_heat(pl,...
    'dbval 101 aluminum', ...
    'dbval 200 steel');
pl=fe_mat('convert SITM',pl);
pl=m_heat(pl,'dbval -unit TM 102 steel')
```

Subtypes `m_heat` supports the following material subtype

1 : Heat equation material
[MatId fe_mat('m_heat','SI',2) k rho C Hf]

`k` conductivity

`rho` mass density

m_heat

[C](#) heat capacity

[Hf](#) heat exchange coefficient

See also Section 4.2.1, section 7.3, [fe_mat](#), [p_heat](#)

m_hyper

Purpose Material function for hyperelastic solids.

Syntax

```
mat= m_hyper('default')
mat= m_hyper('database name')
pl = m_hyper('dbval MatId name');
pl = m_hyper('dbval -unit TM MatId name');
pl = m_hyper('dbval -punit TM MatId name');
```

Description Function based on `m_elastic` function adapted for hyperelastic material. Only subtype 1 is currently used:

1 : Nominal hyperelastic material

Nominal hyperelastic materials are described by a row of the form

```
[MatID typ rho Wtype C_1 C_2 K]
```

with `typ` an identifier generated with the `fe_mat('m_hyper','SI',1)` command, `rho` (density), `Wtype` (value for Energy choice), `C1`, `C2`, `K` (energy coefficients). Possible values for `Wtype` are:

$$0 : W = C_1(J_1 - 3) + C_2(J_2 - 3) + K(J_3 - 1)^2$$

$$1 : W = C_1(J_1 - 3) + C_2(J_2 - 3) + K(J_3 - 1) - (C_1 + 2C_2 + K) \ln(J_3)$$

Other energy functions can be added by editing the `hyper.c` `Enpassiv` function.

In RivlinCube test, `m_hyper` is called in this form:

```
model.pl=m_hyper('dbval 100 Ref'); % this is where the material is defined
```

the hyperelastic material called “Ref” is described in the database of `m_hyper.m` file:

```
out.pl=[MatId fe_mat('type','m_hyper','SI',1) 1e-06 0 .3 .2 .3];
out.name='Ref';
out.type='m_hyper';
out.unit='SI';
```

Here is an example to set your material property for a given structure model:

```
model.pl = [MatID fe_mat('m_hyper','SI',1) typ rho Wtype C_1 C_2 K];
model.Elt(2:end,length(feval(ElemF,'node')+1)) = MatID;
```

m_piezo, p_piezo

Purpose Material function for piezoelectric solids

Syntax

```
mat= m_piezo('database name')
pl = m_piezo('dbval MatId name');
```

Sample calls are given at the end of this page and in section 6.6.

Accepted electrode commands are

ElectrodeMPC

`[model,InputDOF(end+1,1)]=p_piezo('ElectrodeMPC Name',model,'z==5e-5');` defines the isopotential constraint as a case entry `Name` associated with `FindNode` command `z==5e-5`. An illustration is given in the `MPC_actuator demo`.

ElectrodeView

`p_piezo('electrodeview',cf)` outlines the electrodes in the model and prints a clear text summary of electrode information. To only get the summary, pass a model `model` rather than a pointer `cf` to a `feplot` figure.

Material

Theoretical details on piezoelectric materials are given in section 6.1.5. The `m_piezo` `Const` and `BuildConstit` commands support integration constant building for piezo electric volumes integrated in the standard volume elements. Element properties are given by `p_solid` entries, while materials formats are detailed here.

The elastic constitutive law is declared using `ElasMatId` referencing an elastic material declared as a row of `model.pl` with this identifier, see `m_elastic` for input formats. The piezoelectric constants can be declared using the following sub-types

1 : Simplified 3D piezoelectric properties

```
[ProId Type ElasMatId d31 d32 d33 eps1T eps2T eps3T EDType]
```

These simplified piezoelectric properties (6.28) can be used for PVDF, but also for PZT if shear mode actuation/sensing is not considered ($d_{24} = d_{15} = 0$). For `EDType==0` on assumes d is given. For `EDType==1`, e is given. Note that the values of ε^T (permittivity at zero stress) should be given (and not ε^S).

2 : General 3D piezo

```
[ProId Type ElasMatId d_1:18 epsT_1:9]
```

`d_1:18` are the 18 constants of the $[d]$ matrix (see equation (6.29)), and `epsT_1:9` are the 9 constants of the $[\varepsilon^T]$ matrix. One reminds that strains are stored in order xx, yy, zz, yz, zx, yx .

3 : General 3D piezo, e matrix

```
[ProId Type ElasMatId e_1:18 epsT_1:9]
```

`e_1:18` are the 18 constants of the $[d]$ matrix, and `epsT_1:9` are the 9 constants of the $[\varepsilon^T]$ matrix in the constitutive law (6.28).

Shell element properties

Piezo shell elements with electrodes are declared by a combination of a mechanical definition as a layered composite, see [p_shell 2](#), and an electrode definition with element property rows of the form

```
[ProId Type UnderlyingProId ElNodeId1 LayerId1 UNU1 ElNodeId2...]
```

- `Type` typically `fe_mat('p_piezo', 'SI', 1)`
- `UnderlyingProId` : `ProId` of underlying element [p_shell 2](#) composite entry. The `MatIdi` for piezo layers must be associated with piezo electric material properties.
- `ElNodId1` : `NodeId` for electrode 1. This needs to be a node declared in the model but its position is not used since only the value of the electric potential (DOF 21) is used.
- `LayerId` : layer number as declared in the composite entry
- `UNU1` : currently unused property (angle for polarization)

The constitutive law for a piezoelectric shell is given in equation (6.39). The following gives a sample declaration.

```
model=femesh('testquad4'); % Shell MatId 100 ProdId 110
```

```
% MatId 1 : steel, MatId 12 : PZT elastic prop
```

m_piezo, p_piezo

```
model.pl=m_elastic('dbval 1 Steel');
model=feutil('setmat',model,[12 fe_mat('m_elastic','SI',1) 65e9 .3 7800]);
% Sample ULB piezo material, sdtweb m_piezo('sample_ULB')
model.pl=m_piezo(model.pl,'dbval 3 -elas 12 Sample_ULB');

% ProId 111 : 3 layer composite (mechanical properties)
model.il=p_shell(model.il,['dbval 111 laminate ' ...
    '3 1e-3 0 ' ... % MatID 3 (PZT), 1 mm piezo, 0
    '1 2e-3 0 ' ... % MatID 1 (Steel), 2 mm
    '3 1e-3 0']); % MatID 3 (PZT), 1 mm piezo, 0
% ProId 110 : 3 layer piezo shell with electrodes on nodes 1682 and 1683
model.il=p_piezo(model.il,'dbval 110 shell 111 1682 1 0 1683 3 0');

p_piezo('viewdd',model) % Details about the constitutive law
```

p_beam

Purpose Element property function for beams

Syntax

```
il = p_beam('default')
il = p_beam('database','name')
il = p_beam('dbval ProId','name');
il = p_beam('dbval -unit TM ProId name');
il = p_beam('dbval -punit TM ProId name');
```

Description This help starts by describing the main commands : `p_beam Database` and `Dbval`. Supported `p_beam` subtypes and their formats are then described.

[Database,Dbval] ...

`p_beam` contains a number of defaults obtained with `p_beam('database')` or `p_beam('dbval MatId')`. You can select a particular entry of the database with using a name matching the database entries. You can also automatically compute the properties of standard beams

```
circle r                beam with full circular section of radius r.
rectangle b h          beam with full rectangular section of width b and
                        height h. See beam1 for orientation.
Type r1 r2 ...         other predefined sections of subtype 3 are listed using
                        p_beam('info').
```

For example, you will obtain the section property row with `ProId` 100 associated with a circular cross section of $0.05m$ or a rectangular $0.05 \times 0.01m$ cross section using

```
% ProId 100, rectangle 0.05 m by 0.01 m
pro = p_beam('database 100 rectangle .05 .01')
% ProId 101 circle radius .05
il = p_beam(pro.il,'dbval 101 circle .05')
p_beam('info')
% ProId 103 tube external radius .05 internal .04
il = p_beam(il,'dbval -unit SI 103 tube .05 .04')
% Transform to subtype 1
il2=p_beam('ConvertTo1',il)
il(end+1,1:6)=[104 fe_mat('p_beam','SI',1) 0 0 0 1e-5];
```

```
il = fe_mat('convert SITM',il);
% Generate a property in TM, providing data in SI
il = p_beam(il,'dbval -unit TM 105 rectangle .05 .01')
% Generate a property in TM providing data in TM
il = p_beam(il,'dbval -punit TM 105 rectangle 50 10')
```

Beam format description and subtypes

Element properties are described by the row of an element property matrix or a data structure with an `.il` field containing this row (see section 7.4). Element property functions such as `p_beam` support graphical editing of properties and a database of standard properties.

For a tutorial on material/element property handling see section 4.2.1. For a programmers reference on formats used to describe element properties see section 7.4.

1 : standard

```
[ProID  type  J I1 I2 A  k1 k2 Lump NSM]
```

<i>ProID</i>	element property identification number.
<i>type</i>	identifier obtained with <code>fe_mat('p_beam', 'SI', 1)</code> .
<i>J</i>	torsional stiffness parameter (often different from polar moment of inertia <code>I1+I2</code>).
<i>I1</i>	moment of inertia for bending plane 1 defined by a third node <code>nr</code> or the vector <code>vx vy vz</code> (defined in the <code>beam1</code> element). For a case with a beam along x and plane 1 the xy plane <code>I1</code> is equal to $Iz = \int_S y^2 ds$.
<i>I2</i>	moment of inertia for bending plane 2 (containing the beam and orthogonal to plane 1).
<i>A</i>	section area.
<i>k1</i>	(optional) shear factor for motion in plane 1 (when not 0, a Timoshenko beam element is used). The effective area of shear is given by $k_1 A$.
<i>k2</i>	(optional) shear factor for direction 2.
<i>lump</i>	(optional) request for lumped mass model if set to 1.
<i>NSM</i>	(optional) non structural mass (density per unit length).

`bar1` elements only use the section area. All other parameters are ignored.

`beam1` elements use all parameters. Without correction factors (`k1 k2` not given or set to 0), the `beam1` element is the standard Bernoulli-Euler 12 DOF element based on linear interpolations for traction and torsion and cubic interpolations for

flexion (see Ref. [36] for example). When non zero shear factors are given, the bending properties are based on a Timoshenko beam element with selective reduced integration of the shear stiffness [44]. No correction for rotational inertia of sections is used.

3 : Cross section database

This subtype can be used to refer to standard cross sections defined in database. It is particularly used by `nasread` when importing NASTRAN `PBEAML` properties.

```
[ProID  type  0  Section Dim(i) ... NSM]
```

<code>ProID</code>	element property identification number.
<code>type</code>	identifier obtained with <code>fe_mat('p_beam','SI',3)</code> .
<code>Section</code>	identifier of the cross section obtained with <code>comstr('SectionName',-32'</code> where <code>SectionName</code> is a string defining the section (see below).
<code>Dim1 ...</code>	dimensions of the cross section.
<code>NSM</code>	non structural mass can be added as a last parameter.

Cross section, if existing, is compatible with NASTRAN `PBEAML` definition. Equivalent moment of inertia and tensional stiffness are computed at the centroid of the section. Currently available sections are listed with `p_beam('info')`. In particular one has `ROD` (1 dim), `TUBE` (2 dims), `T` (4 dims), `T2` (4 dims), `I` (6 dims), `BAR` (2 dims), `CHAN1` (4 dims), `CHAN2` (4 dims).

See also Section 4.2.1, section 7.4, `fe_mat`

p_heat

Purpose Formulation and material support for the heat equation

Syntax `il = p_heat('default')`

Description This help starts by describing the main commands : `p_heat Database` and `Dbval`. Supported `p_heat` subtypes and their formats are then described.

`[Database,Dbval] ...`

Element properties are described by the row of an element property matrix or a data structure with an `.il` field containing this row (see section 7.4). Element property functions such as `p_solid` support graphical editing of properties and a database of standard properties.

`p_heat` database

```
il=p_heat('database');
```

Accepted commands for the database are

- `d3 Integ SubType` : `Integ` integration rule for 3D volumes (default -3).
- `d2 Integ SubType` : `Integ` integration rule for 2D volumes (default -3).

For fixed values, use `p_heat('info')`.

Example of database property construction

```
il=p_heat([100 fe_mat('p_heat','SI',1) 0 -3 3],...  
          'dbval 101 d3 -3 2');
```

Heat equation element properties

Element properties are described by the row of an element property matrix or a data structure with an `.il` field containing this row (see section 7.4). Element property functions such as `p_beam` support graphical editing of properties and a database of standard properties.

For a tutorial on material/element property handling see section 4.2.1. For a programmers reference on formats used to describe element properties see section 7.4.


```

1 : Volume element for heat diffusion (dimension DIM)
    [ProId fe_mat('p_heat','SI',1) CoordM Integ DIM]

ProID          element property identification number
type           identifier obtained with fe_mat('p_beam','SI',1)
Integ          is rule number in integrules
DIM            is problem dimension 2 or 3 D

2 : Surface element for heat exchange (dimension DIM-1)
    [ProId fe_mat('p_heat','SI',2) CoordM Integ DIM]

ProID          element property identification number
type           identifier obtained with fe_mat('p_beam','SI',2)
Integ          is rule number in integrules
DIM            is problem dimension 2 or 3 D

```

SetFace

This command can be used to define a surface exchange and optionally associated load. Surface exchange elements add a stiffness term to the stiffness matrix related to the exchange coefficient **Hf** defined in corresponding material property. One then should add a load corresponding to the exchange with the source temperature at T_0 through a convection coefficient **Hf** which is **Hf.T_0**. If not defined, the exchange is done with source at temperature equal to 0.

```
model=p_heat('SetFace',model,SelElt,pl,il);
```

- **SelElt** is a findelt command string to find faces that exchange heat (use 'Self-face' to select face of a given preselected element).
- **pl** is the identifiant of existing material property (**MatId**), or a vector defining an **m_heat** property.
- **il** is the identifiant of existing element property (**ProId**), or a vector defining an **p_heat** property.

Command option **-load T** can be used to defined associated load, for exchange with fluid at temperature **T**. Note that if you modify **Hf** in surface exchange material property you have to update the load.

Following example defines a simple cube that exchanges with thermal source at 55 deg on the bottom face.

```
model=femesh('TestHexa8'); % Build simple cube model
model.pl=m_heat('dbval 100 steel'); % define steel heat diffusion parameter
model.il=p_heat('dbval 111 d3 -3 1'); % volume heat diffusion (1)
model=p_heat('SetFace-load55',... % exchange at 55 deg
    model,...
    'SelFace & InNode{z==0}',... % on the bottom face
    100,... % keep same matid for exchange coef
    p_heat('dbval 1111 d3 -3 2')); % define 3d, integ-3, for surface exchange (
cf=feplot(model); fecom colordatapro
def=fe_simul('Static',model); % compute static thermal state
mean(def.def)
```

2D validation

Consider a bi-dimensional annular thick domain Ω with radii $r_e = 1$ and $r_i = 0.5$. The data are specified on the internal circle Γ_i and on the external circle Γ_e . The solid is made of homogeneous isotropic material, and its conductivity tensor thus reduces to a constant k . The steady state temperature distribution is then given by

$$-k\Delta\theta(x,y) = f(x,y) \quad \text{in } \Omega. \quad (8.10)$$

The solid is subject to the following boundary conditions

- $\Gamma_i (r = r_i)$: Neumann condition

$$\frac{\partial\theta}{\partial n}(x,y) = g(x,y) \quad (8.11)$$

- $\Gamma_e (r = r_e)$: Dirichlet condition

$$\theta(x,y) = \theta_{ext}(x,y) \quad (8.12)$$

In above expressions, f is an internal heat source, θ_{ext} an external temperature at $r = r_e$, and g a function. All the variables depend on the variable x and y .

The OpenFEM model for this example can be found in `ofdemos('AnnularHeat')`.

Numerical application : assuming $k = 1$, $f = 0$, $Hf = 1e^{-10}$, $\theta_{ext}(x,y) = \exp(x) \cos(y)$ and $g(x,y) = -\frac{\exp(x)}{r_i} (\cos(y)x - \sin(y)y)$, the solution of the problem is given by

$$\theta(x,y) = \exp(x) \cos(y)$$

See also Section 4.2.1, section 7.4, [fe.mat](#)

p_shell

Purpose Element property function for shells

Syntax

```
il = p_shell('default');
il = p_shell('database ProId name');
il = p_shell('dbval ProId name');
il = p_shell('dbval -unit TM ProId name');
il = p_shell('dbval -punit TM ProId name');
il = p_shell('SetDrill 0',il);
```

Description This help starts by describing the main commands : `p_shell Database` and `Dbval`. Supported `p_shell` subtypes and their formats are then described.

[Database,Dbval] ...

`p_shell` contains a number of defaults obtained with the `database` and `dbval` commands which respectively return a structure or an element property row. You can select a particular entry of the database with using a name matching the database entries.

You can also automatically compute the properties of standard shells with

<code>kirchhoff e</code>	Kirchhoff shell of thickness <code>e</code> .
<code>mindlin e</code>	Mindlin shell of thickness <code>e</code> .
<code>laminate MatIdi Ti Thetai</code>	Specification of a laminate property by giving the different ply <code>MatId</code> , thickness and angle. By default the <code>z</code> values are counted from <code>-thick/2</code> , you can specify another value with a <code>z0</code> .

You can append a string option of the form `-f i` to select the appropriate shell formulation. For example, you will obtain the element property row with `ProId 100` associated with a `.1` thick Kirchhoff shell (with formulation 5) or the corresponding Mindlin plate use

```
il = p_shell('database 100 MindLin .1')
il = p_shell('dbval 100 kirchhoff .1 -f5')
il = p_shell('dbval 100 laminate z0=-2e-3 110 3e-3 30 110 3e-3 -30')
il = fe_mat('convert SITM',il);
il = p_shell(il,'dbval -unit TM 2 MindLin .1') % set in TM, provide data in SI
il = p_shell(il,'dbval -punit TM 2 MindLin 100') % set in TM, provide data in '
```

For laminates, you specify for each ply the `MatId`, thickness and angle.

Shell format description and subtypes

Element properties are described by the row of an element property matrix or a data structure with an `.il` field containing this row (see section 7.4). Element property functions such as `p_shell` support graphical editing of properties and a database of standard properties.

For a tutorial on material/element property handling see section 4.2.1. For a reference on formats used to describe element properties see section 7.4.

`p_shell` currently only supports two subtypes

1 : standard isotropic

```
[ProID type f d 0 h k MID2 RatI12_T3 MID3 NSM Z1 Z2 MID4]
```

`type` identifier obtained with `fe_mat('p_shell','SI',1)`.

`f 0` default, for other formulations the specific help for each element (`quad4`, ...).

`d -1` no drilling stiffness. The element DOFs are the standard translations and rotations at all nodes (DOFs `.01` to `.06`). The drill DOF (rotation `.06` for a plate in the `xy` plane) has no stiffness and is thus eliminated by `fe_mk` if it corresponds to a global DOF direction. The default is `d=1` (`d` is set to 1 for a declared value of zero).

`d` arbitrary drilling stiffness with value proportional to `d` is added. This stiffness is often needed in shell problems but may lead to numerical conditioning problems if the stiffness value is very different from other physical stiffness values. Start with a value of 1. Use `il=p_shell('SetDrill d',il)` to set to `d` the drilling stiffness of all `p_shell` subtype 1 rows of the property matrix `il`.

`h` plate thickness.

`k k` shear correction factor (default 5/6, default used if `k` is zero). This correction is not used for formulations based on triangles since `tria3` is a thin plate element.

`RatI12_T3` Ratio of bending moment of inertia to nominal `T3/I12` (default 1).

`NSM` Non structural mass per unit area.

`MID2` unused.

`MID3` unused.

`z1,z2` (unused) offset for fiber computations.

`MID4` unused.

Shell strain is defined by the membrane, curvature and transverse shear (display with `p_shell('ConstShell')`).

$$\begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ 2\epsilon_{xy} \\ \kappa_{xx} \\ \kappa_{yy} \\ 2\kappa_{xy} \\ \gamma_{xz} \\ \gamma_{yz} \end{Bmatrix} = \begin{bmatrix} N, x & 0 & 0 & 0 & 0 \\ 0 & N, y & 0 & 0 & 0 \\ N, y & N, x & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & N, x \\ 0 & 0 & 0 & -N, y & 0 \\ 0 & 0 & 0 & -N, x & N, y \\ 0 & 0 & N, x & 0 & -N \\ 0 & 0 & N, y & N & 0 \end{bmatrix} \begin{Bmatrix} u \\ v \\ w \\ ru \\ rv \end{Bmatrix} \quad (8.13)$$

2 : composite

[ProID type Z0 NSM SB FT TREF GE LAM MatId1 T1 Theta1 SOUT1 ...]

ProID	Section property identification number.
type	Identifier obtained with <code>fe_mat('p_shell','SI',2)</code> .
Z0	Distance from reference plate to bottom surface.
NSM	Non structural mass per unit area.
SB	Allowable shear stress of the bonding material.
FT	Failure theory.
TREF	Reference temperature.
Eta	Hysteretic loss factor.
LAM	Laminate type.
MatId<i>i</i>	MatId for ply <i>i</i> .
T<i>i</i>	Thickness of ply <i>i</i> .
Theta<i>i</i>	Orientation of ply <i>i</i> .
SOUT<i>i</i>	Stress output request for ply <i>i</i> .

Note that this subtype is based on the format used by NASTRAN for **PCOMP** and is only supported by the **q4cs** SDT element. You can use the **Dbvallaminate** commands to generate standard entries.

$$\begin{Bmatrix} N \\ M \\ Q \end{Bmatrix} = \begin{bmatrix} A & B & 0 \\ B & D & 0 \\ 0 & 0 & F \end{bmatrix} \begin{Bmatrix} \epsilon \\ \kappa \\ \gamma \end{Bmatrix} \quad (8.14)$$

setTheta

When dealing with laminated plates, the classical approach uses a material orientation constant per element. OpenFEM also supports more advanced strategies with orientation defined at nodes but this is still poorly documented.

The material orientation is the reference for plies. Any angle defined in a laminate command is an additional rotation. In the example below, the element orientation is rotated 30 degrees, and the ply another 30. The fibers are thus oriented 60 degrees in the xy plane. Stresses are however given in the material orientation thus with a 30 degree rotation. Per ply output is not currently implemented.

The element-wise material angle is stored for each element. In column 7 for `tria3`, 8 for `quad4`, ... The `setTheta` command is a utility to ease the setting of these angles. By default, the orientation is done at element center. To use the mean orientation at nodes use command option `-strategy 2`.

```
model=ofdemos('composite');
model.il = p_shell('dbval 110 laminate 100 1 30'); % single ply

% Define material angle based on direction at element
MAP=feutil('getnormalElt MAP -dir1',model);
bas=basis('rotate', [], 'rz=30;', 1);
MAP.normal=MAP.normal*reshape(bas(7:15), 3, 3)';
model=p_shell('setTheta', model, MAP);

% Obtain a MAP of material orientations
MAP=feutil('getnormalElt MAP -dir1', model);
feplot(model); fecom('showmap', MAP)

% Set elementwise material angles using directions given at nodes.
% Here a global direction
MAP=struct('normal', ones(size(model.Node, 1), 1)*bas(7:9), ...
          'ID', model.Node(:, 1), 'opt', 2);
model=p_shell('setTheta', model, MAP);

% Using an analytic expression to define components of
% material orientation vector at nodes
data=struct('sel', 'groupall', 'dir', {{'x-0', 'y+.01', 0}}, 'DOF', [.01; .02; .03]);
model=p_shell('setTheta', model, data);
MAP=feutil('getnormalElt MAP -dir1', model);
feplot(model); fecom('showmap', MAP)
```

`model=p_shell('setTheta',model,0)` is used to reset the material orientation to zero.

Technically, shells use the `of_mk('BuildNDN')` rule 23 which generates a basis at each integration point. The first vector `v1x,v1y,v1z` is built in the direction of r lines and `v2x,v2y,v2z` is tangent to the surface and orthogonal to $v1$. When a `InfoAtNode` map provides `v1x,v1y,v1z`, this vector is projected (NEED TO VERIFY) onto the surface and $v2$ taken to be orthogonal.

See also

Section 4.2.1, section 7.4, `fe_mat`

p_solid

Purpose Element property function for volume elements.

Syntax

```
il=p_solid('database ProId Value')
il=p_solid('dbval ProId Value')
il=p_solid('dbval -unit TM ProId name');
il=p_solid('dbval -punit TM ProId name');
model=p_solid('default',model)
```

Description This help starts by describing the main commands : `p_solid Database` and `Dbval`. Supported `p_solid` subtypes and their formats are then described.

`[Database,Dbval,Default] ...`

Element properties are described by the row of an element property matrix or a data structure with an `.il` field containing this row (see section 7.4). Element property functions such as `p_solid` support graphical editing of properties and a database of standard properties.

Accepted commands for the database are

- `d3 Integ` : *Integ* integration rule for quadratic 3D volumes. For information on rules available see `integrules Gauss`. Examples are `d3 2` 2x2x2 integration rule for linear volumes (hexa8 ...); `d3 -3` default integration for all 3D elements, ...
- `d2 Integ` : *Integ* integration rule for quadratic 2D volumes. For example `d2 2` 2x2x2 integration rule for linear volumes (q4p ...). You can also use `d2 1 0 2` for plane stress, and `d2 2 0 2` for axisymetry.
- `fsc Integ` : integration rule selection for fluid/structure coupling.

For fixed values, use `p_solid('info')`.

For a tutorial on material/element property handling see section 4.2.1. For a reference on formats used to describe element properties see section 7.4.

Examples of database property construction

```
il=p_solid([100 fe_mat('p_solid','SI',1) 0 3 0 2], ...
```

```

        'dbval 101 Full 2x2x2', 'dbval 102 d3 -3');
il=fe_mat('convert SITM',il);
il=p_solid(il,'dbval -unit TM 2 Reduced shear')
% Try a smart guess on default
model=femesh('TestHexa8');model.il=[];
model=p_solid('default',model)

```

Subtype 1 : 3D volume element

```
[ProID fe_mat('p_solid','SI',1) Coordm In Stress Isop ]
```

ProID	Property identification number.
Coordm	Identification number of the material coordinates system (not used yet).
In	Integration rule selection (see integrules Gauss). 0 selects the legacy 3D mechanics element (of_mk_pre.c), -3 the default rule.
Stress	Location selection for stress output (NOT USED).
Isop	Integration scheme (will be used to select shear protection mechanisms).

The underlying physics for this subtype are selected through the material property. Examples are 3D mechanics with [m_elastic](#), piezo electric volumes (see [m_piezo](#)), heat equation ([p_heat](#)).

Subtype 2 : 2D volume element

```
[ProId fe_mat('p_solid','SI',2) Form N In]
```

ProID	Property identification number.
Type	Identifier obtained with <code>fe_mat('p_solid','SI',2)</code> .
Form	Formulation (0 plane strain, 1 plane stress, 2 axisymmetric), see details in m_elastic .
N	Fourier harmonic for axisymmetric elements that support it.
In	Integration rule selection (see integrules Gauss). 0 selects legacy 2D element, -3 the default rule.

The underlying physics for this subtype are selected through the material property. Examples are 2D mechanics with [m_elastic](#).

Subtype 3 : ND-1 coupling element

```
[ProId fe_mat('p_solid','SI',3) Integ Form Ndof1 ...]
```

ProID	Property identification number.
Type	Identifier obtained with <code>fe_mat('p_solid','SI',3)</code> .
Integ	Integration rule selection (see <code>integrules Gauss</code>). 0 or -3 selects the default for the element.
Form	1 volume force, 2 volume force proportionnal to density, 3 pressure, 4: fluid/structure coupling, see <code>fsc</code> , 5 2D volume force, 6 2D pressure.

See also Section 4.2.1, section 7.4, `fe_mat`

p_spring

Purpose Element property function for spring and rigid elements

Syntax

```
il=p_spring('default')
il=p_spring('database MatId Value')
il=p_spring('dbval MatId Value')
il=p_spring('dbval -unit TM ProId name');
il=p_spring('dbval -punit TM ProId name');
```

Description This help starts by describing the main commands : `p_spring Database` and `Dbval`. Supported `p_spring` subtypes and their formats are then described.

`[Database,Dbval] ...`

Element properties are described by the row of an element property matrix or a data structure with an `.il` field containing this row (see section 7.4).

Examples of database property construction

```
il=p_spring('database 100 1e12 1e4 0')
il=p_spring('dbval 100 1e12');
il=fe_mat('convert SITM',il);
il=p_spring(il,'dbval 2 -unit TM 1e12') % Generate in TM, provide data in SI
il=p_spring(il,'dbval 2 -punit TM 1e9') % Generate in TM, provide data in TM
```

`p_spring` currently supports 2 subtypes

1 : `standard`

`[ProID type k m c Eta S]`

<code>ProID</code>	property identification number.
<code>type</code>	identifier obtained with <code>fe_mat('p_spring','SI',1)</code> .
<code>k</code>	stiffness value.
<code>m</code>	mass value.
<code>c</code>	viscous damping value.
<code>eta</code>	loss factor.
<code>S</code>	Stress coefficient.

2 : bush

Note that type 2 is only functional with `cbush` elements.

```
[ProId Type k1:k6 c1:c6 Eta SA ST EA ET m v]
```

<code>ProID</code>	property identification number.
<code>type</code>	identifier obtained with <code>fe_mat('p_spring','SI',2)</code> .
<code>ki</code>	stiffness for each direction.
<code>ci</code>	viscous damping for each direction.
<code>SA</code>	stress recovery coef for translations.
<code>ST</code>	stress recovery coef for rotations.
<code>EA</code>	strain recovery coef for translations.
<code>ET</code>	strain recovery coef for rotations.
<code>m</code>	mass.
<code>v</code>	volume.

See also

Section 4.2.1, section 7.4, `fe_mat`, `celas`, `cbush`

p_super

Purpose Element property function for superelements.

Syntax

```
il=p_super('default')
il=p_super('database MatId Value')
il=p_super('dbval MatId Value')
il=p_super('dbval -unit TM ProId name');
il=p_super('dbval -punit TM ProId name');
```

Description If **ProID** is not given, **fe_super** will see if **SE.Opt(3,:)** is defined and use coefficients stored in this row instead. If this is still not given, all coefficients are set to 1. **Element property rows** (in a standard property declaration matrix **il**) for superelements take the forms described below with **ProID** the property identification number and coefficients allowing the creation of a weighted sum of the superelement matrices **SEName.K{i}**. Thus, if **K{1}** and **K{3}** are two stiffness matrices and no other stiffness matrix is given, the superelement stiffness is given by **coef1*K{1}+coef3*K{3}**.

[Database,Dbval] ...

There is no database call for **p_super** entries.

Subtype 1 : simple weighting coefficients

[ProId Type coef1 coef2 coef3 ...]

ProID Property identification number.

Type Identifier obtained with **fe_mat('p_super','SI',1)**.

coef1 Multiplicative coefficient of the first matrix of the superelement (**K{1}**). Superelement matrices used for the assembly of the global model matrices will be **{coef1*K{1}, coef2*K{2}, coef3*K{3}, ...}**. Type of the matrices (stiffness, mass ...) is not changed. Note that you can define parameters for superelement using **fe_case(model,'par')**, see **fe_case**.

Subtype 2 : matrix type redefinition and weighting coefficients

[ProId Type Form type1 coef1 type2 coef2 ...]

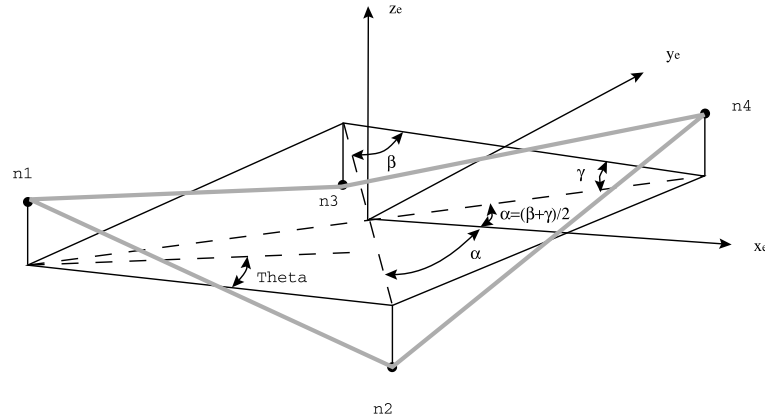
<code>ProID</code>	Property identification number.
<code>Type</code>	Identifier obtained with <code>fe_mat('p_super','SI',2)</code> .
<code>type1</code>	Type redefinition of the first matrix of the superelement ($K\{1\}$) according to SDT standard type (1 for stiffness, 2 for mass, 3 for viscous damping... see <code>MatType</code>).
<code>coef1</code>	Multiplicative coefficient of the first matrix of the superelement ($K\{1\}$). Superelement matrices used for the assembly of the global model matrices will be <code>{coef1*K{1}, coef2*K{2}, coef3*K{3}, ...}</code> . Type of the matrices (stiffness, mass ...) is changed according to <code>type1, type2, ...</code> . Note that you can define parameters for superelement using <code>fe_case(model,'par')</code> , see <code>fe_case</code> .

See also [fesuper](#), section 6.3

quad4, quadb, mitc4

Purpose 4 and 8 node quadrilateral plate/shell elements.

Description



In a model description matrix, **element property rows** for [quad4](#), [quadb](#) and [mitc4](#) elements follow the standard format

```
[n1 ... ni MatID ProID EltID Theta Zoff T1 ... Ti]
```

giving the node identification numbers **ni** (1 to 4 or 8), material **MatID**, property **ProID**. Other **optional** information is **EltID** the element identifier, **Theta** the angle between material *x* axis and element *x* axis, **Zoff** the off-set along the element *z* axis from the surface of the nodes to the reference plane (use [feutil Orient](#) command to check *z*-axis orientation), **Ti** the thickness at nodes (used instead of **il** entry, currently the mean of the **Ti** is used).

If **n3** and **n4** are equal, the [tria3](#) element is automatically used in place of the [quad4](#).

Isotropic materials are currently the only supported (this may change soon). Their declaration follows the format described in [m_elastic](#). Element property declarations follow the format described [p_shell](#).

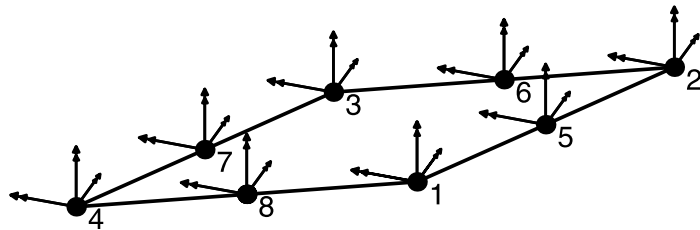
quad4

Supported formulations (**il(3)** see [p_shell](#)) are

- **1** 4 tria3 thin plate elements with condensation of central node.
- **2** Q4WT for membrane and Q4gamma for bending. This is only applicable if the four nodes are in a single plane. When not, formulation **1** is called.
- **4** MITC4 calls the MITC4 element below. This implementation has not been tested extensively, so that the element may not be used in all configurations. It uses 5 DOFs per node with the two rotations being around orthogonal in-plane directions. This is not consistent for mixed element types assembly. Non smooth surfaces are not handled properly because this is not implemented in the `feutil GetNormal` command which is called for each group of `mitc4` elements.
- **5** Q4CS is a second implementation MITC4 elements that supports classical laminated plate theory (composites) as well as the definition of piezo-electric extension actuators.

The definition of local coordinate systems for composite fiber orientation still needs better documentation. Currently, `q4cs` the only element that supports composites, uses the local coordinate system resulting from the `BuildNDN 23` rule. A temporary solution for uniform orientation is provided with `model=feutilb('shellmap -orient dx dy dz',model)`.

quadb



Supported formulations (`il(3)` see `p_shell`) are

- **1** 8 tria3 thin plate elements with condensation of central node.
- **2** isoparametric thick plate with reduced integration. For non-flat elements, formulation **1** is used.

See also [m_elastic](#), [p_shell](#), [fe_mk](#), [feplot](#)

q4p, q8p, t3p, t6p and other 2D volumes

Purpose 2-D volume elements.

Description The [q4p](#), [q5p](#), [q8p](#), [q9a](#), [t3p](#), [t6p](#) elements are topology references for 2D volumes and 3D surfaces.

In a model description matrix, **element property rows** for this elements follow the standard format

```
[n1 ... ni MatID ProID EltID Theta]
```

giving the node identification numbers [n1](#), ..., [ni](#), material [MatID](#), property [ProID](#). Other **optional** information is [EltID](#) the element identifier, [Theta](#) the angle between material x axis and element x axis (material orientation maps are generally preferable).

These elements only define topologies, the nature of the problem to be solved should be specified using a property entry, see section 6.1 for supported problems and [p_solid](#), [p_heat](#), ... for formats.

Integration rules for various topologies are described under [integrules](#). Vertex coordinates of the reference element can be found using an [integrules](#) command containing the name of the element such as `r1=integrules('q4p');`[r1.xi](#).

Backward compatibility note : if no element property entry is defined, or with a [p_solid](#) entry with the integration rule set to zero, the element defaults to the historical 3D mechanic elements described in section 7.18.2.

These volume elements are used for various problem families.

See also [fe_mat](#), [fe_mk](#), [feplot](#)

rigid

Purpose Linearized rigid link constraints.

Description Rigid links are often used to model stiff connections in finite element models. One generates a set of linear constraints that relate the 6 DOFs of master M and slave S nodes by

$$\begin{Bmatrix} u \\ v \\ w \\ r_x \\ r_y \\ r_z \end{Bmatrix}_S = \begin{bmatrix} 1 & 0 & 0 & 0 & z_{MS} & -y_{MS} \\ 0 & 1 & 0 & -z_{MS} & 0 & x_{MS} \\ 0 & 0 & 1 & y_{MS} & -x_{MS} & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} u \\ v \\ w \\ r_x \\ r_y \\ r_z \end{Bmatrix}_M$$

Resolution of linear constraints is performed using `fe_case` or model assembly (see section 4.5.8) calls. The theory is discussed in section 7.14. Note that the master node of a rigid link as 6 DOF, even if the model may only need less (3 for volumes).

If coordinate systems are defined in field `model.bas` (see `basis`), `PID` (position coordinate system) and `DID` (displacement coordinate system) declarations in columns 2 and 3 of `model.Node` are properly handled.

You can use penalized rigid links (`celas` element) instead of truly rigid connections. This requires the selection of a stiffness constant but can be easier to manipulate. To change a group of `rigid` elements into `celas` elements change the element group name `femesh('SetGroup rigid name celas')` and set the stiffness constant `FEelt(femesh('FindEltGroupi'),7) = Kv`.

Although they are linear constraints rather than true elements, such connections can be declared using an element group of rigid connection with a header row of the form `[Inf abs('rigid')]` followed by as many element rows as connections of the form

```
[ n1 n2 DofSel MatId ProId EltId]
```

where node `n2` will be rigidly connected to node `n1` which will remain free. `DofSel` lets you specify which of the 3 translations and 3 rotations are connected (thus `123` connects only translations while `123456` connects both translations and rotations).

The other strategy is to store them as a `case` entry. `rigid` entries are rows of the `Case.Stack` cell array giving `{'rigid', Name, Elt}`. `Name` is a string identifying

the entry. `Elt` is a model description matrix containing `rigid` elements. The elements can also be declared as standard elements as in the following example which generates the mesh of a square plate with a rigid edge

```
femesh('reset');
femesh(';testquad4 divide 10 10;addsel');

% Define a rigid edge
femesh('selelt seledge & innode{x==0}')
femesh('setgroupa1 name rigid')
FEe10(femesh('findel0 group1'),3)=123456; % all 6 DOF are slave
FEe10(femesh('findel0 group1'),4)=0;      % not a DOF equality

femesh('addsel');model=femesh % add to element matrix
% alternative define as a case entry
%model=fe_case(model,'rigid','Rigid edge',FEe10);

% Compute and display modes
def=fe_eig(model,[6 20 1e3]);
feplot(model,def);fecom(';view3;ch8;scd.1');
```

An additional call is `rigidAppend` in order to simply add new rigid links. You may use a list of the form `[MasterNode slaveDOF slaveNode_1 slaveNode_2 ... slaveNode_i]` or of the form of an element matrix (containing a header).

The preceding call would be

```
model=fe_case(femesh,'rigidAppend','Rigid edge',FEe10);
```

or

```
model=fe_case(femesh,'rigidAppend','Rigid edge',...
             [FEe10(2,2) .01 FEe10(2,2) FEe10(3,2)]);
```

The `rigid` function is only used for low level access by generating the subspace `T` that verifies rigid constraints

```
[T,cdof] = rigid(node,elt,mdof)
[T,cdof] = rigid(Up)
```

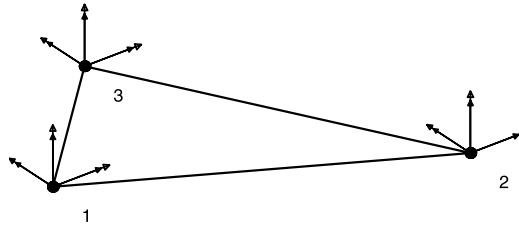
See also

Section 7.14, `celas`

tria3, tria6

Purpose Element functions for a 3 node/18 DOF and 6 nodes/36 DOF shell elements.

Description



In a model description matrix, **element property rows** for [tria3](#) elements follow the standard format

```
[n1 n2 n3 MatID ProID EltID Theta Zoff T1 T2 T3]
```

giving the node identification numbers **ni**, material **MatID**, property **ProID**. Other **optional** information is **EltID** the element identifier, **Theta** the angle between material x axis and element x axis (currently unused), **Zoff** the off-set along the element z axis from the surface of the nodes to the reference plane, **Ti** the thickness at nodes (used instead of **il** entry, currently the mean of the **Ti** is used).

The element only supports isotropic materials with the format described in [m_elastic](#).

The supported property declaration format is described in [p_shell](#). Note that [tria3](#) only supports thin plate formulations.

[tria3](#) uses a T3 triangle for membrane properties and a DKT for flexion (see [45] for example).

[tria6](#) can only be used with [p_shell](#) formulation 5.

See also [quad4](#), [quadb](#), [fe_mat](#), [p_shell](#), [m_elastic](#), [fe_mk](#), [feplot](#)

tria3, tria6

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This section contains detailed descriptions of the functions in *Structural Dynamics Toolbox*. It begins with a list of functions grouped by subject area and continues with the reference entries in alphabetical order. From MATLAB short text information is available through the [help](#) command while the HTML version of this manual can be accessed through [doc](#).

For easier use, most functions have several optional arguments. In a reference entry under syntax, the function is first listed with all the necessary input arguments and then with all *possible* input arguments. Most functions can be used with any number of arguments between these extremes, the rule being that missing, trailing arguments are given default values, as defined in the manual.

As always in MATLAB, all output arguments of functions do not have to be specified, and are then not returned to the user.

As indicated in their synopsis some functions allow different types of output arguments. The different output formats are then distinguished by the number of output arguments, so that all outputs must be asked by the user.

Typesetting conventions and mathematical notations used in this manual are described in section 1.3.

Element functions are detailed in chapter 8.

A list of demonstrations is given in section 1.1.

USER INTERFACE (UI) AND GRAPHICAL USER INTERFACE (GUI) TOOLS	
<code>fecom</code>	UI command function for deformations created with <code>feplot</code>
<code>femesh</code>	UI command function for mesh building and modification
<code>feplot</code>	GUI for 3-D deformation plots
<code>fesuper</code>	UI commands for superelement manipulations
<code>idcom</code>	UI commands for standard identification procedures
<code>idopt</code>	manipulation of identification options
<code>iicom</code>	UI commands for measurement data visualization
<code>ii_mac</code>	GUI for MAC and other vector correlation criteria
<code>iiplot</code>	GUI for the visualization of frequency response data

EXPERIMENTAL MODEL IDENTIFICATION	
<code>idcom</code>	UI commands linked to identification
<code>idopt</code>	manipulation of options for identification related functions
<code>id_rc</code>	broadband pole/residue model identification
<code>id_rcopt</code>	alternate optimization algorithm for <code>id_rc</code>
<code>id_rm</code>	minimal and reciprocal MIMO model creation
<code>id_nor</code>	optimal normal mode model identification
<code>id_poly</code>	weighted least square orthogonal polynomial identification
<code>id_dspi</code>	direct system parameter identification algorithm
<code>ii_poest</code>	narrow-band single pole model identification
<code>ii_pof</code>	transformations between pole representation formats
<code>psi2nor</code>	optimal complex/normal mode model transformation
<code>res2nor</code>	simplified complex to normal mode residue transformation

UI AND GUI UTILITIES	
<code>comgui</code>	general purpose functions for the graphical user interfaces
<code>commode</code>	general purpose parser for UI command functions
<code>comstr</code>	general purpose string handling routine
<code>iimouse</code>	mouse related callbacks (zooming, info, ...)
<code>feutil</code>	mesh handling utilities
<code>ii_plp</code>	overplot vertical lines to indicate pole frequencies
<code>setlines</code>	line style and color sequencing utility

FREQUENCY RESPONSE ANALYSIS TOOLS	
<code>db</code>	amplitude in dB (decibels)
<code>ii_cost</code>	FRF comparison with quadratic and logLS cost
<code>ii_mmif</code>	Multivariate Mode Indicator Function
<code>phaseb</code>	phase (in degrees) with an effort to unwrap along columns
<code>rms</code>	Root Mean Square response

TEST/ANALYSIS CORRELATION TOOLS	
fe_exp	experimental shape expansion
fe_sens	sensor configuration declaration and sensor placement tools
ii_comac	obsolete (supported by ii_mac)
ii_mac	GUI for MAC and other vector correlation criteria
FINITE ELEMENT ANALYSIS TOOLS	
fe2ss	methods to build ss models from full order FEM
fe_c	DOF selection and I/O matrix creation
fe_case	Cases (loads, boundary conditions, etc.) handling
fe_ceig	computation and normalization of complex modes
fe_coor	transformation matrices for Component Mode Synthesis
fe_eig	partial and full eigenvalue computations
fe_load	assembly of distributed load vectors
fe_mat	material property handling utilities
fe_mk	assembly of full and reduced FE models
fe_norm	orthonormalization and collinearity check
fe_reduc	utilities for finite element model reduction
fe_stress	element energies and stress computations
fe_super	generic element function for superelement support
rigid	projection matrix for linearized rigid body constraints
MODEL FORMAT CONVERSION	
nor2res	normal mode model to complex mode residue model
nor2ss	assemble state-space model linked to normal mode model
nor2xf	compute FRF associated to a normal mode model
qbode	fast computation of FRF of a state-space model
res2ss	pole/residue to state space model
res2tf	pole/residue to/from polynomial model
res2xf	compute FRF associated to pole/residue model
ss2res	state-space to pole/residue model
FINITE ELEMENT UPDATE TOOLS	
upcom	user interface for finite element update problems
up_freq	semi-direct update by comparison modal frequencies
up_ifreq	iterative update by comparison of modal frequencies
up_ixf	iterative update based on FRF comparison
up_min	minimization algorithm for FE update algorithms

INTERFACES WITH OTHER SOFTWARE	
<code>ans2sdt</code>	reading of ANSYS binary files (FEMLink)
<code>nasread</code>	read from MSC/NASTRAN <code>.dat</code> , <code>.f06</code> , <code>.o2</code> , <code>.o4</code> files (some with FEMLink)
<code>naswrite</code>	write data to MSC/NASTRAN bulk data deck (some with FEMLink)
<code>nas2up</code>	extended reading of NASTRAN files
<code>ufread</code>	read Universal File Format (some with FEMLink)
<code>ufwrite</code>	write Universal File Format (some with FEMLink)

OTHER UTILITIES	
<code>basis</code>	coordinate transformation utilities
<code>ffindstr</code>	find string in a file
<code>order</code>	sorts eigenvalues and eigenvectors accordingly
<code>remi</code>	integer <code>rem</code> function (<code>remi(6,6)=6</code> and not 0)
<code>setlines</code>	line type and color sequencing
<code>sdth</code>	<i>SDT</i> handle objects
<code>ofact</code>	creation and operators on <code>ofact</code> matrix objects

abaqus

Purpose Interface between ABAQUS and SDT (part of FEMLink) **Warning this function requires MATLAB 7.1 or later.**

Syntax

```
abaqus('read FileName');  
abaqus('job');
```

```
read [*.fil, *.inp, *.mtx]
```

By itself the `read` command imports the model from a `.inp` ASCII input or `.fil` binary output file. Models created by an `*Assembly` command using several instances and/or additional nodes or elements are treated with superelements. Each part instance (called by `*Instance...*end instance`) becomes then a specific superelement in the SDT model. A packaged call allows to get a full model back

```
model=abaqus('read Job-1.inp');  
model=abaqus('ResolveModel',model);
```

The `ResolveModel` command has a limited robustness in the general case due to the difficulty to handle heterogeneous Stack data while renumbering parts of a model. Most cases should be properly handled.

When reading deformations, `getpref('SDT', 'OutOfCoreBufferSize')` is used to determine whether the vectors are left in the file or not. When left, `def.def` is a `v_handle` object that lets you access deformations with standard indexing commands. Use `def.def=def.def(:, :)` to load all. If a modal basis is read, it is stored in the model stack, as `curve, Mode`. If static steps are present all associated deformation are concatenated in order of occurrence in the model stack as `curve, step(1)`.

Command option `-wd` allows to save the model generated in a directory different from the one in which the abaqus files are saved.

You can request the output of element matrices which will then be read into an `upcom` model. To do so, you need to define an element set. To read matrices, you have to provide some information before running the job in order to select which matrices you want to write and read. In the `.inp` input file you may enter the following line

```
*ELSET, ELSET=ALL ELT FOR SDT  
THIN SHELL1 , THIN SHELL1_1
```

(second line contains all the ABAQUS defined sets) just before the `*STEP` line and

```
*ELEMENT MATRIX OUTPUT, ELSET=ALL ELT FOR SDT, STIFFNESS=YES
*ELEMENT MATRIX OUTPUT, ELSET=ALL ELT FOR SDT, MASS=YES
```

just after the `*STEP` line.

Note that this information are automatically generated using the following command

```
abaqus('elementmatrices model.inp'); .
```

Running the Abaqus job generates outputs specified by the user, with `*OUTPUT` commands in the Abaqus job input file. Current default use generates an `odb` file, using commands of the type `*NODE OUTPUT`. The `odb` format however requires the use of Abaqus libraries to be read.

Imports are thus handled in SDT using the `.fil` output binary file. This file is readable without Abaqus, and its reading has been optimized in `FEMLink`. This type of output is generated using commands of the type `*NODE FILE`. A sample command to obtain nodal deformation at the end of a step is then

```
** general command to .fil and ask for nodal deformation field
*OUTPUT, FIELD
*NODE FILE
U
```

All nodal variable keywords should be expressed on separated lines. This must be repeated in all steps of interest in an ABAQUS computation file input `.inp`.

Most common and general nodal variables keywords of interest are the following (this is not applicable to all ABAQUS procedures)

- `U`, `V`, `A` respectively for nodal displacement, velocity and acceleration output
- `RF`, `CF`, `VF`, `TF` respectively for nodal reaction forces, constrained forces, viscous forces, and total forces output
- `GU`, `GV`, `GA` respectively for generalized displacement, velocity and acceleration (when reduction is involved)

Since not all information (materials, set names, ...) can be found in the `.fil`, you may want to combine two reads into an `upcom` model

```
abaqus('read file.inp', 'buildup file.fil');
```

Abaqus features a matrix sparse output starting from version 6.7-1. Their generation is performed in a dedicated step as follows

```
*STEP
*MATRIX GENERATE, STIFFNESS, MASS
*END STEP
```

The output is one ASCII file `.mtx` by matrix requested, which can be read by `abaqus`.

write

`abaqus('write Name.inp',model)`; writes and ABAQUS input file.

`abaqus('BwMTX',model)`; writes all matrices stored in `model.K` in the abaqus sparse output format. Each matrix file is named after the `model.file` entry and `model.Klab`. For a model stored in `model.mat` containing a matrix 'k', the file output will be named `model_k.mat`.

`BwMat` ; `BwMp` ; `BwSet` ; `Bwbas` ; `BwStepEig` are implemented.

JobOpt

`JobOpt = abaqus('JobOpt',Opt)`; This command returns a filled `JobOpt` structure to be run by `sdtjob`. `Opt` is a structure containing at least the field `Job` as the job name or file. `InList` and `OutList` must be filled. Further options concern the fields `Input` when the input file is different from the job name, `RunOptions` to append the usual option to the Abaqus command, `RemoveFile` to remove files from the remote directory when needed.

conv

This command lists conversion tables for elements, topologies, face topologies. You can redefine (enhance) these tables by setting preferences of the form `setpref('FEMLink','abaqus.list',value)`, but please also request enhancements so that the quality of our translators is improved.

splitcelas

`model=abaqus('SplitCelas',model)` splits all `SDT celas` elements to one dimension `celas` elements that can be handled by Abaqus. This command can change the `EltId` so it must be used when meshing the model.

uniquematpro

Merges duplicated `pl/il` instances.

Resolve

This set of commands transforms a raw model import by `abaqus read` into an exploitable SDT model. This is useful when the ABAQUS model has been generated with `*PART` and `*INSTANCE`. In such case, the representation of an ABAQUS model becomes very far from an SDT model. The raw reading obtained by `read` will thus interpret parts as superelements, and leave the instance data, and some internal information not translated.

Some adaptations, performed by `ResolveModel` are thus needed. In particular, renumerations can occur, however all sets definitions are maintained.

- `ResolveModel`

This command will create the elements conforming to the instance information. Commands `ResolveSet` and `ResolveMass` will also be called, to generate a fully exploitable SDT model.

- `ResolveSet`

This command transforms each ABAQUS implicitly defined sets into explicit SDT sets. This is very useful if some sets have been defined in ABAQUS using internal part numerations. Called by `ResolveModel`.

- `ResolveCase`

This command aims at resolving all implicitly defined case entries in the model. Only implicit `MPC` resolution has been implemented at the moment (SDT version of `*TIE` constraints). This also handles the multiple slave resolution in the manner of ABAQUS, and should thus be performed before assembling models if multiple slave error occur.

- `ResolveMass`

This command handles the model stack entry `info,UnResolvedMasses` that may have been created during the `read` call, and assigns mass values missing in mass elements. This is necessary when masses have been defined in an ABAQUS part, such that the attribution of the mass amplitude by `*MASS` is not directly retrievable. Called by `ResolveModel`.

AssembleUserElements

Returns a matrix and its corresponding DOF, from the assembly of all USER ELEMENT instances in an ABAQUS model.

```
[K,dof] = abaqus('AssembleUserElements',model);
```

odb2sdt

Utility functions to transfer Abaqus `.odb` file data into a format similar to MATLAB 6 binary `.mat` file and readable by `sdt hdf`. The changes in the format are introduced to support datasets larger than 2GB.

Abaqus outputs are commonly written in `.odb` files, using a non documented format. The only way to access its data is to use Abaqus CAE or Abaqus Python. These utility functions are to be used with Abaqus Python to extract data from the output database for further use outside Abaqus. The modules used are

- `odbAccess`. Abaqus access libraries.
- `abaqusConstants`. Common output values dictionary, such as `'U'`, `'UR'`
- `Numeric`. Module for array handling utilities.
- `struct`. Module to pack data into binary strings.

For the moment, only nodal data transfer is completely implemented. More information can be found on Python at <http://www.python.org>. Note that `def` is a reserved word in Python for the function definition command; remember not to use it in another way!

The following script is a quick example of what can be done with these functions. It can be launched directly if written in a `.py` file `readODB.py` for example, by `abaqus python readODB.py`

```
from odb2sdt import * # import read functions

jobName='my_abaqus_job'
odb=openOdb(jobName + '.odb')
allNodal2mat(odb)
```

This second script will only write the DOF set in a `.mat` binary file

```
from odb2sdt import * # import read functions

jobName='my_abaqus_job'

odb=openOdb(jobName + '.odb') #open the database
stepName=odb.steps.keys()[0] #get the name of the first step
fieldItem=['U'] #I want the 'U' displacement field
```

```

# get the fieldOutputs instances list from the first frame:
fieldOutputs=odb.steps.__getitem__(stepName).getFrame(0).fieldOutputs

f=matFile(jobName + '_dof.mat')           # Initialize the file
dof2mat(f,fieldOutputs,fieldItem,stepName) # write the DOF array to it
f.close()

```

Once a `file_allNodal.mat` file has been generated, it is possible to load the deformation structure fields using

```
def=abaqus('read file_allNodal.mat')
```

`def` output is here a cell array containing all `def` structures found in the `allNodal.mat` file. Only simple cases of `.odb` outputs are supported. The rest of the data is not automatically read, it can nevertheless be attained using

```
r1=sdthdf('open',file_allNodal.mat);
```

where `r1` is a cell array containing all the fields contained in the `allNodal.mat` file.

`odb2sdt.py` reference

The following lists the main subfunctions in `odb2sdt.py`

<code>matFile(fname)</code>	Creation of a the file <code>fname</code> , with the standard <code>.mat</code> header. <code>f=matFile(fname)</code>
<code>dof2mat(f, fields, fieldItems, stepName)</code>	Writes the DOF array in SDT format to file <code>f</code> . <code>fields</code> is the list of <code>fieldOutput</code> instances from the step named <code>stepName</code> . <code>fieldItems</code> is the sorted list containing the displacement fieldOutputs present in the fieldOuputs list. It must contain in that order, and at least one entry of the list [<code>'U'</code> , <code>'UR'</code> , <code>'UT'</code>]. It is a direct call with no output.
<code>defSet2mat(f, step, fieldList)</code>	Writes a fieldOutput set for all frames of a step, contiguously into file <code>f</code> . <code>step</code> is a step instance, <code>fieldList</code> is the list of fieldOutputs to be output from the frame object. All kind of nodal vector output can be treated although this was designed to treat displacement fields linked to the <code>dof2mat</code> function. It is a direct call with no output. In case of a modal deformation set, the <code>EIGIMAG</code> , <code>EIGFREQ</code> , <code>EIGREAL</code> and <code>DAMPRATIO</code> history-Output data are also output.
<code>nodalScalarValues2mat(f, field, stepName, frameName)</code>	Outputs an array of scalar nodal values to file <code>f</code> , for a particular fieldOutput instance <code>field</code> . <code>stepName</code> is the name of the step considered, <code>frameName</code> the name of the frame. However, since the fieldOutput is given the last two arguments are strings only needed to compose the array name in <code>f</code> .It is a direct call with no output.
<code>allNodal2mat(oddb)</code>	This function combines the lower level nodal output function to create and fill directly a <code>.mat</code> file containing DOFs, deformations sets, and nodal scalar values form an odb instance, created with <code>openOdb</code> . It is a direct call with no output.

The following are lower level calls, and alternative calls, with output in the workspace.

<code>sortFieldList(fieldList)</code>	<p>Returns a field keys list in which the existing displacement field keys have been sorted at the list beginning, in the order 'U', 'UR' , 'UT'. <code>fieldList=sortFieldList(fieldList)</code>.</p>
<code>rmFromList(list1, list2)</code>	<p>Returns <code>list1</code> in which the items in <code>list2</code> have been removed.</p>
<code>arrayHead2mat(f, nValSize, isCpx, dim1,dim2, arrayName)</code>	<p>Low level command. Initialization of an array entry into the file <code>f</code>. The corresponding header is written such that the array values can be written right after. <code>nValSize</code> is the space needed to store the values from the array in Bytes. <code>isCpx</code> takes the value 0 if the data to store are real, or 16 if the values to store are complex. <code>dim1</code> and <code>dim2</code> are the dimensions of the array in direction 1 and 2. <code>arrayName</code> is the name given to the array. It is a direct call with no output.</p>
<code>getNodeIds(frame)</code>	<p>Returns a <code>nodeId</code> array in the workspace, taken in a frame instance. <code>nodeId=getNodeIds(frame)</code></p>
<code>getLabels(frame, fieldKeys)</code>	<p>Returns the list of <code>componentLabels</code> contained in all the <code>fieldKeys</code> list, in a frame instance. It also generates a list in which the field keys are repeated to match the <code>componentLabels</code> list. <code>labels,labelField=getLabels(frame,fieldKeys)</code></p>
<code>setDOF(nodeId, field, fieldKeys)</code>	<p>Returns a DOF array interpreted from a <code>fieldOutputs</code> list, a <code>nodeId</code> array and <code>fieldKeys</code> giving the fieldOutput displacement keys relevant in <code>field</code>. <code>DOF=setDOF(nodeId,fieldOutputs,['U'])</code></p>
<code>readData(value)</code>	<p>A way to output a data member of a value instance regardless of the precision used during the computation. <code>data=readData(value)</code></p>
<code>readNodalValues(field, outList)</code>	<p>Returns optionally the <code>nodeId</code> array, the corresponding data array and the <code>componentLabels</code> lists found, from a <code>fieldOutput</code> instance. <code>OutList</code> is a list of length 3 being [1,1,1] for a complete output, [0,1,0] to output only the data array, and [1,1,0] to output the combo <code>nodeId</code> array and data <code>nodeId,data=readNodalValues(fieldOutput, [1,1,0])</code></p>

abaqus

[Examples](#)

See also

[FEMLink](#)

ans2sdt

Purpose Interface between ANSYS and SDT (part of FEMLink)

Syntax

```
ans2sdt('read FileName')           % .rst, .cdb, .matrix, .mode files
ans2sdt('write FileName')         % .cdb file
ans2sdt('BuildUp FileName')       % .rst and .emat files
... = ans2sdt('def FileName.rst') % .rst or .mode files
```

Description **BuildUp**

`ans2sdt('BuildUp FileName')` reads the binary files `FileName.rst` for model definition and `FileName.emat` for element matrices. The result is stored in `Up` (a type 3 superelement handled by `upcom`). `FileName.mat` is used to store the superelement. Valid calls are

```
Up=ans2sdt('buildup file');
[m,k]=upcom(Up, 'assemble not');
```

For recent versions of ANSYS, you will have to manually add the `ematwrite,yes` command to the input file to make sure that all element matrices are written. This command is not accessible from the ANSYS menu.

There is a partial attempt to fill in element properties in `Up.il`. You can also use `data=stack_get(model, 'info', 'RealConstants', 'getdata')` to obtain the cell array containing the ANSYS real constants for various elements. The index in this cell array corresponds to element `ProId` values.

`def`

This command reads deformations in `.mode` and `.rst` files.

When the `.rst` file can contain multiple deformations, the results is output as a cell array of curves typically stored with `mdl=stack_set(mdl, ans2sdt('def SampleData.rst'))`. The standard names used by ANSYS are NSL (displacement), VSL (velocity response), RF (reaction forces). If you are interested in reading other results, please send a test case.

`conv`

This command lists conversion tables for elements, topologies, face topologies. You can redefine (enhance) these tables by setting preferences of the form `setpref('FEMLink'`

`'ansys.elist',value)`, but please also request enhancements so that the quality of our translators is improved.

read

This command reads files based on their standard ANSYS extension.

`.matrix` files are read assuming ASCII Harwell Boeing format obtained with `HBMAT, Fname,Ext,--,ASCII,STIFF`. RHS vectors or binary matrices are not read yet.

`.mode` files contain deformations which are read into the usual SDT format. `.rst` files contains model information and the function attempts to run the `BuildUp` command.

`.cdb` input files also written by ANSYS using the `CDWRITE ALL,FileName,cdb` command. Please also request enhancements on the support of this format so that the quality of our translators is improved.

ANSYS does not store boundary conditions in the `.rst` files so that these can only be imported from `.cdb` file. If you only have fixed boundary conditions, you can easily generate those with

```
model=ans2sdt('buildup test');           % read model
def=ans2sdt('def test.rst');             % read deformations
model = fe_case(model,'fixdof','Fixed_Dofs', ...
    fe_c(model.DOF,def.DOF,'dof',2));
cf=feplot; cf.model=model; cf.def=def; % display
```

Def

`def=ans2sdt('def FileName.rst')` or `def=ans2sdt('def FileName.mode')` reads deformations in `.rst` or `.mode` files

Write

`ans2sdt('write FileName.cdb',model)` is the current prototype for the ANSYS writing capability. In ANSYS `.cdb` files are written with the `CDWRITE ALL, FileName, cdb` command. This does not currently write a complete `.CDB` file so that some manual editing is needed for an ANSYS run after the write.

See also

[FEMLink](#)

basis

Purpose Coordinate system handling utilities

Syntax

```
p          = basis(x,y)
[bas,x]    = basis(node)
[ ... ]    = basis('Command', ... )
```

Description `[nodeGlob,bas]=basis('nodebas',model)`

NodeBas performs a *local to global node transformation* with recursive transformation of coordinate system definitions stored in **bas**. Column 2 in **nodeLocal** is assumed give displacement coordinate system identifiers **PID** matching those in the first column of **bas**. `[nodeGlobal,bas]= basis(nodeLocal,bas)` is an older acceptable format. `-force` is a command option used to resolve all dependencies in **bas** even when no local coordinates are used in **node**.

Coordinate systems are stored in a matrix where each row represents a coordinate system using any of the three formats

```
CorID Type RefID A1  A2  A3  B1 B2 B3 C1 C2 C3 0 0 0 s
CorID Type 0     NIdA NIdB NIdC 0 0 0 0 0 0 0 0 0 s
CorID Type 0     Ax Ay Az      Ux Uy Uz Vx Vy Vz Wx Wy Wz s
```

Supported coordinate types are **1** rectangular, **2** cylindrical, **3** spherical. For these types, the nodal coordinates in the initial **nodeLocal** matrix are **x y z, r teta z, r teta phi** respectively.

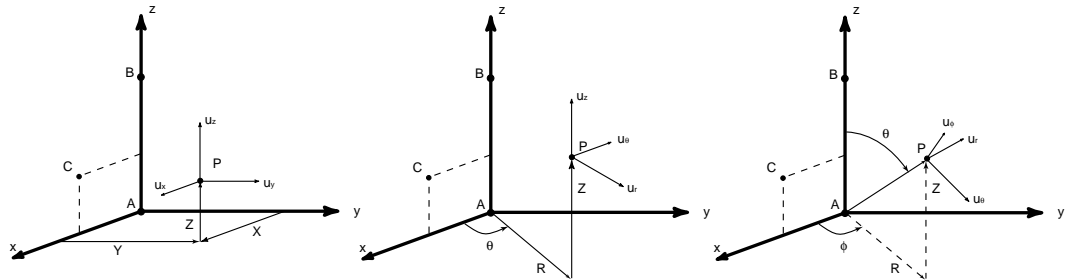


Figure 9.1: Coordinates convention.

The first format defines the coordinate system by giving the coordinates of three nodes **A**, **B**, **C** as shown in the figure above. These coordinates are given in coordinate system **RefID** which can be 0 (global coordinate system) or another **CordId** in the list (recursive definition).

The second format specifies the same nodes using identifiers **NIdA**, **NIdB**, **NIdC** of nodes defined in **node**.

The last format gives, in the global reference system, the position **Ax Ay Az** of the origin of the coordinate system and the directions of the **x**, **y** and **z** axes. When storing these vectors as columns one thus builds the $x_G = [cGL] x_L$ transform.

The **s** scale factor can be used to define position of nodes using two different unit systems. This is used for test/analysis correlation. The scale factor has no effect on the definition of displacement coordinate systems.

```
cGL= basis('trans [ ,t][ ,l][,e]',bas,node,DOF)
```

The *transformation basis for displacement coordinate systems* is returned with this call. Column 3 in **node** is assumed give displacement coordinate system identifiers **DID** matching those in the first column of **bas**.

By default, **node** is assumed to be given in global coordinates. The **l** command option is used to tell basis that the nodes are given in local coordinates.

Without the **DOF** input argument, the function returns a transformation defined at the 3 translations and 3 rotations at each node. The **t** command option restricts the result to translations. With the **DOF** argument, the output is defined at DOFs in **DOF**. The **e** command option returns a square transformation matrix.

```
nodeGlobal = basis('gnode',bas,nodeLocal)
```

Given a single coordinate system definition **bas**, associated nodes **nodeLocal** (with coordinates **x y z**, **r teta z**, **r teta phi** for Cartesian, cylindrical and spherical coordinate systems respectively) are transformed to the global Cartesian coordinate system. This is a low level command used for the global transformation **[node,bas] = basis(node,bas)**.

bas can be specified as a string compatible with a **basis('rotate'** call. In such case, the actual basis is generated on the fly by **basis('rotate')** before applying the node transformation.

```
[p,nodeL] = basis(node)
```

Element basis computation With two output arguments and an input `node` matrix, `basis` computes an appropriate local basis `bas` and node positions in local coordinates `x`. This is used by some element functions (`quad4`) to determine the element basis.

```
basis('rotate')
```

`bas=basis('rotate',bas,'command',basId)`; is used to perform rotations on coordinate systems of `bas` given by their `basId`. `command` is a string to be executed defining rotation in degrees (`rx=45`; defines a 45 degrees rotation along x axis). One can define more generally rotation in relation to another axis defining angle `r=angle` and axis `n=[nx,ny,nz]`. It is possible to define translations (an origin displacement) by specifying in `command` translation values under names `tx`, `ty` and `tz`, using the same formalism than for rotations.

For example, one can define a basis using

```
bas=basis('rotate',[],'rz=30;',1); % 30 degrees / z axis
bas=basis('rotate',[],'r=30;n=[0 1 1]',1); % 30 degrees / [0 1 1] axis
bas=basis('rotate',[],'tx=12;',1); % translation of 12 along x
bas=basis('rotate',[],'ty=24;r=15;n=[1 1 1];',1); % translation of 24 along y
```

```
p = basis(x,y)
```

Basis from nodes (typically used in element functions to determine local coordinate systems). `x` and `y` are two vectors of dimension 3 (for finite element purposes) which can be given either as rows or columns (they are automatically transformed to columns). The orthonormal matrix `p` is computed as follows

$$p = \left[\frac{\vec{x}}{\|\vec{x}\|}, \frac{\vec{y}_1}{\|\vec{y}_1\|}, \frac{\vec{x} \times \vec{y}_1}{\|\vec{x}\| \|\vec{y}_1\|} \right]$$

where \vec{y}_1 is the component of \vec{y} that is orthogonal to \vec{x}

$$\vec{y}_1 = \vec{y} - \vec{x} \frac{\vec{x}^T \vec{y}}{\|\vec{x}\|^2}$$

If `x` and `y` are collinear `y` is selected along the smallest component of `x`. A warning message is passed unless a third argument exists (call of the form `basis(x,y,1)`).

```
p = basis([2 0 0],[1 1 1]) gives the orthonormal basis matrix p
```

```
p =
```

basis

```
1.0000    0    0
    0    0.7071 -0.7071
    0    0.7071  0.7071
```

See also

[beam1](#), section 7.1,section 7.2

Note : the name of this function is in conflict with [basis](#) of the *Financial Toolbox*.

comgui,cingui

Purpose General utilities for graphical user interfaces.

Syntax

```
comgui('Command', ...)  
cingui('Command', ...)
```

`comgui` is an open source function that the user is expected to call directly while `cingui` is closed source and called internally by SDT.

`ImWrite, ...`

`ImwriteFileName.ext` does a clean print of the current figure. It can be called as a command with the options detailed below. The same command can be used to prepare single or multiple image capture, it is then called using `comgui('ImWrite',gf,RO)` with options stored in the `RO` structure. For details on the structure, and for multi-image capture specific options, see `iicom ImWrite`.

- `FileName` The default extension is `.png`. With no file name a dialog opens to select one.
- `-NoCrop` (or `RO.NoCrop=1`) avoids the default behavior where white spaces are eliminated around bitmap images.
- `-FTitle` (or `RO.FTitle=1`) uses the title/legend information to generate a file name starting with the provided filename.

A typical example would be `comgui('imwrite-FTitle plots/root')` which will generate a `root_detail.png` file in local directory `plots`.

For a given plot, `comgui('imFTitle')` can be used to check the target name.

- `-LaTeX` (or `RO.LaTeX=1`) displays LATEX commands to be used to include the figure in a file.
- `-objSet"@Rep{SmallWide}"` provides a tag to obtain predefined `comgui objSet` information to format the figure. Default formats available are
 - `SmallWide` for a wide picture (landscape style) adapted to reports.
 - `SmallSquare` for a square picture (4:3) adapted to reports.
 - `SmallHigh` for a vertical rectangular picture (9:16) adapted to reports.

- `LargeWide` for a wide picture (landscape style) adapted to posters.
- `LargeSquare` for a square picture (4:3) adapted to posters.
- `-clipboard` copies to clipboard.
- `-SubToFig` copies the display to another figure before reformatting (avoids modifying the current figure).
- `-Java` (or `RO.Java=1`) uses java to to a screen capture.
- `-open` (or `RO.open=1`) opens the figure in a browser.
- It is possible to give as a final argument the figure number, to ensure the corret figure it captured. This feature is mostly useful for non `iiplot` and `feplot` figures.

FitLabel

`comgui('fitlabel')` attempts to replace axes of the current figure so that `xlabel`, `ylabel`, ... are not cropped.

objSet

Uniform formatting of figures is often a necessity. The `cingui('objSet',h,Prop)` is the SDT utility to store reusable formatting instructions. `Prop` is a cell array of tags and values classical in MATLAB properties with possible modifications.

- `position` accepts `NaN` for reuse of current values. Thus `[NaN NaN 300 100]` only sets width and height.
- `@xlabel` accepts a value that is a cell array that will be propagated for all x labels. A typical example would be `{'@xlabel',{'FontSize',12}}`. Other accepted components are `@ylabel`, `@zlabel`, `@title`, `@axes`, `@text`,
- `@tag` is assumed to search for object with the given tag, so that its properties can be set. For example `{'@ii_legend',{'FontSize',12}}` will set the fontsize of an object with tag `ii_legend`.
- `@tickfcn` allows a tick generation callback, see `TickFcn`

For the property value, a replacement mechanism is provided if the value is a string starting with `@`.

- The value is a default stored in `sdt_table_generation('Command')`. One can search values by name within a cell array. This is in particular used for preset report formats `@Rep{SmallWide}` in `comgui ImWrite`.
- The value is obtained as the result of a callback `'@setlines(''marker'')` or `'@out=setlines(''marker'');`. This is illustrated in the example below.
- an empty type such as `'',@Rep{SmallWide}` is used to obtain predefined lists of properties.

```
figure(1);t=linspace(0,2*pi);h=plot(t,[1:3]'*sin(t));
cingui('objset',1, ... % Handle to the object to modify
{'',@Rep{SmallWide}', ... % Predefined figure type
 '@line',@setlines(''marker'')} % Line sequencing
```

def.Legend

The `def.Legend` field is used to control dynamic generation of text associated with a given display. It is stored using the classical form of property/value pairs stored in a cell array, whose access can be manual or more robustly done with `sdsetprop`. Accepted properties any text property (see `doc text`) and the specific, case ensitive, properties

- `set` gives the initialization command in a string. This command if of the form `'legend -corner .01 .01 -reset'` with
 - `cornerx y` gives the position of the legend corner with respect to the current axis.
 - `-reset` option deletes any legend existing in the current axis.
- `string` gives a cell array of string whos rows correspond to lines of the legend. `$title` is replaced by the string that would classically be displayed as label by `feplot`.
- `ImWrite` can be used to control file name generation (later used in automated multiple figure generation, see `iicom ImWrite`). The format in this case is a cell array giving the target directory followed by components used to build the string. Numbers then indicate rows of the legend text.

```
[model,def]=hexa8('testeig');cf=feplot(model);
def.Legend={'set','legend -corner .1 .9 -reset', ... % Init
'string',{'\it MyCube';'$title'}, ... % The legend strings
'FontSize',12} % Other test properties
def=sdsetprop(def,'Legend','ImWrite',{sdtdef('tempdir'),'FigRoot', ...
2,1, ... % insert second and first legend lines in file name
'.png'}) % Generate file as png
cf.def=def;
comgui('imFTitle') % Display the file name used comgui('imwrite')
```


commode

Purpose General purpose command parser for user interface command functions.

Syntax `Commode ('CommandFcn','ChainOfCommands')`

Description Most user interfaces operations in the *Structural Dynamics Toolbox* are grouped in UI command functions (`iicom`, `idcom`, `fecom`, `feutil`, etc.). The desired commands and possible options are passed to the command functions as text commands.

Conventions used in the helps to specify string commands used by user interface functions are

<i>italic</i>	standard names for numerical or string values
()	same as italic but used for the on-line rather than HTML help
[c1,c2]	alternatives for a command (separated by commas)

Thus `ch[,c] [i,+,-,+i,-i]` means that `ch 14`, `chc 12:14`, `chc+`, `ch-2` are all valid commands. Commands are text strings so that you can use `fecom ch[1,4]` or `fecom('ch 1 4')` but not `fecom ch 1 4` where `ch`, `1` and `4` are interpreted by MATLAB as 3 separate strings.

When building complex commands you may need to compute the value used for variable. Some commands actually let you specify an additional numeric argument (`feplot('textnode',[1 2 3])` and `feplot('textnode 1 2 3')` are the same) but in other cases you will have to build the string yourself using calls of the form `feplot(['textnode' sprintf('%i',[1 2 3]))]`

The UI command functions only accept one command at a time, so that `commode` was introduced to allow

- *command mode*: replace the MATLAB prompt `>>` by a `CommandFcn>` which directly sends commands to the command function(s).
- *command chaining*: several commands separated by semi-columns `;`. The parsing is then done by `commode`.
- *scripting*: execute all commands in a file.

Most command functions send a command starting by a `';` to `commode` for parsing. Thus `commode ('iicom','cax1; abs')` is the same as `iicom (';cax1;abs')`

commode

The following commands are directly interpreted by `commode` (and not sent to the command functions)

`q,quit` exits the command mode provided by `commode` but not MATLAB .
`script FName` reads the file `FName` line by line and executes the lines as command strings.

The following syntax rules are common to `commode` and MATLAB

`%comment` all characters after a `%` and before the next line are ignored.
`[]` brackets can be used to build matrices.
`;` separate commands (unless within brackets to build a matrix).

See also [comstr](#), [iicom](#), [fecom](#), [femesh](#)

comstr

Purpose String handling functions for the *Structural Dynamics Toolbox*.

Syntax See details below

Description The user interfaces of the *Structural Dynamics Toolbox* have a number of string handling needs which have been grouped in the `comstr` function. The appropriate formats and usual place of use are indicated below.

```
istrue=comstr(Cam,'string')
```

String comparison. `1` is returned if the first characters of `Cam` contain the complete `'string'`. `0` is returned otherwise. This call is used extensively for command parsing. See also `strncmp`.

```
[opt,CAM,Cam]=comstr(CAM,'string','format')
```

Next string match and parameter extraction. `comstr` finds the first character where `lower(CAM)` differs from `string`. Reads the remaining string using the `sscanf` specified `format`. Returns `opt` the result of `sscanf` and `CAM` the remaining characters that could not be read with the given format.

`[opt,CAM,Cam]=comstr(CAM,'string','%c')` is used to eliminate the matching part of `string`.

```
[CAM,Cam] = comstr(CAM,ind)
```

Command segmentation with removal of front and tail blanks. The first `ind` characters of the string command in capitals `CAM` are eliminated. The front and tail blanks are eliminated. `Cam` is a lowercase version of `CAM`. This call to `comstr` is used in all UI command functions for command segmentation.

```
opt = comstr(CAM,[-1 default])
```

Option parameter evaluation. The string `CAM` is evaluated for numerical values which are output in the row vector `opt`. If a set of default values `default` is given any unspecified value in `opt` will be set to the default.

```
date = comstr(CAM,[-3])
```

Return the standard date string. Used by [ufwrite](#), [naswrite](#), etc. See also [date](#), [datenum](#).

```
CAM = comstr(CAM,[-4 nc ])
```

Fills the string [CAM](#) with blanks up to [nc](#) characters.

```
comstr(Matrix,[-5 fid], 'format')
```

Formatted output of [Matrix](#), the [format](#) is repeated as many times as [Matrix](#) has columns and a formatted output to [fid](#) (default is 1 standard output). For example you might use [comstr\(ii_mac\(md1,md2\)*100,\[-5 1\], '%6.0f'\)](#).

```
st1=comstr(st1,-7, 'string')
```

used for dynamic messaging on the command line. On UNIX platforms (the backspace does not work properly on Windows), the string [st1](#) is erased before ['string'](#) is displayed.

```
comstr(tt,-17, 'type')
```

This is used to generate tabular output of the cell array [tt](#) to various supported types : [excel](#) (Microsoft Excel only available on windows), [html](#), [csv](#) (comma separated values, readable by excel), [tex](#).

See also [commode](#)

curvemodel

Purpose Handle object for implicit representation of curves.

Syntax `h=curvemodel('Source',r1,'yRef',fun,'getXFcn',{fun,fun,fun}, ...
'DimPos',[1 3 2]);`

Description **Multi-dim curve** are multi-dimensional arrays (`.Y` field) with information about the various dimensions (`.X`,`.Xlab` fields). `curvemodel` store similar data sets but provide methods to generate the `.X`,`.Xlab`,`.Y` fields content dynamically from an information source.

`curvemodel` objects are derived from MATLAB `handle` objects. If you copy an object's handle, MATLAB copies only the handle and both the original and copy refer to the same object data.

The principle of curve models is that the computation only occurs when the user seeks the required data.

Important fields are

- `.Source` contains the data to be used as source. The source can be a pointer. For example `cf.v_handle.Stack'def1'` can be used to point to a set of deformations stored in a `feplot`, or `iiplot` stack.
- `.DimPos` is used to allow permutations of the array dimensions (implicit equivalent of `permute(c.Y,c.DimPos)`).
- `.xRef` is a cell array of length the number of dimensions in `.Y` allowing the extraction from the source.

Documented methods are

- `.GetData` : creates a copy of the full implicit data.

This functionality mostly undocumented. Support functions are `process_r` that handles delayed signal processing requests, `ii_signal` that supports `curvemodel` commands associated with signal processing. The following is an example for users willing to dig into the code.

curvemodel

```
C1=d_signal('RespsweepSpec') % Create a spectrogram model
C2=C1.GetData; % create a copy where the spectrogram is computed
C2.PlotInfo=ii_plp('plotinfo 2D');
iicom('curveinit','Spectro',C2);
```

db, phaseb

Purpose	Compute the decibel magnitude. Compute the unwrapped phase in degrees.
Syntax	<pre>m = db(xf) p = phaseb(xf)</pre>
Description	<p><code>db</code> computes the decibel magnitude of each element of the matrix <code>xf</code>. An equivalent would be</p> <pre>m = 20*log10(abs(xf))</pre> <p><code>phaseb</code> is an extension to the case of multiple FRF stacked as columns of a matrix <code>xf</code> of the <code>phase</code> routine available in the <i>System Identification Toolbox</i>. It computes the phase in degrees with an effort to keep the phase continuous for each column.</p>
Example	<p>Here is an example that generates the two FRF of a SIMO system and plots their magnitude and phase.</p> <pre>a=[0 1;-1 -.01];b=[0;1];c=[1 0;0 1];d=[0;0]; w=linspace(0,2,100)'; xf=qbode(a,b,c,d,w); clf; subplot(211);plot(w,dbsdt(xf)); title('dB magnitude') subplot(212);plot(w,phaseb(xf));title('Unwrapped phase in degrees')</pre>
See also	The <code>xf</code> format, <code>iiplot</code>

fe2ss

Purpose Build state-space or normal mode form from FE model.

Syntax

```
[sys,basis] = fe2ss('command [options]',MODEL)
[sys,basis] = fe2ss('command [options]',MODEL,C)
[nor,basis] = fe2ss('command -nor', ...)
basis       = fe2ss('command -basis', ...)
```

Description `fe2ss` is meant to allow users to build state-space (see section 5.4) and normal mode models from full order model matrices. Accepted commands are detailed below. Accepted command options

- `-nor` outputs the normal mode model data structure (see section 5.2).
- `-basis` outputs the reduction basis
- `-se` outputs a reduced superelement
- `-loss2c` performs estimates viscous damping based on hysteretic models
- `-dterm` includes static correction as a D term rather than additional modes
- `-ind` specifies indices of modes to be kept. Others are included as a D term.

The procedure is always decomposed in the following steps

- call `fe_reduc` build a reduction basis \mathbf{T} (see section 6.2). This usually includes a call to `fe_eig` with options *EigOpt* provided in the `fe2ss` command
- call `fe_norm` to orthonormalize \mathbf{T} with respect to mass and stiffness (obtain a model in the normal mode form (5.4), see section 5.2) and eliminate collinear vectors if any
- call `nor2ss` or project model matrices depending on the number of outputs

The `basis` output argument is given so that the user can call `nor2ss` repeatedly without computing the basis again. This is in particular useful for changes in the sensor configuration which have no effect on the retained basis. `-nor` and `-basis` can be used to generate the corresponding outputs.

High level input arguments are a `MODEL` (see section 4.2) with a case defined in the model (see section 4.2.3) which **must** contain load and sensor entries (see `fe_case`).

Damping can be specified in the model (using a `DefaultZeta` case entry for example), or given as an additional argument `C` which can be a system damping matrix, a scalar uniform damping ratio or a vector of damping ratios.

The following example compares various damping models.

```
mdl=demosdt('demo ubeam mix');cf=feplot;
mdl=fe_case(mdl,'SensDof','Out',[343.01 343.02 347.03]', ...
    'FixDof','base','z==0')
freq=linspace(10,1e3,2500)';mdl=stack_set(mdl,'info','Freq',freq);
% uniform 1 % modal damping
mdl=stack_rm(mdl,'info','Rayleigh');
mdl=stack_set(mdl,'info','DefaultZeta',.01);
[sys,T] = fe2ss('free 6 10',mdl);
qbode(sys,freq*2*pi,'iipplot "Modal"');
% Rayleigh damping with 1 % viscous at 200 Hz, see sdtweb('damp')
mdl=stack_rm(mdl,'info','DefaultZeta');
mdl=stack_set(mdl,'info','Rayleigh',[0 .01*2/(200*2*pi)]);
[sys2,T] = fe2ss('free 6 10',mdl);
qbode(sys2,freq*2*pi,'iipplot "Rayleigh"');
% Estimate viscous from hysteretic damping
[sys3,T] = fe2ss('free 6 10 -loss2c',mdl);
qbode(sys3,freq*2*pi,'iipplot "Loss"');

iicom('iix',{'Modal','Rayleigh','Loss'});

% display full response
cf.def=fe2ss('sysdef 2',sys,T,cf.Stack{'Freq'});
% use iimouse('cursorOnFeplot') to see deformations at various freq.
```

Free [, Float] [, -dterm] *EigOpt*

See `fe_reduc Free` for calling details, this generates the classical basis with free modes and static correction to the loads defined in the model case (see `fe_case`). With the `-dterm` option, the static correction is given as a D term rather than additional modes.

CraigBampton *nm*

It is really a companion function to `fe_reduc CraigBampton` command. The retained basis combines fixed interface attachment modes and constraint modes associated

to DOFs in `bdof`.

This basis is less accurate than the standard modal truncation for simple predictions of response to loads, but is often preferred for coupled (closed loop) predictions. In the example below, note the high accuracy up to 200 Hz.

```
mdl=demosdt('demo ubeam');cf=feplot;
mdl=fe_case(mdl,'SensDof','Out',[343.01 343.02 347.03]', ...
    'FixDof','Base','z==0')
freq=linspace(10,400,2500)';mdl=stack_set(mdl,'info','Freq',freq);
% uniform 1 % modal damping
mdl=stack_rm(mdl,'info','Rayleigh');
mdl=stack_set(mdl,'info','DefaultZeta',.01);

[sys,T] = fe2ss('CraigBampton 5 10', ...
    fe_case(mdl,'DofSet','IN',314.01));
qbode(sys,freq*2*pi,'iplot "Craig"');

% Same with free modes
[sys2,T2] = fe2ss('Free 5 10', ...
    fe_case(mdl,'Remove','IN','DofLoad','IN',314.01));
qbode(sys2,freq*2*pi,'iplot "Free" -po');

iicom('iixOnly',{'Craig','Free'});iicom(';sub 1 1;ylog')
```

Low level input format

The obsolete low level input arguments are those of `fe_reduc` with the additional damping and output shape matrix information.

```
[sys,basis] = fe2ss('command',m,k,mdof,b,rdof,C,c)
```

- `m, k` symmetric real mass and stiffness matrix
- `mdof` associated DOF definition vector describing DOFs in `m` and `k`
- `b` input shape matrix describing unit loads of interest. Must be coherent with `mdof`.
- `bdof` alternate load description by a set of DOFs (`bdof` and `mdof` must have different length)
- `rdof` contains definitions for a set of DOFs forming an isostatic constraint (see details below). When `rdof` is not given, it is determined through an LU decomposition done before the usual factorization of the stiffness. This operation takes time but may be useful with certain elements for which geometric and numeric rigid body modes don't coincide.
- `C` damping model. Can specify a full order damping matrix using the same DOFs as the system mass `M` and stiffness `K` or a scalar damping ratio to be used in a proportional damping model.
- `c` output shape matrix describing unit outputs of interest (see section 5.1). Must be coherent with `mdof`.

Standard bases used for this purpose are available through the following commands.

See also

[demo_fe](#), [fe_reduc](#), [fe_mk](#), [nor2ss](#), [nor2xf](#)

fecom

Purpose UI command function for the visualization of 3-D deformation plots

Syntax

```
fecom
fecom CommandString
fecom(cf,'CommandString')
fecom('CommandString',AdditionalArgument)
```

Description `fecom` provides a number of commands that can be used to manipulate 3-D deformation plots are handled by the `feplot/fecom` interface. A **tutorial** is given section 4.1. Other examples can be found in `gartfe`, `gartte` and other demos. Details on the interface architecture are given under `feplot`.

This help lists all commands supported by the interface (calling `fecom` or `feplot` is insensitive to the user).


- `cf1=feplot` returns a pointer to the current `feplot` figure (see section 4.1.2). The handle is used to provide simplified calling formats for data initialization and text information on the current configuration. You can create more than one `feplot` figure with `cf=feplot(FigHandle)`. If many `feplot` figures are open, one can define the target giving an `feplot` figure handle `cf` as a first argument.
- without input arguments, `fecom` calls `commode` which provides a command mode for entering different possibly chained `fecom` commands.
- the first input argument should be a string containing a single `fecom` command, or a chain of semi-column separated commands starting with a semi-column (`fecom(';com1;com2')`). Such commands are parsed by `commode`.
- some commands, such as `TextNode`, allow the use of additional arguments

AddNode,Line

These commands start to implement direct model modification in the `feplot` figure. Sample calls are illustrated in section 2.2.1.

Anim[,One][,Time,Freq][,col][nCycle *i*, Start *i*, Step]

Deformed structure animation. The animation is not movie based so that you can actively rotate, change mode, ... without delay. The `AnimStep` command is only used when you really want to create movies.

The animation is started/interrupted using the animation button  which calls the `AnimStart` command. You can set animation properties in the `General` tab of the `feplot properties` figure.

To control animation speed and replay you can use `fecom('AnimTime nStep tStep tStart')` which specifies the number of times that you want the animation to run (0 to run continuously), the minimum time spent at each time step (default zero), and the wait time between successive runs of the same animation (default 0, only works with time mode animation). You can also use `fecom('AnimTime StepInc')` to define the step increment of the animation. You may need to fix the color limits manually using `cf.ua.clim=[0 1e3]`.

```
demosdt('demobartime'); fecom AnimeTime5;
```

Accepted `Anim` options are

- `Freq` the default animation (use of `AnimFreq` to return to the default) adds a certain phase shift ($2\pi/nCycle$) to the amplification factor of the deformations currently displayed and updates the plot. The default `nCycle` value is obtained using `feplot AnimnCycle25`.
- `Time` starts the animation in a mode that increments deformations while preserving the amplification. This is appropriate for animation of time responses.
- `One` animates the current axis only rather than the default (all).
- `Col` sets color animation to dual sided (alternates between a max value and its opposite) rather than the default of no animation. You can animate colors without deformations if you define colors for the current selection without defining a deformation.
- `Slider On,Off,Tog` opens an slider to select deformation.

Animation speed is very dependent on the figure renderer. See the `fecom Renderer` command.

`AnimAvi [,AnimMovie i]`

Creating a movie. `M=feplot('anim movie 50')` returns a MATLAB movie with 50 animation steps.

Starting with MATLAB 6.0 you can use `fecom('AnimAvi')` to create a movie. The movie is created using `avifile` commands with defaults in set to `sdtdef('avi')`. You may change the defaults (see `avifile`) using for example

```
sdtdef('avi',{'quality',100,'fps',1,'compression','Cinepak'})
```

Compressions of `None` gives highest quality. `Indeo3` and `Indeo5` give poor results with colored patches.

`caxi`, `ca+`

Change current *axes*. `cax i` makes the axis *i* (an integer number) current. `ca+` makes the next axis current.

For example, `fecom(';sub2 1;cax1;show line;ca+;show sensor')` displays a line plot in the first axis and a sensor plot in the second.

See also the `Axes` tab in the `feplot properties` figure and the `iicom sub` command. In particular `SubStep` is used to increment the deformation numbers in each subplot.

`ch[,c] [i,+, -, +i, -i],`

Displayed deformation control. `feplot` is generally used to initialize a number of deformations (as many as columns in `mode`). `ch i` selects the deformation(s) *i* to be displayed (for example `ch 1 2` overlays deformations 1 and 2). By default the first deformation is displayed (for line and sensor plots with less than 5 deformations, all deformations are overlaid). You can also increment/decrement using the `ch+` and `ch-` commands or the `+` and `-` keys when the current axis is a plot axis. `ch+i` increments by *i* from the current deformation.

You can also select deformations shown in the `Deformations` tab in the `feplot properties` figure.

When using more than one axis (different views or deformations), the `ch` commands are applied to all `feplot` axes while the `chc` commands only apply to the current axis.

The `SubStep` command is useful to obtain different deformations in a series of axes. Thus to display the first 4 modes of a structure you can use: `fecom(';sub 1 1;ch1;sub 2 2 step')` where the `sub 1 1` is used to make sure that everything is reinitialized. You can then see the next four using `fecom('ch+4')`.

For line and sensor plots and multiple channels, each deformation corresponds to an object and is given a color following the `ColorOrder` of the current axis is used.

`feplot` line and sensor plots compatible with the use of `setlines` for line type sequences.

`ColorData` [`,sel i`] [`Type`] [`,-alpha i`]

Color definitions Color information is defined for element selections (see the `fecom Sel` commands) and should be defined with the selection using a call of the form, `cf.sel(i)={'SelectionString','ColorData', ...}`. `fecom('colordata sel i ...',...)` is the corresponding low level call. See also `fecom ColorBar` and `fecom ColorLegend` commands.

Accepted options for the command are

- `-alpha val` can be used to set face transparency. This is only valid using OpenGL rendering and is not compatible with the display of masses (due to a MATLAB rendering bug).
- `-edgealpha val` is used for edge transparency
- `-ColorBarTitle "val"` is used to open a colorbar with the appropriate title (see `ColorBar` and `ColorScale` commands). A `.ColorBar` field can be used for calls with a data structure input.

Accepted `ColorData` commands are listed below

- Eval** `fecom('ColorData EvalZ')` does dynamic evaluation of the color field based on current displacements. Accepted eval options are `x,y`, `z`, `a` for single axis translations or translation amplitudes. `RadZ,TanZ` for radial and tangential displacement (assumed cylindrical coordinates with `z` axis).
- Ener** `fecom('ColorData EnerK')` calls `fe_stress` to compute the strain energy for the elements in the current selection and deformation set and displays the result as element colors. For more details, see `fe_stress feplot`.
- Group, Mat, Pro, i** `fecom('ColorDataGroup')` defines a color for each element group, `Mat` for each `MatId`, and `Pro` for each `ProId`. `ColorDataI` gives a color for each separate triplet. A color map can be given as a second argument.
- `ColorData Group -edge` affects colors to nodes rather than surfaces and displays a colored wire-frame.
- The color animation mode is set to `ScaleColorOne`.
- Stress** the `ColordataStressi` command defines the selection color by calling `fe_stress` with command `Stressi`. The color animation mode is set to `ScaleColorOne`. This requires material and element properties to be defined with `InitModel`.
- x, y, z, all,DOF** `fecom('ColorDataZ')` defines a color that is proportional to motion in the `z` direction, ... `ColorData19` will select DOF 19 (pressure). The color animation mode is set to `ScaleColorDef`. `fecom('ColorDataALL')` defines a color that is proportional to motion norm.

Uniform in this mode the deformation/object index is used to define a uniform color following the axis `ColorOrder`.

Elt `fecom('ColorDataElt',data)` specifies element colors. Nominal format is a curve (see `fe_stress Ener`) or a struct with `.data .EltId`. Older formats are a `struct` with fields `.data .IndInElt` or two arguments `data,IndInElt`.

Node low level call to set a color defined at nodes `fecom('ColorData',cmode)` where `cmode` is a `size(node,1)` by `size(mode,2)` matrix defining *nodal colors* for each deformation (these are assumed to be consistent with the current deformation set). Values are scaled, see the `ScaleColor` command. `fecom('ColorDataNode',mode,mdof)` defines nodal colors that are proportional to the norm of the nodal displacement. You can obtain nodal colors linked to the displacement in a particular direction using `i1=fe_c(mdof,.03,'ind');fecom('ColorDataNode', md0(i1,:), mdof(i1))` even though for displacements in the `xyz` directions `fecom('ColorDataZ')` is shorter.

Note: When displaying results colors are sometimes scaled using the amplification factor used for deformations. Thus, to obtain color values that match your input exactly, you must use the `fecom ScaleColorOne` mode. In some animations you may need to fix the color limits manually using `cf.ua.clim=[0 1e3]`.

Color [,sel*i*] [Edge ..., Face ..., Legend]

Default `EdgeColor` and `FaceColor` properties of the different patches can be set to `none`, `interp`, `flat`, `white`, ... using `fecom('ColorEdgeNone')`, ...

`fecom('ColorEdge',ColorSpec)` where `ColorSpec` is any valid MATLAB color specification, is also acceptable.

`EdgeColor` and `FaceColor` apply to the current selection. The optional `Seli` argument can be used to change the current selection before applying the command.

You can also modify the properties of a particular object using calls of the form `set(cf.o(i),'edgecolor',ColorSpec)` (see `fecom go` commands and illustrations in `gartte`).

`fecom('ColorLegend')` uses the MATLAB legend command to create a legend for group, material or property colors. Of course, the associated selection must have such colors defined with a `Colordata[M,P,G]` command.

ColorBar

`fecom('colorbar')` calls the MATLAB `colorbar` to display a color scale to the left of the figure. `feplot` updates this scale when you change the deformation shown. Custom displays are handled by specifying the corresponding fields as further arguments in the format `fecom('colorbar','CustomField',NewVal,...)`, where `CustomField` is a standard `colorbar` field, and `NewVal` the custom value to set. `fecom ColorBarOff` is used to reinitialize a subplot without a color bar.

In the following example, one plots the actual z displacement using a custom colorbar.

```
cf=demosdt('DemoGartfePlot');
fecom('colordataEvalZ -edgealpha .1')
% Disp in CM (*100), 2sided ([-cmax cmax]), instant (updated scale)
fecom('ColorScale Unit 100 2Sided Instant');
fecom('colorbar', ...
      'units','normalized','position',[.88 .5 .04 .4], ...
      'YAxisLocation','left','FontSize',12, ...
      '@xlabel',{'String','cm','FontSize',14})
```

A `.ColorBar` field can be used for `ColorData` calls with a data structure input.

ColorAlpha

`fecom ColorAlpha` starts a specific coloring mode where the transparency is indexed on the colormap level. This can be used to highlight high strain areas in volume models. `-EdgeAlpha val` may be used to make the edges transparent.

Uniform transparency of faces and edges is obtained using the `FaceEdgeAlpha` entry in the object context menu or with a command of the form below.

```
d_ubeam; cf=feplot;
% Use Value based alpha and Set the edges to be 10% transparent
fecom('ColorAlpha -edgealpha .1');
```

ColorScale

Once colors defined with `fecom ColorData`, multiple scaling modes are supported. For calling examples, see `fecom ColorBar`. The modes are accessible through the `feplot:Anim` menu.

- `Tight` corresponds to a value of `[cmin cmax]`. `cf.ua.clim` can be used to force values.

- `1Sided` corresponds to a value of `[0 cmax]`. This is typically used for energy display.
- `2Sided` corresponds to a value of `[-cmax cmax]`. This is typically used for translations, stresses, ...
- `Fixed` the color limits set in `cf.ua.clim` are used.
- `Off` the values are set at during manual refreshes (calls to `fecom('ch')` but not during animation. This mode is useful to limit computation costs but the color may get updated at the end of an animation.
- `Instant` the values of `cmin,cmax` are obtained using the current deformation.
- `Transient` the values are obtained using a range of deformations. For time domain animation, estimation is done dynamically, so that you may have to run your animation cycle once to find the true limit.
- `One` does not scale color deformations (default starting with SDT 6.4)
- `Unit coef` defines a fixed color scaling coefficient. This is typically used to provide more convenient units (1e-6 to have stress colors in MPa rather than Pa for example).
- `Def` uses the amplification coefficient set for the associated deformation.

Cursor

If a time deformation is defined in the `feplot` figure, one can see time curve at a specific node using `fecom CursorNodeIiplot` command. A node cursor then appears on the `feplot` displayed model, and clicking on a node shows corresponding curve in the `iiplot` figure. Reciprocally one can show a cursor on the `iiplot` curve to show corresponding time deformation in `feplot` using `iicom CursorOnFeplot` command. Note that this functionality should only be used for small models.

Following example let you test this functionality.

```
model=femesh('testhexa8'); cf=feplot(model); model=cf.mdl; % simple cube
data=struct('def',[1 1 1 1]','DOF',[5 6 7 8]'+.03,...
```

```
'curve',fe_curve('test sin 10e-2 5000 1 5000e-4'));
model=fe_case(model,'DofLoad','topload',data); % sin load
model=fe_case(model,'FixDof','basefix','z==0'); % fix base
model=fe_time('timeopt newmark .25 .5 0 1e-4 5000',model); % time computation
cf.def=fe_time(model); % show time animation

fecom CursorNodeIplot % display cursor on feplot
ci=iipplot;iicom(ci,'ch',{'NodeId',5}) % Test the callback

iicom CursorOnFeplot % display cursor on iipplot

% Cursor following animation
fecom(sprintf('AnimCursor%i Start100',ci.opt(1)))
```

ga *i*

`fecom('ga i')` or `cf.ga(i)` gets pointers to the associated axes. See details under the same `iicom` command. A typical application would be to set multiple axes to the same view using `iimouse('view3',cf.ga(:))`.

go *i*

Get handles to `fecom` objects. This provides an easy mechanism to modify MATLAB properties of selected objects in the plot (see also the `set` command).

For example, `set(fecom('go2'),'linewidth',2)` will use thick lines for `feplot` object 2 (in the current `feplot` axis).

You will probably find easier to use calls of the form `cf=feplot` (to get a handle to the current `feplot` figure) followed by `set(cf.o(2),'linewidth',2)`. If the `feplot` object is associated to more than one MATLAB object (as for text, mixed plate/beam, ...) you can access separate pointers using `cf.o(2,1)`. The `gartte` demo gives examples of how to use these commands.

LabFcn

Titles for each deformation should be generated dynamically with the `def.LabFcn` callback. `def=fe_def('lab',def)` attempts to provide a meaningful default callback for the data present in the `def` structure.

The callback string is interpreted with a call to `eval` and should return a string defining the label for each channel. Local variables for the callback are `ch` (number

of the channel currently displayed in `feplot`) and `def` (current deformation).

For example `def.LabFcn='sprintf(''t=%.2f ms'',def.data(ch)*1000)'` can be used to display times of a transient response in ms.

`fecom('TitOpt111')` turns automatic titles on (see `iicom`). `fecom('TitOpt0')` turns them off.

Legend, Head

Placing a simple title over the deformation can be too coarse. Defining a `comgui` `def.Legend` field provides a more elaborate mechanism to generate multi-line legends dynamically.

The `iicom` `head` commands can be used to place additional titles in the figure. `cf.head` returns a pointer to the header axis. Mode titles are actually placed in the header axis in order to bypass inappropriate placement by MATLAB when you rotate/animate deformations.

Info

Displays information about the declared structure and the objects of the current plot in the command window. This info is also returned when displaying the *SDT handle* pointing to the `feplot` figure. Thus `cf=feplot` returns

```
cf =
  FEPLLOT in figure 2
  Selections: cf.sel(1)='groupall';
             cf.sel(2)='WithNode {x>.5}';
  Deformations: [ {816x20} ]
  Sensor Sets: [ 0 (current 1)]
  Axis 3 objects:
  cf.o(1)='sel 2 def 1 ch 9 ty1'; % mesh
  cf.o(2) % title
```

which tells what data arrays are currently defined and lists `feplot` objects in the current axis. `fecom('pro')` opens the `feplot properties` figure which provides an interactive GUI for `feplot` manipulations.

InitDef[, Back]

Initialization of deformations. You can (re)declare deformations at any point using `cf.def(i)=def`. Where `cf` a *SDT* handle to the figure of interest and *i* the

deformation set you which to modify (if only one is defined, `cf.def` is sufficient). Acceptable forms to specify the deformation are

- `def` is a structure with fields `.def`, `.DOF`, `.data`. Note that `.Legend` and `.LabFcn` can be used to control associated titles, see `comgui def.Legend`.
- `{mode,mdof,data}` a set of vectors, a vector of DOFs. For animation of test results, `mdof` can be given using the 5 column format used to define arbitrary sensor directions in `fe_sens`. The optional `data` is a vector giving the meaning of each column in `mode`. `fecom head` is used to generate the label.
- `ci.Stack{'IdMain'}`, see section 2.3.1 for identification procedures and section 5.6 for the pole residue format
- `[]` resets deformations
- `{def,'sensors'}` defines sensor motion in a case where sensors are defined in the case (that can be accessed through `cf.CStack{'sensors'}`). It is then expected that `def.DOF` matches the length of the sensor `tdof` field).
- `{def,TR}` supports automatic expansion/restitution, see illustrated in the `fe_sens WireExp` command. The same result can be obtained by defining a `def.TR` field.

`feplot(cf,'InitDef',data)` is an alternate calling format that defines the current deformation. `InitDef` updates all axes. `InitDefBack` returns without updating plots.

load, InitModel

Initialization of structure characteristics. The preferred calling format is `cf.model=model` where the fields of `model` are described in section 7.6. This makes sure that all model information is stored in the `feplot` figure. `cf.mdl` then provides a handle that lets you modify model properties in scripts without calling `InitModel` again.

Lower level calls are `cf.model={node,elt,bas}` (or `feplot('InitModel',node,elt,bas)` (see `basis` for `bas` format information). `InitModelBack` does not update the plot (you may want to use this when changing model before redefining new deformations).

The command is also called when using `femesh plotelt`, or `upcom plotelt` (which is equivalent to `cf.model=Up`). Note that `cf.model=UFS(1)` for a data stack resulting from `ufread` and `cf.model=Up` for type 3 superelement.

Load from file `fecom('Load', 'FileName')` will load the model from a binary `FileName.mat` file. `fecom('FileImportInfo')` lists supported import formats.

`fecom('Load-Back', FileName)` is used to load, but **not display** the model (this is used for very large model reading).

`fecom('Load-Hdf', 'FileName')` loads a model from a HDF5 `.mat` file but retains most data at `v_handle` pointers to the file.

InitSens

Initialization of sensors. You can declare sensors independently of the degrees of freedom used to define deformations (this is in particular useful to show measurement sensors while using `modeshape` expansion for deformations). Sensor and arrow object show the sensor sets declared using `initsens`.

Translation sensors in global coordinates can be declared using a DOF definition vector `cf.sens(i)={mdof}` or `feplot('initsens',mdof)`. In the first calling format, the current sensor set is first set to `i`.

Sensors in other directions are declared by replacing `mdof` by a 5 column matrix following the format

```
SensorId  NodeId  nx  ny  nz
```

with `SensorId` an arbitrary identifier (often 101.99 for sensor of unknown type at node 101), `NodeId` the node number of the sensor position, `[nx ny nz]` a unit vector giving the sensor direction in global coordinates (see section 3.1).

`fe_sens` provides additional tools to manipulate sensors in arbitrary directions. Examples are given in the `gartte` demo.

Plot

`feplot('plot')`, the same as `feplot` without argument, refreshes axes of the current figure. If refreshing the current axis results in an error (which may occasionally happen if you modify the plot externally), use `clf;iicom('sub')` which will check the consistency of objects declared in each axis. Note that this will delete `Text` objects as well as objects created using the `SetObject` command.

Pro

`feplot('pro')` initializes or refreshes the `feplot` property GUI. You can also use the `Edit:Feplot Properties ...` menu. A description of this GUI is made in section 4.1.

`fepplot('ProViewOn')` turns entry viewing on.

Renderer [OpenGL, zBuffer, Painters] [, default]

This command can be used to switch the renderer used by `fepplot`. Animation speed is very dependent on the figure renderer. When creating the figure `fecom` tries to guess the proper renderer to use (`painters`, `zbuffer`, `opengl`), but you may want to change it (using the `Fepplot:Render` menu or `set(gcf, 'renderer', 'painters')`, ...). `painters` is still good for wire frame views, `zbuffer` has very few bugs but is very slow on some platforms, `opengl` is generally fastest but still has some significant rendering bugs on UNIX platforms.

To avoid crashes when opening `fepplot` in OpenGL mode use `cingui('Renderer zbuffer default')` in your MATLAB startup file.

Save, FileExport

Save the model to a `.mat` file or export it to supported formats.

`fecom('FileExportInfo')` lists supported export formats.

`fecom('Save -savesel file.mat')` also saves the selection(s) which allows faster reload of large models. `fecom('Save -savedef file.mat')` also saves the deformations(s).

Scale [,Dofs, Dof*i*, equal, match, max, one]

Automatic deformation scaling. Scaling of deformations is the use of an amplification factor very often needed to actually see anything. A deformation scaling coefficient is associated with each deformed object. The `Scale` commands let you modify all objects of the current axis as a group.

You can specify either a length associated with the maximum amplitude or the scaling coefficient.

The base coefficient `scc` for this amplification is set using `fecom('ScaleCoef scc')`, while `fecom('ScaleDef scd')` sets the target length. `fecom('scd 0.01')` is an accepted shortcut. If `scd` is zero an automatic amplitude is used. You can also modify the scaling deformation using the `l` or `L` keys (see `iimouse`).

`fecom` supports various scaling modes summarized in the table below. You can set this modes with `fecom('scalemax')` ... commands.

Scaling mode	Scaling of 1st deformation	Scaling of other deformations
<code>max</code>	Amplitude of Max DOF set to <code>scd</code> .	Amplitude of Max DOF set to <code>scd</code> .
<code>equal</code>	Amplitude of Max DOF set to <code>scd</code> .	Amplitude of other deformations equal to the first one
<code>match</code>	Amplitude of Max DOF set to <code>scd</code> .	Amplitude of other deformations set to optimize superposition. When using two deformation sets, rather than two modes in the same set, their DOFs must be compatible.
<code>coef</code>	Deformation amplitude multiplied by <code>scd</code> .	Same as first deformation.
<code>one</code>	Sets <code>scd</code> to 1 and uses <code>coef</code> mode (so further changes to <code>scd</code> lead to amplification that is not equal to 1).	Same as first deformation.

Warning : using `ScaleMax` or `AnimFreq` can lead to negative or complex amplification factors which only makes sense for frequency domain shapes. `fecom('scalecoef')` will come back to positive amplification of each object in the current `feplot` axis.

`ScaleDof i` is used to force the scaling DOF to be `i`. As usual, accepted values for `i` are of the form `NodeId.DofId` (1.03 for example). If `i` is zero or not a valid DOF number an automatic selection is performed. `ScaleDof` can only be used with a single deformation set.

You can change the `scale` mode using the `FEplot:Scale` menu or in the `Axes` tab of the `feplot properties` figure.

`Sel [ElementSelectors, GroupAll, Reset]`

Selection of displayed elements. What elements are to be displayed in a given object is based on the definition of a selection (see section 7.12).

The default command is `'GroupAll'` which selects all elements of all element groups (see section 7.2 for details on model description matrices). `cf.sel(1)='Group1 3:5'` will select groups 1, 3, 4 and 5. `cf.sel(1)='Group1 & ProId 2 & WithNode {x>0}'` would be a more complex selection example.

To define other properties associated with the selection (`fecom ColorData` in particular), use a call of the form `cf.sel(i)={'SelectionString', 'OtherProp', OtherProp}`

To return to the default selection use `fecom('SelReset')`.

`fecom('Sel ... -linface')` can be used to generate first order faces for second order elements, which allows faster animation.

`SetObject i [,ty j] [,def k] [,ch k] [,sel s, sen s]`

Set properties of object i. Plots generated by `feplot` are composed of a number of objects. 1 (surface view), 2 (wire frame view), 3 (stick view of sensors), 4 (undeformed structure), 5 (node text labels), 6 (DOF text labels), 7 (arrow view of sensors). A printout of objects existing in the current plot and their properties is given when displaying the *SDT handle* associated to the `feplot` figure (using `cf=feplot;disp(cf)`).

The `SetObject` command lets you modify these properties and add/remove new objects. Objects are added if the object number does not exist and removed if the declared object type is zero ($j=0$).

The deformation number k is an index in the deformation(s) currently selected for the plot using the `fecom ch`.

The elements actually displayed can be specified by giving an existing selection number s . For sensor objects, you can specify the sensor set with `sen s` if you don't want to use the current one.

The current axis summary obtained with `cf=feplot` gives the preferred calling format `cf.o(i)='SetObjArgs'` where you give a string with the options associated to the `SetObject` command. The use of these commands is illustrated in the `gartte` demo. Note that you can use the call, to set other MATLAB properties of the created objects. `cf.o(1)={'ty2 def1 ch1 sel1','linewidth',2,'edgecolor','r'}` will, for example, create a wire-frame object using thick red lines.

`Show [patch,line,sensor,arrow, ...]`

Basic plots are easily created using the `show` commands which are available in the `FEplot:Show ...` menu).

<code>patch</code>	surface view with hidden face removal and possible color coding (initialized by <code>fecom('ShowPatch')</code>). <code>cf.o(1)</code> object type is 1. For color coding, see <code>colordata</code> commands.
<code>line</code>	wire frame plot of the deformed structure (initialized by <code>fecom('ShowLine')</code>). <code>cf.o(2)</code> object type is 2.
<code>sens</code>	<i>Sensor plots</i> with sticks at sensor locations in the direction and with the amplitude of the response (initialized by <code>fecom('ShowSen')</code>). <code>cf.o(2)</code> object type is 3.
<code>arrow</code>	<i>Sensor plots</i> with arrows at sensor locations in the direction and with the amplitude of the response (initialized by <code>fecom('ShowArrow')</code>). <code>cf.o(2)</code> object type is 7.
<code>DefArrow</code>	Deformation plots with lines connecting the deformed and undeformed node positions. (initialized by <code>fecom('ShowDef')</code>). <code>cf.o(2)</code> object type is 8.
<code>Bas len</code>	shows triaxes centered at the position of each local basis. The <code>length</code> of the triax arrow is specified by <code>len</code> .
<code>FEM</code>	only shows FEM element groups for models mixing test and FEM information
<code>test</code>	only shows test element groups for models mixing test and FEM information
<code>links</code>	shows a standard plot with the test and FEM meshes as well as links used for topological correlation (see <code>fe_sens</code>).
<code>map</code>	<code>fecom('ShowMap',MAP)</code> displays the vector map specified in MAP (see <code>feutil GetNormalMap</code>). Nota : to see the real orientation, use the <code>fecom('scaleone');</code> instruction. <code>fecom('ShowUndef',MAP)</code> also displays the underlying structure. MAP can also be an element selection, as in the example below <code>demosdt('demogartfeplot');fecom('ShowMap','EltName quad4')</code>
<code>NodeMark</code>	<code>fecom('shownodemark',1:10,'color','r','marker','o')</code> displays the node positions of given <code>NodeId</code> (here 1 to 10) as a line. Here a series of red points with a o marker. You can also display positions with <code>fecom('shownodemark',[x y z],'marker','x')</code> . Command option <code>-noclear</code> allows to overlay several <code>shownodemark</code> plots, <i>e.g.</i> to show two distinct sets of nodes with different colors at once.
<code>Traj</code>	<code>fecom('ShowTraj',(1:10)')</code> displays the trajectories of the node of NodeIds 1 to 10 for current deformation. Command option <code>-axis</code> is used to display axis node trajectories.
<code>2def</code>	is used for cases where you want to compare two deformations sets. The first two objects only differ but the deformation set they point to (1 and 2 respectively). A typical call would be <code>cf.def(1)={md1,mdof,f1}; cf.def(2)={md2,mdof,f2}; fecom('show2def')</code> .

Once the basic plot created, you can add other objects or modify the current list using the `Text` and `SetObject` commands.

`Sub [i j], SubIso, SubStep`

Drawing figure subdivision (see `iicom` for more details). This lets you draw more than one view of the same structure in different axes. In particular the `SubIso` command gives you four different views of the same structure/deformation.

`SubStep` or `Sub i j Step` increments the deformation shown in each subplot. This command is useful to show various modeshapes in the same figure. Depending on the initial state of the figure, you may have to first set all axes to the same channel. Use `fecom('ch1;sub 2 2 step')` for example.

`Text [off, Node [,Select], Dof d]`

Node/DOF text display. `TextOff` removes all text objects from the current `feplot` axis. `TextNode` displays the numbers of the nodes in `FENode`. You can display only certain node numbers by a node selection command `Select`. Or giving node numbers in `fecom('textnode', i)`. Text properties can be given as extract arguments, for example `fecom('textnode', i, 'FontSize', 12, 'Color', 'r')`.

`TextDOF` displays the sensor node and direction for the current sensor

`TextDOF Name` displays sensor labels of a `cf.CStack{'Name'} SenDof` entry. Additional arguments can be used to modify the text properties. `fecom('textdof')` displays text linked to currently declared sensors, see `feplot InitSens` command (note that this command is being replaced by the use of `SensDof` entries).

`TitOpt [,c] i`

Automated title/label generation options. `TitOpt i` sets title options for all axes to the value `i`. `i` is a three digit number with units corresponding to `title`, decades to `xlabel` and hundreds to `ylabel`. By adding a `c` after the command (`TitOptC 111` for example), the choice is only applied to the current axis.

The actual meaning of options depends on the plot function (see `iipplot`). For `feplot`, titles are shown for a non zero title option and not shown otherwise. Title strings for `feplot` axes are defined using the `fecom head` command.

Triax [, On, Off]

Orientation triax. Orientation of the plotting axis is shown using a small triax. `Triax` initializes the triax axis or updates its orientation. `TriaxOff` deletes the triax axis (in some plots you do not want it to show). Each triax is associated to a given axis and follows its orientation. The triax is initially positioned at the lower left corner of the axis but you drag it with your mouse.

Finally can use `fecom('triaxc')` to generate a triax in a single active subplot.

UnDef [, Dot, Line, None]

Undeformed structure appearance. The undeformed structure is shown as a line which is made visible/invisible using `UnDef` (`UnDefNone` forces an invisible mesh). When visible, the line can show the node locations (use `UnDefDot`) or link nodes with dotted lines (use `UnDefLine`).

View [...]

Orientation control. See `iimouse view`.

See also `feplot`, `fe_exp`, `feutil`

femesh

Purpose Finite element mesh handling utilities.

Syntax

```
femesh CommandString  
femesh('CommandString')  
[out,out1] = femesh('CommandString',in1,in2)
```

Description You should use `feutil` function that provides equivalent commands to `femesh` but using model data structure.

`femesh` provides a number of tools for mesh creation and manipulation. `femesh` uses global variables to define the proper object of which to apply a command. `femesh` uses the following *standard global variables* which are declared as global in your workspace when you call `femesh`

<code>FEnode</code>	main set of nodes
<code>FEn0</code>	selected set of nodes
<code>FEn1</code>	alternate set of nodes
<code>FEelt</code>	main finite element model description matrix
<code>FEel0</code>	selected finite element model description matrix
<code>FEel1</code>	alternate finite element model description matrix

By default, `femesh` automatically uses base workspace definitions of the standard global variables (even if they are not declared as global). When using the standard global variables within functions, you should always declare them as global at the beginning of your function. If you don't declare them as global modifications that you perform will not be taken into account, unless you call `femesh` from your function which will declare the variables as global there too. The only thing that you should avoid is to use `clear` (instead of `clear global`) within a function and then reinitialize the variable to something non-zero. In such cases the global variable is used and a warning is passed.

Available `femesh` commands are

;

Command chaining. Commands with no input (other than the command) or output argument, can be chained using a call of the form `femesh(';Com1;Com2')`. `commode` is then used for command parsing.

Add FEel *i* FEel *j*, AddSel

Combine two FE model description matrices. The characters *i* and *j* can specify any of the main **t**, selected **0** and alternate **1** finite element model description matrices. The elements in the model matrix **FEel *j*** are appended to those of **FEel *i***.

AddSel is equivalent to **AddFEeltFEel0** which adds the selection **FEel0** to the main model **FEelt**.

This is an example of the creation of **FEelt** using 2 selections (**FEel0** and **FEel1**)

```
femesh('Reset');
femesh('Testquad4'); % one quad4 created
femesh('Divide',[0 .1 .2 1],[0 .3 1]); % divisions
FEel0=FEel0(1:end-1,:); % suppress 1 element in FEel0
femesh('AddSel'); % add FEel0 into FEelt
FEel1=[Inf abs('tria3');9 10 12 1 1 0]; % create FEel1
femesh('Add FEelt FEel1'); % add FEel1 into FEelt
femesh PlotElt % plot FEelt
```

AddNode [,New] [, From *i*] [,eps1 *val*]

Combine, append (without/with new) **FEen0** to **FEenode**. Additional uses of **AddNode** are provided using the format

```
[AllNode,ind]=femesh('AddNode',OldNode,NewNode);
```

which combines **NewNode** to **OldNode**. **AddNode** finds nodes in **NewNode** that coincide with nodes in **OldNode** and appends other nodes to form **AllNode**. **ind** gives the indices of the **NewNode** nodes in the **AllNode** matrix.

NewNode can be specified as a matrix with three columns giving **xyz** coordinates. The minimal distance below which two nodes are considered identical is given by **sdtdef eps1** (default **1e-6**).

```
[AllNode,ind]=femesh('AddNode From 10000',OldNode,NewNode);
```

 gives node numbers starting at 10000 for nodes in **NewNode** that are not in **OldNode**.

SDT uses an optimized algorithm available in **feutilb**. See **feutil AddNode** for more details.

AddTest [, -EGID *i*] [,NodeShift, Merge, Combine]

Combine test and analysis models. When combining test and analysis models you typically want to overlay a detailed finite element mesh with a coarse wire-frame

representation of the test configuration. These models coming from different origins you will want combine the two models in `FEelt`.

By default the node sets are considered to be disjoint. New nodes are added starting from `max(FEnode(:,1))+1` or from `NodeShift+1` if the argument is specified. Thus `femesh('addtest NodeShift',TNode,TElt)` adds test nodes `TNode` to `FEnode` while adding `NodeShift` to their initial identification number. The same `NodeShift` is added to node numbers in `TElt` which is appended to `FEelt`. `TElt` can be a wire frame matrix read with `ufread`.

With `merge` it is assumed that some nodes are common but their numbering is not coherent. `femesh('addtest merge',NewNode,NewElt)` can also be used to merge to FEM models. Non coincident nodes (as defined by the `AddNode` command) are added to `FEnode` and `NewElt` is renumbered according to the new `FEnode`. `Merge-Edge` is used to force mid-side nodes to be common if the end nodes are.

With `combine` it is assumed that some nodes are common and their numbering is coherent. Nodes with new `NodeId` values are added to `FEnode` while common `NodeId` values are assumed to be located at the same positions.

You can specify an `EGID` value for the elements that are added using `AddTest -EGID -1`. In particular negative `EGID` values are display groups so that they will be ignored in model assembly operations.

The combined models can then be used to create the test/analysis correlation using `fe_sens`. An application is given in the `gartte` demo, where a procedure to match initially different test and FE coordinate frames is outlined.

Divide *div1 div2 div3*

Mesh refinement by division of elements. `Divide` applies to all groups in `FEe10`.

See equivalent `feutil Divide` command.

```
% Example 1 : beam1
femesh('Reset');
femesh(';Testbeam1;Divide 3;PlotE10'); % divide by 3
fecom TextNode
```

```
% Example 2 : you may create a command string
number=3;
st=sprintf(';Testbeam1;Divide %f;PlotE10',number);
femesh('Reset');
femesh(st);
```



```

fecom TextNode

% Example 3 : you may use uneven division
femesh('Reset');femesh('testquad4'); % one quad4 created
femesh('DivideElt',[0 .1 .2 1],[0 .3 1]);
femesh PlotE10

```

DivideInGroups

Finds groups of **FEe10** elements that are not connected (no common node) and places each of these groups in a single element group.

```

femesh('Reset');femesh('testquad4'); % one quad4 created
femesh('RepeatSel 2 0 0 1'); % 2 quad4 in the same group
femesh('DivideInGroups'); % 2 quad4 in 2 groups

```

DivideGroup *i* ElementSelectors

Divides a single group *i* of **FEelt** in two element groups. The first new element group is defined based on the element selectors (see section 7.12).

Extrude *nRep tx ty tz*

Extrusion. Nodes, lines or surfaces that are currently selected (put in **FEe10**) are extruded *nRep* times with global translations *tx ty tz*.

You can create irregular extrusions giving a second argument (positions of the sections for an axis such that **tx ty tz** is the unit vector).

See **feutil Extrude** for more details.

```

% Example 1 : beam
femesh('Reset');
femesh('Testbeam1'); % one beam1 created
femesh(';Extrude 2 1 0 0;PlotE10'); % 2 extrusions in x direction

% Example 2 : you may create the command string
number=2;step=[1 0 0];
st=sprintf(';Testbeam1;Extrude %f %f %f %f',[number step]);
femesh('Reset');
femesh(st); femesh PlotE10

```

```
% Example 3 : you may use uneven extrusions in z direction
femesh('Reset'); femesh('Testquad4')
femesh('Extrude 0 0 0 1', [0 .1 .2 .5 1]); %
% 0 0 0 1      : 1 extrusion in z direction
% [0 .1 .2 .5 1] : where extrusions are made
femesh PlotE10
```

FindElt *ElementSelectors*

Find elements based on a number of selectors described in section 7.12. The calling format is

```
[ind,elt] = femesh('FindElt withnode 1:10')
```

where **ind** gives the row numbers of the elements (but not the header rows except for unique superelements which are only associated to a header row) and **elt** (optional) the associated element description matrix. **FindE10** applies to elements in **FEe10**.

When operators are accepted, equality and inequality operators can be used. Thus **group~=[3 7]** or **pro < 5** are acceptable commands. See also **SelElt**, **RemoveElt** and **DivideGroup**, the **gartfe** demo, **fecom** selections.

FindNode *Selectors*

Find node numbers based on a number of selectors listed in section 7.11.

Different selectors can be chained using the logical operations **&** (finds nodes that verify both conditions), **|** (finds nodes that verify one or both conditions). Condition combinations are always evaluated from left to right (parentheses are not accepted).

Output arguments are the numbers **NodeID** of the selected nodes and the selected nodes **node** as a second optional output argument.

As an example you can show node numbers on the right half of the **z==0** plane using the commands

```
fecom('TextNode',femesh('FindNode z==0 & x>0'))
```

Following example puts markers on selected nodes

```
model=demosdt('demo ubeam'); femesh(model); % load U-Beam model
fecom('ShowNodeMark',femesh('FindNode z>1.25'),'color','r')
fecom('ShowNodeMark',femesh('FindNode x>0.2*z|x<-0.2*z'),...
      'color','g','marker','o')
```

Note that you can give numeric arguments to the command as additional `femesh` arguments. Thus the command above could also have been written

```
fecom('TextNode',femesh('FindNode z== & x>=',0,0)))
```

See also the `gartfe` demo.

`Info [,FEelti, Nodei]`

Information on global variables. `Info` by itself gives information on all variables. The additional arguments `FEelt ...` can be used to specify any of the main `t`, selected `0` and alternate `1` finite element model description matrices. `InfoNodei` gives information about all elements that are connected to node `i`. To get information in `FEelt` and in `FEnode`, you may write

```
femesh('InfoElt') or femesh('InfoNode')
```

`Join [,e10] [group i, EName]`

*Join the groups *i* or all the groups of type EName.* `JoinAll` joins all the groups that have the same element name. By default this operation is applied to `FEelt` but you can apply it to `FEe10` by adding the `e10` option to the command. Note that with the selection by group number, you can only join groups of the same type (with the same element name).

```
femesh('Reset'); femesh(';Test2bay;PlotElt');
% Join using group ID
femesh('InfoElt'); % 2 groups at this step
femesh JoinGroup1:2 % 1 group now
% Join using element name
femesh('Reset'); femesh(';Test2bay;PlotElt');
femesh Joinbeam1 % 1 group now
```

`Model [,0]`

`model=femesh('Model')` returns the FEM structure (see section 7.6) with fields `model.Node=FEnode` and `model.Elt=FEelt` as well as other fields that may be stored in the `FE` variable that is persistent in `femesh`. `model=femesh('Model0')` uses `model.Elt=FEe10`.

ObjectBeamLine *i*, ObjectMass *i*

Create a group of `beam1` elements. The node numbers *i* define a series of nodes that form a continuous beam (for discontinuities use 0), that is placed in `FEe10` as a single group of `beam1` elements.

For example `femesh('ObjectBeamLine 1:3 0 4 5')` creates a group of three `beam1` elements between nodes 1 2, 2 3, and 4 5.

An alternate call is `femesh('ObjectBeamLine',ind)` where `ind` is a vector containing the node numbers. You can also specify a element name other than `beam1` and properties to be placed in columns 3 and more using `femesh('ObjectBeamLine -EltName',ind,prop)`.

`femesh('ObjectMass 1:3')` creates a group of concentrated `mass1` elements at the declared nodes.

```
femesh('Reset')
FENode = [1 0 0 0 0 0 0; 2 0 0 0 0 0 .15; ...
          3 0 0 0 .4 1 .176;4 0 0 0 .4 .9 .176];
prop=[100 100 1.1 0 0]; % MatId ProId nx ny nz
femesh('ObjectBeamLine',1:4,prop);femesh('AddSel');
%or femesh('ObjectBeamLine 1 2 0 2 3 0 3 4;AddSel');
% or femesh('ObjectBeamLine',1:4);
femesh('ObjectMass',3,[1.1 1.1 1.1])
femesh AddSel
femesh PlotElt; fecom TextNode
```

ObjectHoleInPlate

Create a `quad4` mesh of a hole in a plate. The format is `'ObjectHoleInPlate NO N1 N2 r1 r2 ND1 ND2 NQ'`. See `feutil ObjectHoleInPlate` for more details.

```
FENode = [1 0 0 0 0 0 0; 2 0 0 0 1 0 0; 3 0 0 0 0 2 0];
femesh('ObjectHoleInPlate 1 2 3 .5 .5 3 4 4');
femesh('Divide 3 4'); % 3 divisions around, 4 divisions along radii
femesh PlotE10
% You could also use the call
FENode = [1 0 0 0 0 0 0; 2 0 0 0 1 0 0; 3 0 0 0 0 2 0];
% n1 n2 n3 r1 r2 nd1 nd2 nq
r1=[ 1 2 3 .5 .5 3 4 4];
st=sprintf('ObjectHoleInPlate %f %f %f %f %f %f %f %f',r1);
femesh(st); femesh('PlotE10')
```

ObjectHoleInBlock

Create a `hexa8` mesh of a hole in a rectangular block. The format is `'ObjectHoleInBlock x0 y0 z0 nx1 ny1 nz1 nx3 ny3 nz3 dim1 dim2 dim3 r nd1 nd2 nd3 ndr'`. See `feutil ObjectHoleInBlock` for more details.

```
femesh('Reset')
femesh('ObjectHoleInBlock 0 0 0 1 0 0 0 1 1 2 3 3 .7 8 8 3 2')
femesh('PlotE10')
```

Object[Quad,Beam,Hexa] MatId ProId

Create or add a model containing `quad4` elements. The user must define a rectangular domain delimited by four nodes and the division in each direction. The result is a regular mesh.

For example `femesh('ObjectQuad 10 11',nodes,4,2)` returns model with 4 and 2 divisions in each direction with a `MatId` 10 and a `ProId` 11.

```
node = [0 0 0; 2 0 0; 2 3 0; 0 3 0];
femesh('Objectquad 1 1',node,4,3); % creates model
femesh('PlotElt')
```

```
node = [3 0 0; 5 0 0; 5 2 0; 3 2 0];
femesh('Objectquad 2 3',node,3,2); % matid=2, proid=3
femesh('PlotElt'); femesh Info
```

Divisions may be specified using a vector between `[0,1]` :

```
node = [0 0 0; 2 0 0; 2 3 0; 0 3 0];
femesh('Objectquad 1 1',node,[0 .2 .6 1],linspace(0,1,10));
femesh('PlotElt');
```

Other supported object topologies are beams and hexaedrons. For example

```
femesh('Reset')
node = [0 0 0; 2 0 0; 1 3 0; 1 3 1];
femesh('Objectbeam 3 10',node(1:2,:),4); % creates model
femesh('AddSel');
femesh('Objecthexa 4 11',node,3,2,5); % creates model
femesh('AddSel');
femesh PlotElt; femesh Info
```

`Object[Arc, Annulus, Circle,Cylinder,Disk]`

Build selected object in `FEel0`. See `feutil Object` for a list of available objects. For example:

```
femesh('Reset')
femesh(';ObjectArc 0 0 0 1 0 0 0 1 0 30 1;AddSel');
femesh(';ObjectArc 0 0 0 1 0 0 0 1 0 30 1;AddSel');
femesh(';ObjectCircle 1 1 1 2 0 0 1 30;AddSel');
femesh(';ObjectCircle 1 1 3 2 0 0 1 30;AddSel');
femesh(';ObjectCylinder 0 0 0 0 0 4 2 10 20;AddSel');
femesh(';ObjectDisk 0 0 0 3 0 0 1 10 3;AddSel');
femesh(';ObjectAnnulus 0 0 0 2 3 0 0 1 10 3;AddSel');
femesh('PlotElt')
```

`Optim [Model, NodeNum, EltCheck]`

`OptimModel` removes nodes unused in `FEelt` from `FENode`.

`OptimNodeNum` does a permutation of nodes in `FENode` such that the expected matrix bandwidth is smaller. This is only useful to export models, since here DOF renumbering is performed by `fe_mk`.

`OptimEltCheck` attempts to fix geometry pathologies (warped elements) in `quad4`, `hexa8` and `penta6` elements.

`Orient, Orient i [, n nx ny nz]`

Orient elements. For volumes and 2-D elements which have a defined orientation, `femesh('Orient')` calls element functions with standard material properties to determine negative volume orientation and permute nodes if needed. This is in particular needed when generating models via `Extrude` or `Divide` operations which do not necessarily result in appropriate orientation (see `integrules`). When elements are too distorted, you may have a locally negative volume. A warning about `warped` volumes is then passed. You should then correct your mesh. Note that for 2D meshes you need to use 2D topology holders `q4p`, `t3p`, ...

Orient normal of shell elements. For plate/shell elements (elements with parents of type `quad4`, `quadb` or `tria3`) in groups `i` of `FEelt`, this command computes the local normal and checks whether it is directed towards the node located at `nx ny nz`. If not, the element nodes are permuted so that a proper orientation is achieved. A `-neg` option can be added at the end of the command to force orientation away rather than towards the nearest node.

`femesh('Orient i',node)` can also be used to specify a list of orientation nodes. For each element, the closest node in `node` is then used for the orientation. `node` can be a standard 7 column node matrix or just have 3 columns with global positions.

For example

```
% Init example
femesh('Reset'); femesh(';Testquad4;Divide 2 3;')
FEelt=FEel0; femesh('DivideGroup1 withnode1');
% Orient elements in group 2 away from [0 0 -1]
femesh('Orient 2 n 0 0 -1 -neg');
```

Plot [Elt, E10]

Plot selected model. `PlotElt` calls `feplot` to initialize a plot of the model contained in `FEelt`. `PlotE10` does the same for `FEel0`. This command is really just the declaration of a new model using `feplot('InitModel',femesh('Model'))`.

Once the plot initialized you can modify it using `feplot` and `fecom`.

Lin2quad, Quad2Lin, Quad2Tria, etc.

Basic element type transformations.

Element type transformation are applied to elements in `FEel0`. See `feutil Lin2Quad` fore more details and a list of transformations.

```
% create 4 quad4
femesh(';Testquad4;Divide 2 3');
femesh('Quad2Tria'); % conversion
femesh PlotE10
% create a quad, transform to triangles, divide each triangle in 4
femesh(';Testquad4;Quad2Tria;Divide2;PlotE10;Info');
% lin2quad example:
femesh('Reset'); femesh('Testhexa8');
femesh('Lin2Quad epsl .01');
femesh('Info')
```

RefineBeam l

Mesh refinement. This function searches `FEel0` for beam elements and divides elements so that no element is longer than `l`.

Remove[Elt,E10] *ElementSelectors*

Element removal. This function searches `FEelt` or `FEe10` for elements which verify certain properties selected by *ElementSelectors* and removes these elements from the model description matrix. A sample call would be

```
% create 4 quad4
femesh('Reset'); femesh(';Testquad4;Divide 2 3');
femesh('RemoveE10 WithNode 1')
femesh PlotE10
```

RepeatSel *nITE tx ty tz*

Element group translation/duplication. `RepeatSel` repeats the selected elements (`FEe10`) *nITE* times with global axis translations *tx ty tz* between each repetition of the group. If needed, new nodes are added to `FENode`. An example is treated in the `d_truss` demo.

```
femesh('Reset'); femesh(';Testquad4;Divide 2 3');
femesh(';RepeatSel 3 2 0 0'); % 3 repetitions, translation x=2
femesh PlotE10
% alternate call:
%                                     number, direction
% femesh(sprintf(';repeatsel %f %f %f %f', 3, [2 0 0]))
```

Rev *nDiv OrigID Ang nx ny nz*

Revolution of selected elements in `FEe10`. See `feutil Rev` for more details. For example:

```
FENode = [1 0 0 0 .2 0 0; 2 0 0 0 .5 1 0; ...
          3 0 0 0 .5 1.5 0; 4 0 0 0 .3 2 0];
femesh('ObjectBeamLine',1:4);
femesh('Divide 3')
femesh('Rev 40 0 0 0 360 0 1 0');
femesh PlotE10
fecom(';Triax;View 3;ShowPatch')
% An alternate calling format would be
%   divi origin angle direct
%r1 = [40 0 0 0 360 0 1 0];
%femesh(sprintf('Rev %f 0 %f %f %f %f %f %f',r1))
```


RotateSel *OrigID Ang nx ny nz*

Rotation. The selected elements **FEe10** are rotated by the angle *Ang* (degrees) around an axis passing through the node of number *OrigID* (or the origin of the global coordinate system) and of direction [*nx ny nz*] (the default is the **z** axis [0 0 1]). The origin can also be specified by the xyz values preceded by an **o**

```
femesh('RotateSel o 2.0 2.0 2.0    90 1 0 0')
```

This is an example of the rotation of **FEe10**

```
femesh('Reset');
femesh(';Testquad4;Divide 2 3');
% center is node 1, angle 30, around axis z
%
%                               Center angle dir
st=sprintf(';RotateSel %f %f %f %f %f',[1    30    0 0 1]);
femesh(st); femesh PlotE10
fecom(';Triax;TextNode'); axis on
```

Sel [Elt,E10] *ElementSelectors*

Element selection. **SelElt** places in the selected model **FEe10** elements of **FEelt** that verify certain conditions. You can also select elements within **FEe10** with the **SelE10** command. Available element selection commands are described under the **FindElt** command and section 7.12.

```
femesh('SelElt ElementSelectors').
```

SelGroup *i*, SelNode *i*

Element group selection. The element group *i* of **FEelt** is placed in **FEe10** (selected model). **SelGroup*i*** is equivalent to **SelEltGroup*i***.

Node selection. The node(s) *i* of **FEnode** are placed in **FEn0** (selected nodes).

SetGroup [*i,name*] [Mat *j*, Pro *k*, EGID *e*, Name *s*]

Set properties of a group. For group(s) of **FEelt** selector by number *i*, name *name*, or **all** you can modify the material property identifier *j*, the element property identifier *k* of all elements and/or the element group identifier *e* or name *s*. For example

```
femesh('SetGroup1:3 pro 4')
femesh('SetGroup rigid name celas')
```

If you know the column of a set of element rows that you want to modify, calls of the form `FEelt(femesh('FindEltSelectors'),Column)= Value` can also be used.

```
model=femesh('Testubeamplot');
FEelt(femesh('FindEltwithnode {x==-.5}'),9)=2;
femesh PlotElt;
cf.sel={'groupall','colordatamat'};
```

You can also use `femesh('set groupa 1:3 pro 4')` to modify properties in `FEe10`.

`SymSel OrigID nx ny nz`

Plane symmetry. `SymSel` replaces elements in `FEe10` by elements symmetric with respect to a plane going through the node of number `OrigID` (node 0 is taken to be the origin of the global coordinate system) and normal to the vector `[nx ny nz]`. If needed, new nodes are added to `FEnode`. Related commands are `TransSel`, `RotateSel` and `RepeatSel`.

Test

Some unique element model examples. See list with `femesh('TestList')`. For example a simple cube model can be created using

```
model=femesh('TestHexa8'); % hexa8 test element
```

`TransSel tx ty tz`

Translation of the selected element groups. `TransSel` replaces elements of `FEe10` by their translation of a vector `[tx ty tz]` (in global coordinates). If needed, new nodes are added to `FEnode`. Related commands are `SymSel`, `RotateSel` and `RepeatSel`.

```
femesh('Reset');
femesh(';Testquad4;Divide 2 3;AddSel');
femesh(';TransSel 3 1 0;AddSel'); % Translation of [3 1 0]
femesh PlotElt
fecom(';Triax;TextNode')
```

`UnJoin Gp1 Gp2`

Duplicate nodes which are common to two groups. To allow the creation of interfaces with partial coupling of nodal degrees of freedom, `UnJoin` determines which nodes

are common to the element groups *Gp1* and *Gp2* of **FEelt**, duplicates them and changes the node numbers in *Gp2* to correspond to the duplicate set of nodes. In the following call with output arguments, the columns of the matrix **InterNode** give the numbers of the interface nodes in each group **InterNode = femesh('UnJoin 1 2')**.

```
femesh('Reset'); femesh('Test2bay');  
femesh('FindNode group1 & group2') % nodes 3 4 are common  
femesh('UnJoin 1 2');  
femesh('FindNode group1 & group2') % no longer any common node
```

A more general call allows to separate nodes that are common to two sets of elements **femesh('UnJoin', 'Selection1', 'Selection2')**. Elements in *Selection1* are left unchanged while nodes in *Selection2* that are also in *Selection1* are duplicated.

See also

[fe_mk](#), [fecom](#), [feplot](#), section 4.2, demos [gartfe](#), [d_ubeam](#), [beambar](#) ...

feutil

Purpose Finite element mesh handling utilities.

Syntax `[out,out1] = feutil('CommandString',model,...)`

Description `feutil` provides a number of tools for mesh creation and manipulation.

Some commands return the model structure whereas some others return only the element matrix. To mesh a complex structure one can mesh each subpart in a different model structure (mdl0, mdl1, ...) and combine each part using `AddTest` command. To handle complex model combination (not only meshes but whole models with materials, bases, ...), one can use the `feutilb CombineModel` command.

Available `feutil` commands are

Advanced

Advanced command with non trivial input/output formats or detailed options are listed under `feutila`.

AddElt

```
model.Elt=feutil('AddElt',model.Elt,'EltName',data)
```

This command can be used to add new elements to a model. `EltName` gives the element name used to fill the header. `data` describes elements to add (one row per element). Following example adds `celas` elements to the basis of a simple cube model.

```
femesh('Reset'); model=femesh('Testhexa8'); % simple cube model
data=[1 0 123 0 0 1 1e3; 2 0 123 0 0 1 1e3;
      3 0 123 0 0 1 1e3; 4 0 123 0 0 1 1e3]; % n1 n2 dof1 dof2 EltId ProId k
model.Elt=feutil('AddElt',model.Elt,'celas',data);
cf=feplot(model);
```

AddNode [,New] [, From i] [,eps1 val]

```
[AllNode,ind]=feutil('AddNode',OldNode,NewNode);
```

Combine (without command option `New`) or *append* (with command option `New`) `NewNode` to `OldNode`. Without command option `New`, `AddNode` combines `NewNode`

to `OldNode`: it finds nodes in `NewNode` that coincide with nodes in `OldNode` and appends other nodes to form `AllNode`. With command option `New`, `AddNode` simply appends `NewNode` to `OldNode`.

`AllNode` is the new node matrix with added nodes. `ind` (optional) gives the indices of the `NewNode` nodes in the `AllNode` matrix.

`NewNode` can be specified as a matrix with three columns giving `xyz` coordinates. The minimal distance below which two nodes are considered identical is given by `sdtdef eps1` (default `1e-6`).

`[AllNode,ind]=feutil('AddNode From 10000',OldNode,NewNode);` gives node numbers starting at 10000 for nodes in `NewNode` that are not in `OldNode`.

SDT uses an optimized algorithm available in `feutilb`.

By default, nodes that repeated in `NewNode` are coalesced onto the same node (a single new node is added). If there is not need for that coalescence, you can get faster results with `AddNode-nocoal`.

`ind=feutilb('AddNode -near eps1 value',n1,n2);` returns a sparse matrix with non zero values in a given column indicating of `n1` nodes that are within `eps1` of each `n2` node (rows/columns correspond to `n1/n2` node numbers).

`id=feutilb('AddNode -nearest eps1 value',n1,xyz);` returns vector giving the nearest `n1` `NodeId` to each `xyz` node the search area being limited to `eps1`. When specified with a 7 column `n2`, the result is `sparse(n2(:,1),1,n1_index)`. For fine meshes the algorithm can use a lot of memory. If `n2` is not too large it is then preferable to use an `AddNode` command with a tolerance sufficient for a match `[n3,ind]=feutil('AddNode eps1 value',n1,n2);id=n3(ind,1)`.

AddSet [NodeId, EltId, FaceId]

`model=feutil('AddSetNodeId',model,'name','FindNodeString')` adds the selection `FindNodeString` as a `set` of nodes `name` to `model`. `'FindNodeString'` can be replaced by a column vector of `NodeId`. Syntax is the same for `AddSetEltId` with a `FindEltString` selection.

The option `-id value` can be added to the command to specify a set ID.

Following example defines a set of each type on the `ubeam` model:

```
model=demosdt('demo ubeam');
cf=feplot
model=feutil('AddSetNodeId',model,'nodeset','z==1');
model=feutil('AddSetEltId -id18',model,'eltset','WithNode{z==0}');
```

```
[elt,ind]=feutil('FindElt setname eltset',model); % FindElt based on set name
r1=cf.Stack{'eltset'};r1.type='FaceId';r1.data(:,2)=1;
cf.Stack{'set','faceset'}=r1;
r1=cf.Stack{'nodeset'};r1.type='DOF';r1.data=r1.data+0.02;
cf.Stack{'set','dofset'}=r1;
fecom(cf,'curtab Stack','eltset');
```

`AddTest` `[-EGID i]` `[,NodeShift,Merge,Combine]`

```
model=feutil('AddTest',mdl1,mdl2);
```

Combine test and analysis models. When combining test and analysis models you typically want to overlay a detailed finite element mesh with a coarse wire-frame representation of the test configuration. These models coming from different origins you will want combine the two models in `model`.

If you aim at combining several finite element models into an assembly, with proper handling of materials, element IDs, bases,..., you should rather use the more appropriate `feutilb CombineModel` command.

- **By default** the node sets are considered to be disjoint. New nodes are added starting from `max(mdl1.Node(:,1))+1` or from `NodeShift+1` if the argument is specified. Thus `feutil('AddTest NodeShift',mdl1,mdl2)` adds `mdl2` nodes to `mdl1.Node` while adding `NodeShift` to their initial identification number. The same `NodeShift` is added to node numbers in `mdl2.Elt` which is appended to `mdl1.Elt`. `mdl2` can be a wire frame matrix read with `ufread` for example.
- With command option `Merge` it is assumed that some nodes are common but their numbering is not coherent. Non coincident nodes (as defined by the `AddNode` command) are added to `mdl1.Node` and `mdl2.Elt` is renumbered according to resulting `model.Node`. Command option `Merge-Edge` is used to force mid-side nodes to be common if the end nodes are.
- With command option `Combine` it is assumed that some nodes are common and their numbering is coherent. Nodes of `mdl2.Node` with new `NodeId` values are added to `mdl1.Node` while common `NodeId` values are assumed to be located at the same positions.
- You can specify an `EGID` value for the elements that are added using `AddTest -EGID -1` for example. In particular negative `EGID` values are display groups so that they will be ignored in model assembly operations. Command option

`keepertest` allows to retain existing test frames when adding a new one. If the same `EGID` is declared, test frames are then combined in the same group.

- Command option `-NoOri` returns model without the `Info,OrigNumbering` entry in the model stack.

The combined models can then be used to create the test/analysis correlation using `fe_sens`. An application is given in the `gartte` demo, where a procedure to match initially different test and FE coordinate frames is outlined. See `sdtweb('pre')` for details about test frame meshing strategies.

`AddTest` command attempts to retain as much information as possible (nodes, elements, materials, etc.) when adding the 2 models. This feature is however rather limited with complex models.

Divide `div1 div2 div3`

```
model=feutil('Divide div1 div2 div3',model);
```

Mesh refinement by division of elements. `Divide` applies to all groups in `model.Elt`. To apply the division to a selection within the model use `ObjectDivide`. Currently supported divisions are

- segments : elements with `beam1` parents are divided in `div1` segments of equal length.
- quadrilaterals: elements with `quad4` or `quadb` parents are divided in a regular mesh of `div1` by `div2` quadrilaterals.
- hexahedrons: elements with `hexa8` or `hexa20` parents are divided in a regular grid of `div1` by `div2` by `div3` hexahedrons.
- `tria3` can be divided with an equal division of each segment specified by `div1`.
- `mass1` and `celas` elements are kept unchanged.

If your elements have a different name but the same topological structure declare the proper parent name or use the `SetGroupName` command before and after `divide`. The division preserves properties other than the node numbers.

You can obtain unequal divisions by declaring additional arguments whose lines give the relative positions of dividers. For example, an unequal 2 by 3 division of a `quad4` element would be obtained using

```
model=feutil('divide',[0 .1 1],[0 .5 .75 1],model) (see also the gartfe demo).
```

```
% Example 1 : beam1
femesh('Reset'); model=femesh('Testbeam1'); % build simple beam model
model=feutil('Divide 3',model); % divide by 3
cf=feplot(model); fecom('TextNode'); % plot model and display NodeId

% Example 2 : you may create a command string
femesh('Reset'); model=femesh('Testbeam1'); % build simple beam model
number=3;
st=sprintf('Divide %f',number);
model=feutil(st,model);
cf=feplot(model); fecom('TextNode')

% Example 3 : you may use uneven division
femesh('Reset'); model=femesh('Testquad4'); % one quad4 created
model=feutil('Divide',model,[0 .1 .2 1],[0 .3 1]);
feplot(model);
```

An inconsistency in division for quad elements was fixed with version 1.105, you can obtain the consistent behavior (first division along element x) by adding the option `-new` anywhere in the `divide` command.

DivideInGroups

```
elt=feutil('DivideInGroups',model);
```

Finds groups that are not connected (no common node) and places each of these groups in a single element group.

DivideGroup *i* ElementSelectors

```
elt=feutil('DivideGroup i ElementSelector',model);
```

Divides a single group i in two element groups. The first new element group is defined based on the element selectors (see section 7.12).

For example `elt=feutil('divide group 1 withnode{x>10}',model);`

EltId

`[EltId]=feutil('EltId',elt)` returns the element identifier for each element in `elt`. It currently does not fill `EltId` for elements which do not support it.

[EltId,elt]=feutil('EltIdFix',elt) returns an `elt` where the element identifiers have been made unique.

Command option `-elt` can be used to set new `EltId`.

```
model=femesh('TestHexa8')
[EltId,model.Elt]=feutil('EltIdFix',model.Elt); % Fix and get EltId
[model.Elt,EltIdPos]=feutil('eltid-elt',model,EltId*18); % Set new EltId
model.Elt(EltIdPos>0,EltIdPos(EltIdPos>0)) % New EltId
```

Extrude *nRep tx ty tz*

Extrusion. Nodes, lines or surfaces of model are extruded *nRep* times with global translations *tx ty tz*. Elements with a `mass1` parent are extruded into beams, element with a `beam1` parent are extruded into `quad4` elements, `quad4` are extruded into `hexa8`, and `quadb` are extruded into `hexa20`.

You can create irregular extrusions. For example, `model=feutil('Extrude 0 0 0 1',model,[0 logspace(-1,1,5)])` will create an exponentially spaced mesh in the *z* direction. The second argument gives the positions of the sections for an axis such that *tx ty tz* is the unit vector.

```
% Example 1 : beam
femesh('Reset'); model=femesh('Testbeam1'); % one beam1 created
model=feutil('Extrude 2 1 0 0',model); % 2 extrusions in x direction
cf=feplot(model);
```

```
% Example 2 : you may create the command string
number=2;step=[1 0 0];
st=sprintf('Extrude %f %f %f %f',[number step]);
femesh('Reset'); model=femesh('Testbeam1'); % one beam1 created
model=feutil(st,model);
cf=feplot(model);
```

```
% Example 3 : you may uneven extrusions in z direction
femesh('Reset'); model=femesh('Testquad4');
model=feutil('Extrude 0 0 0 1',model,[0 .1 .2 .5 1]);
    % 0 0 0 1      : 1 extrusion in z direction
    % [0 .1 .2 .5 1] : where extrusions are made
feplot(model)
```

GetDof *ElementSelectors*

Command to obtain DOF from a model, or from a list of `NodeId` and DOF.

Use `mdof=feutil('GetDof',dof,NodeId);` to generate a DOF vector from a list of DOF indices `dof`, a column vector (e.g. `dof=[.01;.02;.03]`), and a list of `NodeId`, a column vector. The result will be sorted by DOF, equivalent to `mdof = [NodeId+dof(1);NodeId+dof(2);...]`.

Call `mdof=feutil('GetDof',NodeId,dof);` will output a DOF vector sorted by `NodeId`, equivalent to `mdof = [NodeId(1)+dof;NodeId(2)+dof;...]`.

The nominal call to get DOFs used by a model is `mdof=feutil('GetDOF',model)`. These calls are performed during assembly phases (`fe_mk`, `fe_load`, ...). This supports elements with variable DOF numbers defined through the element rows or the element property rows. To find DOFs of a part of the model, you should add a `ElementSelector` string to the `GetDof` command string.

Note that node numbers set to zero are ignored by `feutil` to allow elements with variable number of nodes.

FindElt *ElementSelectors*

Find elements based on a number of selectors described in section 7.12. The calling format is

```
[ind,elt] = feutil('FindElt ElementSelector',model);
```

where `ind` gives the row numbers of the elements in `model.Elt` (but not the header rows except for unique superelements which are only associated to a header row) and `elt` (optional) the associated element description matrix.

When operators are accepted, equality and inequality operators can be used. Thus `group~=[3 7]` or `pro < 5` are acceptable commands. See also `SelElt`, `RemoveElt` and `DivideGroup`, the `gartfe` demo, `fecom` selections.

FindNode *Selectors*

Find node numbers based on a number of node selectors listed in section 7.11.

Different selectors can be chained using the logical operations `&` (finds nodes that verify both conditions), `|` (finds nodes that verify one or both conditions). Condition combinations are always evaluated from left to right (parentheses are not accepted).

The calling format is

```
[NodeId,Node] = feutil('FindNode NodeSelector',model);
```

Output arguments are the `NodeId` of the selected nodes and the selected nodes `Node` as a second optional output argument.

As an example you can show node numbers on the right half of the `z==0` plane using the commands

```
fecom('TextNode',feutil('FindNode z==0 & x>0',model))
```

Following example puts markers on selected nodes

```
demosdt('demo ubeam'); cf=feplot; % load U-Beam model
fecom('ShowNodeMark',feutil('FindNode z>1.25',cf.mdl),'color','r')
fecom('ShowNodeMark-noclear',feutil('FindNode x>0.2*z|x<-0.2*z',cf.mdl),
      'color','g','marker','o')
```

Note that you can give numeric arguments to the command as additional `feutil` arguments. Thus the command above could also have been written `feutil('FindNode z== & x>=',0,0)`

See also the `gartfe` demo.

GetEdge[Line,Patch]

These `feutil` commands are used to create a mode containing the 1D edges or 2D faces of a model. A typical call is

```
femesh('reset'); model=femesh('Testubeam');
elt=feutil('GetEdgeLine',model); feutil('infoelt',elt)
```

`GetEdgeLine` supports the following variants `MatId` retains inter material edges, `ProId` retains inter property edges, `Group` retains inter group edges, `all` does not eliminate internal edges, `InNode` only retains edges whose node numbers are in a list given as an additional `feutil` argument.

These commands are used for `SelEdge` and `SelFace` element selection commands. `SelFace` preserves the `EltId` and adds the `FaceId` after it to allow face set recovery.

GetElemF

Header row parsing. In an element description matrix, element groups are separated by header rows (see section 7.2) which for the current group `jGroup` is given

by `elt(EGroup(jGroup),:)` (one can obtain `EGroup` - the positions of the headers in the element matrix - using `[EGroup,nGroup]=getegroup(model.Elt)`). The `GetElemF` command, whose proper calling format is

```
[ElemF,opt,ElemP] = feutil('GetElemF',elt(EGroup(jGroup),:),[jGroup])
```

returns the element/superelement name `ElemF`, element options `opt` and the parent element name `ElemP`. It is expected that `opt(1)` is the `EGID` (element group identifier) when defined.

Get [Line,Patch]

`Line=feutil('GetLine',node,elt)` returns a matrix of lines where each row has the form `[length(ind)+1 ind]` plus trailing zeros, and `ind` gives node indices (if the argument `node` is not empty) or `node` numbers (if `node` is empty). `elt` can be an element description matrix or a connectivity line matrix (see `feplot`). Each row of the `Line` matrix corresponds to an element group or a line of a connectivity line matrix. For element description matrices, redundant lines are eliminated.

`Patch=feutil('GetPatch',Node,Elt)` returns a patch matrix where each row (except the first which serves as a header) has the form `[n1 n2 n3 n4 EltN GroupN]`. The `ni` give node indices (if the argument `Node` is not empty) or node numbers (if `Node` is empty). `Elt` must be an element description matrix. Internal patches (it is assumed that a patch declared more than once is internal) are eliminated.

The `all` option skips the internal edge/face elimination step. These commands are used in wire-frame and surface rendering.

GetNode Selectors

`Node=feutil('GetNode Selectors',model)` returns a matrix containing nodes rather than `NodeIds` obtained with the `feutil FindNode` command. The indices of the nodes in `model.Node` can be returned as a 2nd optional output argument. This command is equivalent to the `feutil` call

```
[NodeId,Node]=feutil('FindNode Selectors',model).
```

GetNormal [Elt,Node] [,Map],GetCG

`[normal,cg]=feutil('GetNormal [elt,node]',model)` returns normals to elements/nodes in `model`.

`CG=feutil('GetCG',model)` returns the CG locations. Command option `-dir i` can be used to specify a local orientation direction other than the normal (this is

typically used for composites).

`MAP=foutil('getNormal Map',model)` returns a data structure with the following fields

<code>ID</code>	column of identifier (as many as rows in the <code>.normal</code> field). For <code>.opt=2</code> contains the <code>NodeId</code> . For <code>.opt=1</code> contains the <code>EltId</code> .
<code>normal</code>	$N \times 3$ where each row specifies a vector at <code>ID</code> or <code>vertex</code> .
<code>opt</code>	1 for MAP at element center, 2 for map at nodes.
<code>vertex</code>	$N \times 3$ matrix giving vertex positions if the map is not associated with nodes.

The MAP data structure may be viewed using

```
fecom('ShowMap',MAP);fecom('ScaleOne');
```

`Info [,Elt, Nodei]`

`foutil('Info',model)`; *Information on model.* `Info` by itself gives general information about `model`. `InfoNodei` gives information about all elements that are connected to node of `NodeId i`.

`Join [group i, EltName]`

*Join the groups *i* or all the groups of type *EltName*.* `JoinAll` joins all the groups that have the same element name. Note that with the selection by group number, you can only join groups of the same type (with the same element name). `JoinAll` joins all groups with identical element names.

You may join groups using there ID

```
femesh('Reset'); model=femesh('Test2bay');
% Join using group ID
foutil('Info',model); % 2 groups at this step
model=foutil('JoinGroup1:2',model) % 1 group now
foutil('Info',model);
% Join using element types
% Note you can give model (above) or element matrix (below)
femesh('Reset'); model=femesh('Test2bay');
model.Elt=foutil('Joinbeam1',model.Elt); % 1 group now
```

`Matid,ProId,MPID`

`MatId=feutil('MatId',model)` returns the element material identifier for each element in `model.Elt`.

One can also modify `MatId` of the model giving a third argument. `model=feutil('MatId',model,r1)` can be a global shift on all non zero `MatId` or a matrix whose first column gives old `MatId` and second new `MatId`. `MatId` renumbering is applied to elements, `model.pl` and `model.Stack 'mat'` entries. The `ProId` command works similarly.

`MPIId` returns a matrix with three columns `MatId`, `ProId` and group numbers.

`model.Elt=feutil('mpid',model,mpid)` can be used to set properties of elements in `model.Elt` matrix.

ObjectBeamLine *i*, ObjectMass *i*

`elt=feutil('ObjectBeamLine i')`; Create a group of `beam1` elements. The node numbers *i* define a series of nodes that form a continuous beam (for discontinuities use 0), that is placed in `elt` as a single group of `beam1` elements.

For example `elt=feutil('ObjectBeamLine 1:3 0 4 5')` creates a group of three `beam1` elements between nodes 1 2, 2 3, and 4 5.

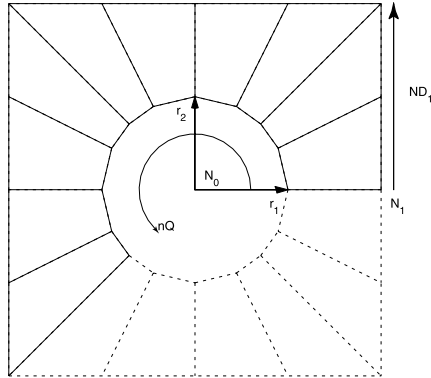
An alternate call is `elt=feutil('ObjectBeamLine',ind)` where `ind` is a vector containing the node numbers. You can also specify a element name other than `beam1` and properties to be placed in columns 3 and more using `elt=feutil('ObjectBeamLine -EltName',ind,prop)`.

`elt=feutil('ObjectMass 1:3')` creates a group of concentrated `mass1` elements at the declared nodes.

```
model=struct('Node',[1 0 0 0 0 0 0; 2 0 0 0 0 0 .15; ...
                    3 0 0 0 .4 1 .176;4 0 0 0 .4 .9 .176], 'Elt',[]);
prop=[100 100 1.1 0 0]; % MatId ProId nx ny nz
model.Elt=feutil('ObjectBeamLine 1 2 0 2 3 0 3 4',prop);
% or model.Elt=feutil('ObjectBeamLine',1:4);
model.Elt=feutil('ObjectMass',model,3,[1.1 1.1 1.1]);
%model.Elt(end+1:end+size(elt,1),1:size(elt,2))=elt;
feplot(model);fecom textnode
```

ObjectHoleInPlate

```
model=feutil('ObjectHoleInPlate ...',model);
```



Create a `quad4` mesh of a hole in a plate. The format is `'ObjectHoleInPlate NO N1 N2 r1 r2 ND1 ND2 NQ'` giving the center node, two nodes to define the edge direction and distance, two radii in the direction of the two edge nodes (for elliptical holes), the number of divisions along a half quadrant of edge 1 and edge 2, the number of quadrants to fill (the figure shows 2.5 quadrants filled).

```
model=struct('Node',[1 0 0 0 0 0 0; 2 0 0 0 1 0 0; 3 0 0 0 0 2 0],'Elt
model=feutil('ObjectHoleInPlate 1 2 3 .5 .5 3 4 4',model);
model=feutil('Divide 3 4',model); % 3 divisions around, 4 divisions along
feplot(model)
% You could also use the call
model=struct('Node',[1 0 0 0 0 0 0; 2 0 0 0 1 0 0; 3 0 0 0 0 2 0],'Elt
%   n1 n2 n3 r1 r2 nd1 nd2 nq
r1=[ 1 2 3 .5 .5 3 4 4];
st=sprintf('ObjectHoleInPlate %f %f %f %f %f %f %f %f',r1);
model=feutil(st,model);
```

ObjectHoleInBlock

`model=feutil('ObjectHoleInBlock ...')`; Create a `hexa8` mesh of a hole in a rectangular block. The format is `'ObjectHoleInBlock x0 y0 z0 nx1 ny1 nz1 nx3 ny3 nz3 dim1 dim2 dim3 r nd1 nd2 nd3 ndr'` giving the center of the block (`x0 y0 z0`), the directions along the first and third dimensions of the block (`nx1 ny1 nz1 nx3 ny3 nz3`, third dimension is along the hole), the 3 dimensions (`dim1 dim2 dim3`), the radius of the cylinder hole (`r`), the number of divisions of each dimension of the cube (`nd1 nd2 nd3`, the 2 first should be even) and the number of divisions along the radius (`ndr`).

```
mdl0=feutil('ObjectHoleInBlock 0 0 0 1 0 0 0 1 1 2 3 3 .7 8 8 3 2')
```

Object[Quad,Beam,Hexa] MatId ProId

`model=feutil('ObjectQuad MatId ProId',model,nodes,div1,div2)` Create or add a model containing `quad4` elements. The user must define a rectangular domain delimited by four nodes and the division in each direction (`div1` and `div2`). The result is a regular mesh.

For example `model=feutil('ObjectQuad 10 11',nodes,4,2)` returns model with 4 and 2 divisions in each direction with a `MatId` 10 and a `ProId` 11.

An alternate call is `model=feutil('ObjectQuad 1 1',model,nodes,4,2)`: the quadrangular mesh is added to the model.

```
node = [0 0 0; 2 0 0; 2 3 0; 0 3 0];
model=feutil('Objectquad 1 1',node,4,3); % creates model
```

```
node = [3 0 0; 5 0 0; 5 2 0; 3 2 0];
model=feutil('Objectquad 2 3',model,node,3,2); % matid=2, proid=3
feplot(model);
```

Divisions may be specified using a vector between `[0,1]` :

```
node = [0 0 0; 2 0 0; 2 3 0; 0 3 0];
model=feutil('Objectquad 1 1',node,[0 .2 .6 1],linspace(0,1,10));
feplot(model);
```

Other supported object topologies are beams and hexaedrons. For example

```
node = [0 0 0; 2 0 0; 1 3 0; 1 3 1];
model=feutil('Objectbeam 3 10',node(1:2,:),4); % creates model
model=feutil('Objecthexa 4 11',model,node,3,2,5); % creates model
feutil('infoelt',model)
```

Object[Arc, Annulus, Circle, Cylinder, Disk]

These object constructors follow the format

```
model=feutil('ObjectAnnulus x y z r1 r2 nx ny nz Nseg NsegR',model)
model=feutil('ObjectArc xc yc zc x1 y1 z1 x2 y2 z2 Nseg obt',model)
model=feutil('ObjectCircle x y z r nx ny nz Nseg',model)
model=feutil('ObjectCylinder x1 y1 z1 x2 y2 z2 r divT divZ',model)
model=feutil('ObjectDisk x y z r nx ny nz Nseg NsegR',model)
```

For example:


```

model=feutil('object arc 0 0 0 1 0 0 0 1 0 30 1');
model=feutil('object arc 0 0 0 1 0 0 0 1 0 30 1',model);
model=feutil('object circle 1 1 1 2 0 0 1 30',model);
model=feutil('object circle 1 1 3 2 0 0 1 30',model);
model=feutil('object cylinder 0 0 0 0 0 4 2 10 20',model);
model=feutil('object disk 0 0 0 3 0 0 1 10 3',model);
model=feutil('object annulus 0 0 0 2 3 0 0 1 10 3',model);
feplot(model)

```

ObjectDivide

Applies a `Divide` command to a selection within the model

```

node = [0 0 0; 2 0 0; 2 3 0; 0 3 0];
model=feutil('Objectquad 1 1',node,4,3); % creates model
model=feutil('ObjectDivide 3 2',model,'WithNode 1');
feplot(model);

```

Optim [Model, NodeNum, EltCheck]

`model.Node=feutil('Optim...',model);`
`model.Node=feutil('OptimModel',model)` removes nodes unused in `model.Elt` from `model.Node`.
`model.Node=feutil('OptimNodeNum',model)` does a permutation of nodes in `model.No` such that the expected matrix bandwidth is smaller. This is only useful to export models, since here DOF renumbering is performed by `fe_mk`.
`model=feutil('OptimEltCheck',model)` attempts to fix geometry pathologies (warped elements) in `quad4`, `hexa8` and `penta6` elements.

Orient, Orient i [, n nx ny nz]

Orient elements. For volumes and 2-D elements which have a defined orientation `model.Elt=feutil('Orient',model)` calls element functions with standard material properties to determine negative volume orientation and permute nodes if needed. This is in particular needed when generating models via `Extrude` or `Divide` operations which do not necessarily result in appropriate orientation (see `integrules`). When elements are too distorted, you may have a locally negative volume. A warning about `warped` volumes is then passed. You should then correct your mesh.

Note that for 2D meshes you need to use 2D element names (`q4p`, `t3p`, ...) rather than `quad4`, `tria3`, Typically `model.Elt=feutil('setgroup1 name q4p',model)`.

Orient normal of shell elements. For plate/shell elements (elements with parents of type `quad4`, `quadb` or `tria3`) in groups `i` of `model.Elt`, `model.Elt=feutil('Orient i n nx ny nz',model)` command computes the local normal and checks whether it is directed towards the node located at `nx ny nz`. If not, the element nodes are permuted to that a proper orientation is achieved. A `-neg` option can be added at the end of the command to force orientation away rather than towards the nearest node.

`model.Elt=feutil('Orient i',model,node)` can also be used to specify a list of orientation nodes. For each element, the closest node in `node` is then used for the orientation. `node` can be a standard 7 column node matrix or just have 3 columns with global positions.

For example

```
% Init example
femesh('Reset'); model=femesh('Testquad4');
model=feutil('Divide 2 3',model);
model.Elt=feutil('Dividegroup1 WithNode1',model);
% Orient elements in group 2 away from [0 0 -1]
model.Elt=feutil('Orient 2 n 0 0 -1 -neg',model);
MAP=feutil('GetNormal MAP',model);MAP.normal
```

`Lin2Quad`, `Quad2Lin`, `Quad2Tria`, etc.

Basic element type transformations.

`model=feutil('Lin2Quad eps1 .01',model)` is the generic command to generate second order meshes.

`Lin2QuadCyl` places the mid-nodes on cylindrical arcs.

`Lin2QuadKnownNew` can be used to get much faster results if it is known that none of the new mid-edge nodes is coincident with an existing node. The inverse operation can be performed using `Quad2Lin` command.

`Quad2Tria` searches elements for `quad4` element groups and replaces them with equivalent `tria3` element groups.

`Hexa2Tetra` replaces each `hexa8` element by four `tetra4` elements (this is really not a smart thing to do).

`Hexa2Penta` replaces each `hexa8` element by six `tetra4` elements (warning : this

transformation may lead to incompatibilities on the triangular faces). `Penta2Tetra` replaces each `penta6` element by 11 `tetra4` elements.

Command option `KnownNew` can be used for `Hexa2Tetra`, `Hexa2Penta`, and `Penta2Tetra`. Since these commands add nodes to the structure, quicker results can be obtained if it is known that none of the new nodes are coincident with existing ones. In a more general manner, this command option is useful if the initial model features coincident but free surfaces (*e.g.* two solids non connected by topology, when using coupling matrices). The default behavior will add only one node for both surfaces thus coupling them, while the `KnownNew` alternative will add one for each.

```
% create 2x3 quad4
femesh('Reset'); model=femesh('Testquad4');
model=feutil('Divide 2 3',model);
model=feutil('Quad2Tria',model); % conversion
feplot(model)
% create a quad, transform to triangles, divide each triangle in 4
femesh('Reset'); model=femesh('Testquad4');
model=feutil('Quad2Tria',model);
model=feutil('Divide2',model);
cf=feplot(model); cf.model
% create a hexa8 and transform to hexa20
femesh('Reset'); model=femesh('Testhexa8');
model=feutil('Lin2Quad eps1 .01',model);
feutil('InfoElt',model)
```

`RefineBeam l`, `RefineToQuad`

The `RefineBeam` command searches `model.Elt` for beam elements and divides elements so that no element is longer than `l`.

```
femesh('Reset'); model=femesh('Testbeam1'); % create a beam
model=feutil('RefineBeam 0.1',model);
```

The `RefineToQuad` command transforms first order triangles, quadrangles, penta, tetra, and hexa to quad and hexa only while dividing each element each in two. The result is a conform mesh.

```
mdl=femesh('testtetra4');mdl=feutil('RefineToQuad',mdl);
feplot(mdl);
```

RemoveElt *ElementSelectors*

```
[model.Elt,RemovedElt]=feutil('RemoveElt ElementSelectors',model);
```

Element removal. This function searches `model.Elt` for elements which verify certain properties selected by *ElementSelectors* and removes these elements from the model description matrix. 2nd output argument `RemovedElt` is optional and contains removed elements. A sample call would be

```
% create 3x2 quad4
femesh('Reset'); model=femesh('Testquad4');model=feutil('Divide 2 3',model);
[model.Elt,RemovedElt]=feutil('RemoveElt WithNode 1',model);
feplot(model)
```

Remove [Pro, Mat] *MatId, ProId*

Mat, Pro removal This function takes in argument the ID of a material or integration property and removes the corresponding entries in the model `pl/il` fields and in the stack `mat/pro` entries.

- Command option `-all` removes all `pl/il` entries found in the model and its stack.
- Command option `-unused` removes all `pl/il` entries not used by any element.

Sample calls are provided in the following to illustrate the use.

```
model=femesh('testhexa8');
model=stack_set(model,'pro','integ',p_solid('default'));
model=stack_set(model,'mat','steel',m_elastic('default steel'));
model=feutil('remove pro 110',model);
model=feutil('remove pro',model,111);
model=feutil('remove mat 100',model);
model=feutil('remove mat 100 pro 1',model);
model=feutil('remove pro -all',model); % Command option -all
model=feutil('remove mat pro -all',model);
model=femesh('testhexa8'); % Command option -unused
model=feutil('remove mat pro -unused',model);
```

Renumber

`model=futil('Renumber',model,NewNodeNumbers)` can be used to change the node numbers in the model. Currently nodes, elements, DOFs and deformations, nodeset, par, cyclic and other Case entries are renumbered.

`NewNodeNumbers` is the total new NodeIds vector. `NewNodeNumbers` can also be a scalar and then defines a global NodeId shifting. If `NewNodeNumbers` has two columns, first giving old NodeIds and second new NodeIds, a selective node renumbering is performed.

If `NewNodeNumbers` is not provided values `1:size(model.Node,1)` are used. This command can be used to meet the OpenFEM requirement that node numbers be less than $2^{31}/100$. Another application is to joint disjoint models with coincident nodes using

Command option `-NoOri` asks not to add the `info,OrigNumbering` data in the model stack. `info,OrigNumbering` is only useful when the user needs to convert something specific linked to the new node numerotation that is outside model.

```
[r1,i2]=futil('AddNode',model.Node,model.Node);
model=futil('Renumber',model,r1(i2,1));
```

RepeatSel *nITE tx ty tz*

Element group translation/duplication. `RepeatSel` repeats the elements of input `model` *nITE* times with global axis translations *tx ty tz* between each repetition of the group. If needed, new nodes are added to `model.Node`. An example is treated in the `d.truss` demo.

```
femesh('Reset'); model=femesh('Testquad4');
model=futil('Divide 2 3',model);
model=futil('RepeatSel 3 2 0 0',model); % 3 repetitions, tx=2
feplot(model)
% an alternate call would be
%                                     number, direction
% model=futil(sprintf('Repeatsel %f %f %f %f', 3, [2 0 0]))
```

Rev *nDiv OrigID Ang nx ny nz*

Revolution. The elements of `model` are taken to be the first meridian. Other meridians are created by rotating around an axis passing trough the node of number *OrigID* (or the origin of the global coordinate system) and of direction [*nx ny nz*] (the default is the *z* axis [0 0 1]). *nDiv*+1 (for closed circle cases `ang=360`, the

first and last are the same) meridians are distributed on a sector of angular width *Ang* (in degrees). Meridians are linked by elements in a fashion similar to extrusion. Elements with a `mass1` parent are extruded into beams, element with a `beam1` parent are extruded into `quad4` elements, `quad4` are extruded into `hexa8`, and `quadb` are extruded into `hexa20`.

The origin can also be specified by the `x y z` values preceded by an `o` using a command like `model=feutil('Rev 10 o 1.0 0.0 0.0 360 1 0 0')`.

You can obtain an uneven distribution of angles using a second argument. For example `model=feutil('Rev 0 101 40 0 0 1',model,[0 .25 .5 1])` will rotate around an axis passing by node 101 in direction *z* and place meridians at angles 0 10 20 and 40 degrees.

```
model=struct('Node',[1 0 0 0 .2 0 0; 2 0 0 0 .5 1 0; ...
                  3 0 0 0 .5 1.5 0; 4 0 0 0 .3 2 0]','Elt',[]);
model.Elt=feutil('ObjectBeamLine',1:4);
model=feutil('Divide 3',model);
model=feutil('Rev 40 o 0 0 0 360 0 1 0',model);
feplot(model)
fecom('; triax; view 3; showpatch')
% An alternate calling format would be
%      divi origin angle direct
% r1 = [40 0 0 0 360 0 1 0];
% model=feutil(sprintf('Rev %f o %f %f %f %f %f %f %f',r1))
```

RotateNode *OrigID Ang nx ny nz*

Rotation. The nodes of `model` are rotated by the angle *Ang* (degrees) around an axis passing through the node of number *OrigID* (or the origin of the global coordinate system) and of direction [*nx ny nz*] (the default is the *z* axis [0 0 1]). The origin can also be specified by the `x y z` values preceded by an `o` `model=feutil('RotateNode o 2.0 2.0 2.0 90 1 0 0',model)` One can define as a second argument a list of NodeId or a FindNode string command to apply rotation on a selected set of nodes. `model=feutil('RotateNode o 2.0 2.0 2.0 90 1 0 0',model,'x==1')`

For example:

```
femesh('reset'); model=femesh('Testquad4'); model=feutil('Divide 2 3',model);
% center is node 1, angle 30, around axis z
%
%              Center angle  dir
st=sprintf('RotateNode %f %f %f %f %f',[1      30  0 0 1]);
```

```

model=feutil(st,model);
feplot(model); fecom(';triax;textnode'); axis on

```

SelElt *ElementSelectors*

```

elt=feutil('SelElt ElementSelectors',model)

```

Element selection. **SelElt** extract selected element from **model** that verify certain conditions. Available element selection commands are described under the **FindElt** command and section 7.12.

SetGroup [*i,name*] [Mat *j*, Pro *k*, EGID *e*, Name *s*]

Set properties of a group. For group(s) selected by number *i*, name *name*, or **all** you can modify the material property identifier *j*, the element property identifier *k* of all elements and/or the element group identifier *e* or name *s*. For example

```

model.Elt=feutil('SetGroup1:3 Pro 4',model);
model.Elt=feutil('SetGroup rigid Name celas',model)

```

If you know the column of a set of element rows that you want to modify, calls of the form `model.Elt(feutil('FindEltSelectors',model),Column)= Value` can also be used.

```

femesh('Reset'); model=femesh('Testubeamplot');
model.Elt(feutil('FindElt WithNode{x==-.5}',model),9)=2;
cf=feplot(model);
cf.sel={'groupall','colordatamat'};

```

SetPro

Set an integration property data (ProId). You can modify an **il** property of ID *i* by giving its name and its value using an integrated call of the type

```

model=femesh('testhexa8');model.il
model=feutil('SetPro 111 Integ=2',model);model.il

```

The names related to the integration properties are documented in the **p_functions**, **p_solid**, **p_shell**, **p_beam**, ... To get a type use calls of the form `p_pbeam('PropertyUnit`

The command can also be used to define additional property information (`.MAP` for field at nodes, `.gstate` for field at integration points and `.NLdata` for non linear behavior data.

StringDOF

`feutil('stringdof',sdof)` returns a cell array with cells containing string descriptions of the DOFs in `sdof`.

SymSel *OrigID nx ny nz*

Plane symmetry. `SymSel` replaces elements in `FEe10` by elements symmetric with respect to a plane going through the node of number `OrigID` (node 0 is taken to be the origin of the global coordinate system) and normal to the vector `[nx ny nz]`. If needed, new nodes are added to `FEnode`. Related commands are `TransSel`, `RotateSel` and `RepeatSel`.

Trace2Elt

```
elt=feutil('Trace2Elt',ldraw);
```

Convert the `ldraw` trace line matrix (see [ufread 82](#) for format details) to element matrix with `beam1` elements. For example:

```
TEST.Node=[1001 0 0 0 0 0 0 ; 1003 0 0 0 0.2 0 0 ;
           1007 0 0 0 0.6 0 0 ; 1009 0 0 0 0.8 0 0 ;
           1015 0 0 0 0 0.2 0 ; 1016 0 0 0 0.2 0.2 0;
           1018 0 0 0 0.6 0.2 0; 1019 0 0 0 0.8 0.2 0];
L=[1001 1003 1007 1009];
ldraw(1,[1 82+[1:length(L)]]]=[length(L) L];
L=[1015 1016 1018 1019];
ldraw(2,[1 82+[1:length(L)]]]=[length(L) L];
L=[1015 1001 0 1016 1003 0 1018 1007 0 1019 1009 0];
ldraw(3,[1 82+[1:length(L)]]]=[length(L) L];
TEST.Elt=feutil('Trace2Elt',ldraw);
cf=feplot(TEST)
```

TransSel *tx ty tz*

Translation of the selected element groups. `TransSel` replaces elements by their translation of a vector `[tx ty tz]` (in global coordinates). If needed, new nodes are added. Related commands are `SymSel`, `RotateSel` and `RepeatSel`.

```
femesh('Reset'); model=femesh('Testquad4'); model=feutil('Divide 2 3',model);
model=feutil('TransSel 3 1 0',model); % Translation of [3 1 0]
feplot(model); fecom(';triax;textnode')
```


Please, note that this command is usefull to translate only part of a model. If the full model must be translated, use `basis` command `gnode`. An example is given below.

```
femesh('Reset'); model=femesh('Testquad4'); model=feutil('Divide 2 3',model)
model.Node=basis('gnode','tx=3;ty=1;tz=0;',model.Node);
feplot(model); fecom(';triax;textnode')
```

UnJoin *Gp1 Gp2*

Duplicate nodes which are common to two groups. To allow the creation of interfaces with partial coupling of nodal degrees of freedom, `UnJoin` determines which nodes are common to the element groups `Gp1` and `Gp2` of `model.Elt`, duplicates them and changes the node numbers in `Gp2` to correspond to the duplicate set of nodes. In the following call with 2 output arguments (2nd is optionnal), the columns of the matrix `InterNode` give the numbers of the interface nodes in each group `[model,InterNode]=feutil('UnJoin 1 2',model)`.

```
femesh('Reset'); model=femesh('Test2bay');
feutil('FindNode group1 & group2',model) % nodes 3 4 are common
model=feutil('UnJoin 1 2',model);
feutil('FindNode group1 & group2',model) % no longer any common node
```

A more general call allows to separate nodes that are common to two sets of elements `[model,InterNode]=feutil('UnJoin',model,'Selection1','Selection2')`. Elements in `Selection1` are left unchanged while nodes in `Selection2` that are also in `Selection1` are duplicated.

See also

`feutila`, `fe_mk`, `fecom`, `feplot`, section 4.2, demos `gartfe`, `d_ubeam`, `beambar` ...

feutilb, fe_caseg

Purpose Gateway functions for advanced FEM utilities in SDT.

Description This function is only used for internal SDT operation and actual implementation will vary over time. The following commands are documented to allow user calls and SDT source code understanding.

Assemble

Optimized strategies for assembly are provided in SDT through the `Assemble` command. More details are given in section 4.5.8.

AddNode

This command provides optimized operation when compared to the `feutil` equivalent and finer control.

CombineModel

```
mo1=feutilb('combinemodel',mo1,mo2);  
[mo1,r1]=feutilb('combinemodel',mo1,mo2);
```

Integrated combining of two separate models. This call aims at creating an assembly from two separate mechanical components. This command properly handles potential `NodeId`, `EltId`, `ProId`, or `MatId` overlaying by setting disjoint ID sets before assembly. Stack or Case entries with overlaying names are resolved, adding (1) to common names in the second model. Sets with identical names between both models are concatenated into a single set. The original numbering matrix for `mo2` is output as a second argument (`r1` in the second example call).

`mo1` is taken as the reference to which `mo2` will be added, the `Node/Elt` appending is performed by `feutilAddTest`.

- Command option `-cleanMP` cleans up duplicated mat/pro entries in the combined model.
- Command option `-noSetCat`, forces the sets duplication with incremented names (adding (1)), instead of concatenation when sets with identical names are found.

- Command option `CompatNodeElt` asks not to shift `NodeId` and `EltId` in the second model. It then assumes the ID ranges are fully compatible in both models.
- Command option `CompatMatPro` asks not to shift `MatId` and `ProId` in the second model. It then assumes these IDs to be fully compatible between both models.
- Command option `CompatBas` asks no to shift the `BasId` in the second model. It then assumes these IDs to be fully compatible between both models.

dTKT

`K = feutilb('dtkt',T,K)` functional equivalent to `diag(T'*k*T)` but this call supports out of core and other optimized operations obtained through compiled functionalities. `K` may be a cell array of matrices, in which case one operates on each cell of the array.

GeomRB, ...

`def=feutilb('geomrb',node,RefXYZ,adof,m)` returns a geometric rigid body modes. If a mass matrix consistent with `adof` is provided the total mass, position of the center of gravity and inertia matrix at CG is computed. You can use `def=feutilb('geomrb cg',Up)` to force computation of rigid body mass properties.

`def=feutilb('geomrbMass',model)` returns the rigid body modes and mass, center of gravity and inertia matrix information.

`il=feutilb('GeomRBBeam1',mdl,RefXYZ)` returns standard `p_beam` properties for a given model section where `RefXYZ` is the coordinates of the reference point from the gravity center.

Match

Non conform mesh matching utilities. The objective is to return matching elements and local coordinates for a list of nodes.

Matching elements mean

- for volumes, that the physical node is within the element. If volumes may be negative, check orientation using `feutil orient`.

- for surfaces, that that the orthogonal projection of the node is within the element
- for lines that the orthogonal projection on the line is between the line extremities.

A typical node matching call would be

```
model=femesh('test hexa8');
match=struct('Node',[.1 .1 .1;.5 .5 .5;1 1 1]);
match=feutilb('match -info radius .9 tol 1e-8',model,match)
```

Accepted command options are

- `MatchSurf` has the same objective but uses a completely different strategy to match nodes on a surface. This is typically well suited for contact applications.
- `radiusrad`. The search is limited to points that are not too far a way from matchable element centers. Defining a search radius manually can help prevent matching for elements that are too far away or on the contrary allow matching within elements that are very large so that interior points may be far from the center.
- `tolval` modifies the `1e-8` tolerance used to stop the non-linear search for the match point in second order elements

The output structure contains the fields

```
.Node      original positions
.rstj      position in element coordinates and jacobian information.
.StickNode orthogonal projection on element surface if the original node is not
           within the element, otherwise original position.
.Info      one row per matched node/element giving NodeId if exact match, num-
           ber of nodes per element, and element type.
.match     obtained when calling the command with -info, typically for row by
           row post-processing of the match. A cell array with one row per matched
           node/element giving elname, slave element row, rstj, sticknode
```

MpcFromMatch

This command is used to build multiple point constraints from a match.

```
feutilb('MpcFromMatch',model,match).
```

The solution retained for surfaces is to first project the arbitrarily located connection point P on the element surface onto a point Q on the neutral fiber used where element nodes are located. Then $Q1$ or $P1$ shape functions and their derivatives are used to define a linear relation between the 6 degree of freedom of point Q and the 3 or 4 nodes of the facing surface. Motion at P is then deduced using a linearized rigid PQ link. One chooses to ignore rotations at the nodes since their use is very dependent on the shell element formulation.

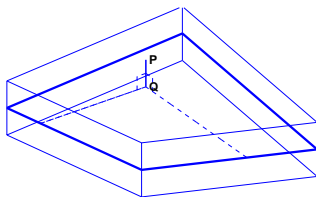


Figure 9.2: Non conform mesh handling

The local element coordinates are defined by $x_j^e, j = 1 : 3$ along the r coordinate line

$$x_j^e = \alpha_x \frac{\partial N_i}{\partial r} x_{ij} \text{ with } \alpha_x = 1 / \left\| \frac{\partial N_i}{\partial r} x_{ij} \right\| \quad (9.1)$$

y^e that is orthogonal to x^e and in the $x^e, \frac{\partial N_i}{\partial s} x_{ij}$ plane, and z^e that defines an orthonormal basis.

The local rotations at point Q are estimated from rotations at the corner nodes using

$$R_j = x_j^e \frac{\partial N_i}{\partial y^e} u_{ik} z_k^e - y_j^e \frac{\partial N_i}{\partial x^e} u_{ik} z_k^e + \frac{1}{2} z_j^e \left(\frac{\partial N_i}{\partial x^e} u_{ik} y_k^e - \frac{\partial N_i}{\partial y^e} u_{ik} x_k^e \right) \quad (9.2)$$

with u_{ik} the translation at element nodes and $j = 1 : 3, i = 1 : N_{node}, k = 1 : 3$. Displacement at Q is interpolated simply from shape functions, displacement at P is obtained by considering that the segment QP is rigid.

For volumes, displacement is interpolated using shape functions while rotations are obtained by averaging displacement gradients in orthogonal directions

$$\begin{aligned} \text{theta}_x &= (-N_{y,z} + N_{z,y}) / 2 \{u\} \\ \text{theta}_y &= (N_{x,z} - N_{z,x}) / 2 \{u\} \\ \text{theta}_w &= (-N_{x,y} + N_{y,x}) / 2 \{u\} \end{aligned} \quad (9.3)$$

You can check that the constraints generated do not constrain rigid body motion using `fe_caseg('rbcheck',model)` which builds the transformation associated to linear constraints and returns a list of DOFs where geometric rigid body modes do not coincide with the transformation.

PlaceInDof

`def2 = feutilb('PlaceInDof',DOF,def)` returns a structure with identical fields but with shapes ordered using the specified `DOF`. This is used to eliminate DOFs, add zeros for unused DOFs or simply reorder DOFs. See also `fe_def SubDof`.

StressCut

The `StressCut` command is the gateway for dynamic stress observation commands. Typical steps of this command are

- View mesh generation, see section 4.4.1.
- Generate a selection `sel=fe_caseg('stresscut -selout',VIEW,model);`
- Display the selection in `feplot` using `fe_caseg('stresscut',sel,cf)`
- Observe the result using `curve=fe_caseg('StressObserve',cf.sel(2),def)`

For the selection generation, accepted options are

- `VIEW` can be a mesh so that `feutilb Match` is used find elements associated with viewing positions. A structure `struct('type','Gauss')` to return selection at Gauss points. A structure `struct('type','BeamGauss')` to return selection at beam Gauss points.
- a `model` or `feplot` handle `cf` can be provided as third argument.
- `-SelOut` requires selection output.
- `-Radiusval` provides a search radius for the `feutilb Match` call.

The `sel` data structure is a standard selection (see `feplot sel`) with additional field `.StressObs` a structure with the following fields

- `.cta` observation matrix for stress components. The expected sort is to have all components at first node, all at second node, ...
- `.DOF` expected DOF needed for the observation.
- `.X`, `.Xlab` labels for the observation, see `Multi-dim curve` for details.

- `.CritFcn` callback to be evaluated, see `fe_stress CritFcn`.
- `.Node`, `.Elt` nodes and elements for the view mesh.
- `.trans` structure for the observation of interpolated displacement (needed when view mesh nodes are not nodes of the original mesh).

StressObserve

The `StressCut` command typically returns all stress components (x , y , and z), for a relevant plot, it is useful to define a further post-treatment, using the `sel.StressObs.CritFcn` callback. This callback is called once the stress observation have been performed. The current result is stored in variable `r1`, and follows the dimensions declared in field `.X` of the observation. For example to extract stresses in the x direction, the callback is

```
sel.StressObs.CritFcn='r1=r1(1,:,:);';
```

The `StressObserve` command outputs the stress observation in an `curve` structure. You can provide a callback `-crit "my_callback"`. If empty, all components are kept.

```
data=fe_caseg('StressObserve -crit"',cf.sel(2),def);  
iipplot(data); % plot results
```

TKT

`K = feutilb('tkt',T,K)` functional equivalent to `T'*k*T` but this call supports out of core and other optimized operations obtained through compiled functionalities. `K` may be a cell array of matrices, in which case one operates on each cell of the array.

Write

`feutilb('WriteFileName.m',model)` writes a clean output of a model to a script. Without a file name, the script is shown in the command window.

`feutilb('_writeil',model)` writes properties. `feutilb('_writepl',model)` writes materials.

ZoomClip

The command accessible through the axes context menu `Clip`, can now also be called from the command line `fe_caseg('ZoomClip',cf.ga,[xyz_left;xyz_right])`.

feplot

Purpose Gateway function for 3-D visualization of structures. See also the companion function `fecom`.

Syntax

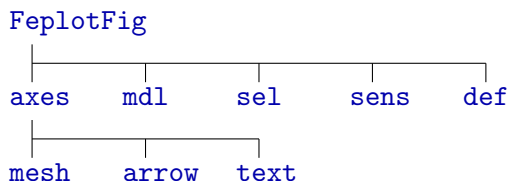
```
feplot
feplot(FigHandle)
feplot(model)
feplot(model,def)
```

Description `fecom` gives a complete list of commands. The rest of this section gives more details on the `feplot` architecture. For a tutorial see section 4.1. Basic ways to call `feplot` are

- `feplot` refreshes all `feplot` axes of the current figure.
- `cf=feplot` returns a *SDT handle* to the current `feplot` figure. You can create more than one `feplot` figure with `cf=feplot(FigHandle)`.
- `cf=feplot(model)` or `cf.model=model` initializes the FE model displayed in the current figure (see `fecom load`). `cf=feplot(model,def)` initializes the FE model and a deformation set.
- `cf.def=def` and `cf.def(i)=def` initializes a deformation set. For more details (in particular display of test shapes), see `fecom InitDef`.
- `cf.Stack` and `cf.CStack` calls are detailed in section 4.1.2.

The old formats `feplot(node,elt,mode,mdof,2)` and `cf.model={Node,Elt}` are still supported but you are encouraged to switch to the new and more general procedure outlined above.

Views of deformed structures are obtained by combining information from various data arrays that can be initialized/modified at any time. The object hierarchy is outlined below with the first row being data arrays that store information and the second row objects that are really displayed in MATLAB `axes`.



axes describe axes to be displayed within the **feplot** figure. Division of the figure into subplots (MATLAB axes) is obtained using the **fecom Sub** commands. Within each plot, basic displays (wire mesh, surface, sensor, arrow corresponding to **mesh**, **arrow**, or **text** objects) can be obtained using the **fecom Show** commands while more elaborate plots are obtained using **fecom SetObject** commands. Other axes properties (rotations, animation, deformation selection, scaling, title generation, etc.) can then be modified using **fecom** commands.

mdl *Model data structure* (see section 7.6) **cf.mdl** is a handle to the model contained in the **feplot** figure. The model must be defined before any plot is possible. It is initialized using the **fecom InitModel** command or using the method **cf.model**.

Stack *Model Stack entries* are stored in **cf.mdl.Stack**, but can be more easily reached using **cf.Stack{i}** or **cf.Stack{EntryName}** or modified using **cf.Stack{EntryType,EntryName}={EntryType,EntryName,EntryData}**.

CStack *Case Stack entries* are stored in the stack case (itself stored in **cf.mdl.Stack**). They can be more easily reached using **cf.CStack{i}** or **cf.CStack{EntryName}** or modified using **cf.CStack{EntryType,EntryName}={EntryType,EntryName,EntryData}**.

sel *Element selections* describe which elements are displayed. The standard selection displays all elements of all groups. **fecom Sel** commands or **cf.sel(i)** let you define selections that only display some elements. See also the **fecom SetObject** commands. Color information is defined for each selection (see **fecom Color** commands). **cf.sel(i) = 'ElementSel'** initializes a selection to use element selected by *ElementSel*. Note that you may want to declare color data simultaneously using **cf.sel(i) = {'ElementSel', 'Colordata Command', Args}**. **cf.o(i) = {'ObjectSpec', 'PatchProperty', PatchValue}** modifies the properties of object *i* in the current **feplot** axis.

sens (obsolete) *sensor selections* describe sets of sensors. Sensor selections are used to display the response at measurement locations through stick or arrows. Initialized using the `InitSens` command or `cf.sens(i)` calls (see `fecom`). `cf.sens(i)={sdof}` initializes a sensor set (see `fecom InitSens`).

def *deformation sets* describe deformations at a number of DOFs. Initialized using the `InitDef` command or `cf.def(i)` calls (see `fecom`). `cf.def(i)={def,dof}` is also accepted. `cf.def(i)={def,dof,freq}` where `freq` is a list of frequencies of poles automatically generates title labels for each deformation (see `fecom InitDef`).

Objects

mesh

mesh objects represent a deformed or undeformed finite element mesh. They are used both for wire-frame and surface representations. **mesh** objects are characterized by indices giving the element selection, deformation set, channel (deformation number), and color type. They can be modified using calls or the form

```
cf = feplot; % get sdth object handle
cf.o(2) = 'sel 1 def 1 ch 3'
```

or equivalently with `fecom SetObject` commands. `fecom Show` commands reset the object list of the current axis.

Each **mesh** object is associated to up to three MATLAB **patch** objects associated respectively with real surfaces, segments and isolated nodes. You can access individual pointers to the **patch** objects using `cf.o(i,j)` (see `fecom go` commands).

arrow

Arrow objects are used to represent sensors, actuators, boundary conditions, ... They are characterized by indices giving their sensor set, deformation set, channel (deformation number), and arrow type. They can be modified using calls or the form (see `fecom SetObject` commands)

```
cf = feplot; % get sdth object handle
cf.o(2) = 'sen 1 def 1 ch 3'
```

The *SDT* currently supports stick sensors (object type 3) and arrows at the sensor tip (type 7). Other arrow types will eventually be supported.

text

`fecom` text objects are vectorized lists of labels corresponding to nodes, elements, DOFs, ... They can be initialized using `fecom Text` commands and deleted with `textoff`. You can use `cf.o(i)` (see `fecom go` commands) to get handles to the associated MATLAB text objects and thus set font name size, ... `set(cf.o(1), 'fontsize', 7)` for example.

Data arrays

`feplot` stores information in various data arrays `cf.mdl` for the model, `cf.def(i)` for the definition of deformations, `cf.sel(i)` for element selections for display and `cf.sens(i)` for sensor selections.

mdl

The model currently displayed is stored in `cf.mdl`, see `fecom InitModel`.

def

The deformations currently displayed are stored in `cf.def`, see `fecom InitDef` for accepted input formats.

sel

element selections describe a selection of elements to be displayed. The standard selection displays all elements of all groups. `fecom Sel` commands let you define selections that only display some elements.

<code>.selelt</code>	string used for element selection
<code>.vert0</code>	position of vertices (nodes) in the undeformed configuration
<code>.node</code>	node numbers associated to the various vertices
<code>.cna</code>	array (as many as currently declared deformations) of sparse observation matrices giving the linear relation between deformation DOFs and translation DOFs at the selection nodes. The observation matrix gives all x translations followed by all y translations and all z translations.
<code>.fs</code>	face definitions for true surfaces (elements that are not represented by lines or points). <code>.ifs</code> gives the element indices (possibly repeated if multiple faces)
<code>.f2</code>	face definitions for lines (if any). <code>.if2</code> gives the element indices (possibly repeated if multiple faces).
<code>.f1</code>	face definitions for points (if any).
<code>.fvcs</code>	FaceVertexCData for true surfaces (see <code>fecom ColorData</code> commands). Can also be a string, which is then evaluated to obtain the color, or a function handle used in <code>ColorAnimFcn</code> .
<code>.fvc2</code>	FaceVertexCData for lines
<code>.fvc1</code>	FaceVertexCData for points

`sens`

sensor selections describe sets of sensors. Sensor selections are used to display the response at measurement locations through stick or arrows. The `InitSens` command is being replaced by the definition of `SensDof` stack entries.

<code>.vert0</code>	position of vertices (nodes) in the undeformed configuration
<code>.node</code>	node numbers associated to the various vertices
<code>.ntag</code>	numerical tag identifying each sensor
<code>.dir</code>	direction associated with each sensor
<code>.cta</code>	array (as many as currently declared deformations) of sparse observation matrices giving the linear relation between deformation DOFs and measurements.
<code>.opt</code>	[Created]
<code>.arrow</code>	defines how the arrow is related to the measurement

See also `fecom`, `femesh`, `feutil`, tutorial in section 4.1

fesuper

Purpose User interface for superelement support.

Syntax

```
                fesuper('CommandString')
[out,out1] = fesuper('CommandString', ...)
model      = fesuper(model,'CommandString', ... )
```

Description Superelements (see section 6.3 for more details) should be declared as **SE** entries in `model.Stack`. When using this format, you should specify `model` as the first argument `fesuper` so that any modification to the superelement is returned in the modified stack. Superelement should be short lower case names to allow name encoding.

F ...

Get full model from superelement model.

```
SE=demosdt('demo ubeam'); SE=SE.GetData; % Load full model.
model=fesuper('SESelAsSe',[],SE); % Build SE model.
Node=fesuper('FNode',model); % Get full model nodes.
Elt=fesuper('FElt',model); % Get full model elements.
mfull=fesuper('FSEModel',model); % Get full model.
```

Get,Set ...

Get,set properties from a superelement. Standard superelement fields are detailed in section 6.3.2. `get` and `set` commands are obsolete, you should really use direct access to the `feplot` stack. For example

```
cf=demosdt('demo cmsSE feplot');
SE1=cf.Stack{'se1'};
SE1=stack_set(SE1,'info','EigOpt',[5 10.1 1e3]);
SE1=fe_reduc('CraigBampton -SE -UseDof',SE1);
cf.Stack{'se1'}=SE1; fecom('curtabStack','SE:se1')
```

A new command to perform reduction is under development.

`mdl=fesuper(mdl,'setTR',name,'fe_reduc command')` calls `fe_reduc` to assemble and reduce the superelement. The command option `-drill` can be added

to the `fe_reduc` command to consider drilling stiffness in shells. For example `mdl=fesuper(mdl, 'SetTR', 'SE1', 'CraigBampton -UseDof -drill');`
The modes to be kept in the superelement can be set using `mdl=fesuper(mdl, 'setStack', name, 'info', 'EigOpt', EigOptions);`

Damp

`model=fesuper('Damp',model,'SEname',damp);` Defines a modal damping on the superelement `SEname`. `damp` can be a scalar `zeta0` and defines a global damping ratio on all computed modes. `damp` can also be a vector `[zeta0 f0 zeta1]` defining a first damping ratio `zeta0` for frequencies lower than `f0` Hz and another damping ratio `zeta1` for higher frequencies. Note that all modes are computed.

SEDef

Superelement restitutions. These commands are used to handle model partial or full restitutions for visualization and recovery handling.

`SEDefInit` is used to prepare the model for restitution matters. It adds in `model.Stack` an entry `info,SeRestit` containing the necessary data for restitution *i.e.* to perform $\{q\} = [T]\{q_R\}$. This aims to limit generic work needed for multiple restitutions. Syntax is `model=fesuper('SEDefInit',model)`.

`SEDef` is used to implement restitution on full model DOFs. Syntax is `dfull=fesuper('SeDef', cf, def)`

SEBuildSel

`SEBuildSel` is used to perform partial restitution on a model. This command sets `feplot` to display a restitution mesh and computes the corresponding deformation vectors. The restitution selection is defined as a cell array with rows of the form `SeName,EltSel` for selection of each superelement. An `EltSel` entry set to `'groupall'` thus displays the full superelement. `EltSel` can also be an element matrix (useful to display deformations on a test frame) or even a vector of NodeIds. To discard a superelement from display, use an empty string for `EltSel`. By default a superelement not mentioned in the selection is displayed.

After the generation of superelement selections, it is possible to set a global selection on the full mesh by adding an entry with an empty superelement name (see illustration below).

Accepted command options are

- `-nojoin` avoids grouping elements of the same topology in a single group.
- `-LinFace` can be used to generate selections that only use first order faces (`tria3` instead of `tria6`, ...)
- `-NoOptim` is used to skip the restitution optimization phase.
- `-cGL` (used in SDT/Rotor) is used in cases with local bases associated with each superelement. In this case, `data.cGL` is a cell array used to define a local rotation associated with each superelement. Typically, this is equal to `data.cGL{jEt}=reshape mdl.bas(j1,7:15),3,3);`.
- `-RotDof` (used in SDT/Rotor) large angle DOF

The following example is based on a gimbal model reduced in three superelements: `base`, `gimbal` and `tele`. A partial restitution is proposed.

```
model=demosdt('demogimbal-reduce')
cf=feplot(model)
def=fe_eig(model,[5 10 1e3 0 1e-5]);

Sel={'gimbal' 'groupall';
     'tele' 'InNode{z>=0}';
     'base' '' }; % base not displayed
fesuper('SEBuildSel',cf,Sel);
cf.def=def;

% Second selection example
Sel={'gimbal' 'groupall';
     'tele' '';
     'base' 'groupall'
     '', 'InNode{z>=0}'}; % global selection
fesuper('SEBuildSel',cf,Sel);
```

If you have previously initialized a full restitution with `fesuper('SeDefInit',cf)`, data to optimize partial restitution will be initialized. To obtain a partial restitution of a set of vectors, use `data=cf.sel.cna1;dfull=fesuper('sedef',data,dred)`.

SE ...

`SEDoF` is an internal command used to implement proper responses to `feutil GetDof` commands. It is assumed that the superelement `.DOF` field is defined prior to setting the information in the `model.Stack`.

`SEMPC` is an internal command that need to be documented.

`SECon` may also need some documentation.

SEAdd ...

`SEAdd SENAME` commands are used to append superelements to a model. With no command option `fesuper('SEAdd name',model,SE,[matId proId])` appends a new superelement to the `model.Elt` field (creates a group `SE` if necessary) and saves the provided `SE` as a stack entry. `[matId proId]` can be given as a last argument to define properties associated to added superelement.

`SE` is usually a standard SDT model, with fields `.Node`, `.Elt`, `.Stack...` But this command accepts models defined only from element matrices (needs `.K`, `.Opt` and `.DOF` fields). It can be useful to cleanly import element matrices from other codes for example (see section 4.1.9).

`SEAdd -unique NodeId0 EltId0 SENAME` is used to add a single superelement and to give its ranges of implicit nodes and elements. `NodeId0` is the lower bound of the range of the superelement implicit nodes (use 1 for no shift). `NodeIdEnd` is given by `NodeIdEnd-NodeId0=max(SE.Node(:,1))`. `EltId0` is the lower bound of the range of the superelement elements. The `EltId` range width is equal to the maximum `EltId` of the superelement.

`SEAdd -trans nrep tx ty tz <NodeShift> NodeId0 EltId0 SENAME` is used to repeat the model with a given translation step (`nrep`). `NodeId0` is the lower bound of the range of the first superelement implicit nodes. The range width is equal to the maximum `NodeId` of the superelement. The ranges of implicit nodes for repeated superelements are translated so that there is no overlap excepted if `NodeShift` argument is given (then there is a `NodeId` range overlap of `NodeShift` nodes. It can be useful if the repeated superelement intersections are not void: then `NodeShift` can be the number of intersection nodes between 2 superelements). `EltId0` is the lower bound of the `EltId` range of elements of the first superelement. There is no `EltId` range overlap.

For example

```
model=femesh('testhexa8');
model=feutil('renumber',model,model.Node(:,1)*10);
mo1=fesuper('SEAdd -trans 5 0 0 1 10000 10000 cube',[],model)
```

feplot(mo1)

`SEAdd -disk nodeId0 eltId0 SENAME` is used to repeat a sector model in cyclic symmetry. It is assumed that the `symmetry` case entry exists in the model (see `fe_cyclic Build`).

In all these cases, matrix of nodes of the superelement is sorted by NodeId before it is added to the stack of the model (so that `SE.Node(end,1)==max(SE.Node(:,1))`).

SEAssemble ...

The command `fesuper('SEAssemble',model)` is used to assemble matrices of superelements that are used in `model`. A basis reduction from superelement `Case.T` (Interface `DofSet` is ignored) is performed.

SEDispatch ...

The command `fesuper('SEDispatch',model)` is used to dispatch constraints (`mpc`, `rbe3`, `rigid` elements, ...) of the global model in the related superelements, and create `DofSet` on the interface DOFs.

Rigid elements in `model.Elt` are distributed to the superelements (may be duplicated) that contain the slave node. The master node of the rigid element must be present in the superelement node matrix, even if it is unused by its elements (`SESelAsSE` called with selections automatically adds those nodes to the superelements).

Other constraints (`mpc`, `rbe3`, `FixDof`) are moved to superelement if all constraint DOFs are within the superelement. Constraints that span multiple superelements are not dispatched.

A unit `DofSet` (identity `def` matrix, same DOFs) is defined in superelements that contain all DOFs of the global model `DofSet`.

Finally a `DofSet` (identity `def` matrix) is defined on superelement DOFs that are active in the global model and shared by another superelement. Those `DofSet` are stored in the `'Interface'` entry of each superelement stack.

SEIntNode ...

The command `fesuper('SEIntNode',model)` can be used to define explicitly superelement interface nodes, taking into account local basis.

SESelAsSE ...

Selection as superelement. The command `fesuper('SESelAsSE', model, Sel)` is used to split a model in some superelement models, or to build a model from sub models taken as superelements.

`Sel` can be a FindElt string selector, or a model data structure.

If `Sel` is a FindElt string selector, the elements corresponding to the selection are removed from `model`, and then added as a superelement model. The implicit NodeId of the superelement are the same as the former NodeId in `model`. **Warning:** the selection by element group is not available due to internal renumbering operations performed in this task.

If `Sel` is a model, it is simply added to `model` as a superelement.

`Sel` can also be a cell array of mixed types (FindElt string selector or model data structure): it is the same as calling sequentially a `SESelAsSE` command for each element of the cell array (so avoid using group based selection for example, because after the first selection `model.Elt` may change).

You can give a name to each superelement in the second column of `Sel` `{Selection_or_model,SEname; ...}`. If name is not given (only one column in `Sel`), default `seID` is used.

By default, superelements `Mat/ProId` are generated and incremented from 1001. It is possible to specify the `MatId` and/or `ProId` of the superelements created by adding a third column to `Sel`, with either a scalar value to apply to `MatId` and `ProId` or a line vector under the format `[MatId ProId]`. *E.g.* `Sel={Selection,SEname,[1001 1001];...}`. When the third column is left empty for certain lines, the default behavior is applied for these lines only.

Master nodes of the global model rigid elements are added to the superelements that contain corresponding slave nodes.

Following example divides the `d.cms` model into 2 sub superelement models.

```
mdl=demosdt('demo cms');
model=fesuper('SESelAsSE',mdl, ...
    {'WithNode{x>0|z>0}';'WithNode{x<0|z<0}'});
```

The command option `-dispatch` can be used to dispatch constraints (`rigid` elements, `mpc`, `rbe3` ...) of the global model in the related superelements and create `DofSet` on the interface DOFs. It is the same as calling the `fesuper SEDispatch` command after `SESelAsSE` without command option.

SERemove

`model=fesuper('SERemove',model,'name')` searches superelement `name` in the model and removes it from Stack and element matrix.

SERenumber

`SE=fesuper('renumber',model,'name')` searches superelement `name` in the model stack and rennumbers based on the entry in the `SE` element group. If `name` refers to multiple superelements, you should provide the row number in `model.Elt`.

s_

Superelement name coding operations. `num=fesuper('s_name')` returns the number coding the superelement `name`. `name=fesuper('s_',num)` decodes the number. `elt=fesuper('s_name',model)` extracts elements associated with a given superelement.

See also

[fe_super](#), [upcom](#), section 4.1.9, section 6.3

fe_c

Purpose DOF selection and input/output shape matrix construction.

Syntax

```
c                    = fe_c(mdof, adof)
c                    = fe_c(mdof, adof, cr, ty)
b                    = fe_c(mdof, adof, cr) '
[adof, ind, c] = fe_c(mdof, adof, cr, ty)
ind                  = fe_c(mdof, adof, 'ind', ty)
adof                 = fe_c(mdof, adof, 'dof', ty)
labels              = fe_c(mdof, adof, 'dofs', ty)
```

Description This function is quite central to the flexibility of DOF numbering in the *Toolbox*. FE model matrices are associated to *DOF definition vectors* which allow arbitrary DOF numbering (see section 7.5). `fe_c` provides simplified ways to extract the indices of particular DOFs (see also section 7.10) and to construct input/output matrices. The input arguments for `fe_c` are

`mdof` *DOF definition vector* for the matrices of interest (be careful not to mix DOF definition vectors of different models)

`adof` *active DOF definition vector*.

`cr` *output matrix associated to the active DOFs*. The default for this argument is the identity matrix. `cr` can be replaced by a string `'ind'` or `'dof'` specifying the unique output argument desired then.

`ty` *active/fixed option* tells `fe_c` whether the DOFs in `adof` should be kept (`ty=1` which is the default) or on the contrary deleted (`ty=2`).

The input `adof` can be a standard DOF definition vector but can also contain wild cards as follows

`NodeID.0` means all the DOFs associated to node `NodeID`
 `0.DofID` means `DofID` for all nodes having such a DOF
`-EltID.0` means all the DOFs associated to element `EltID`

The convention that DOFs `.07` to `.12` are the opposite of DOFs `.01` to `.06` is supported by `fe_c`, but this should really only be used for combining experimental and analytical results where some sensors have been positioned in the negative directions.

The output argument `adof` is the actual list of DOFs selected with the input argument. `fe_c` seeks to preserve the order of DOFs specified in the input `adof`. In

particular for models with nodal DOFs only and

- `adof` contains no wild cards: no reordering is performed.
- `adof` contains node numbers: the expanded `adof` shows all DOFs of the different nodes in the order given by the wild cards.

The first use of `fe_c` is the **extraction** of particular DOFs from a DOF definition vector (see `b,c` page 243). One may for example want to restrict a model to 2-D motion in the xy plane (impose a fixed boundary condition). This is achieved as follows

```
[adof,ind] = fe_c(mdof,[0.01;0.02;0.06]);  
mr = m(ind,ind); kr = k(ind,ind);
```

Note `adof=mdof(ind)`. The vector `adof` is the DOF definition vector linked to the new matrices `kr` and `mr`.

Another usual example is to fix the DOFs associated to particular nodes (to achieve a clamped boundary condition). One can for example fix nodes 1 and 2 as follows

```
ind = fe_c(mdof,[1 2], 'ind',2);  
mr = m(ind,ind); kr = k(ind,ind);
```

Displacements that do not correspond to DOFs can be fixed using `fe_coor`.

The second use of `fe_c` is the creation of **input/output shape matrices** (see `b,c` page 152). These matrices contain the position, direction, and scaling information that describe the linear relation between particular applied forces (displacements) and model coordinates. `fe_c` allows their construction without knowledge of the particular order of DOFs used in any model (this information is contained in the DOF definition vector `mdof`). For example the output shape matrix linked to the relative x translation of nodes 2 and 3 is simply constructed using

```
c=fe_c(mdof,[2.01;3.01],[1 -1])
```

For reciprocal systems, input shape matrices are just the transpose of the collocated output shape matrices so that the same function can be used to build point load patterns.

Example

Others examples may be found in `adof` section.

See also

`fe_mk`, `feplot`, `fe_coor`, `fe_load`, `adof`, `nor2ss`
Section 4.2.3

fe_case

Purpose UI function to handle FEM computation cases

Syntax

```
Case = fe_case(Case, 'EntryType', 'Entry Name', Data)
fe_case(model, 'command' ...)
```

Description *FEM computation cases* contains information other than nodes and elements used to describe a FEM computation. Currently supported entries in the case stack are

<code>cyclic</code>	(SDT) used to support cyclic symmetry conditions
<code>DofLoad</code>	loads defined on DOFs (handled by <code>fe_load</code>)
<code>DofSet</code>	(SDT) imposed displacements on DOFs
<code>FixDof</code>	used to eliminated DOFs specified by the stack data
<code>FSurf</code>	surface load defined on element faces (handled by <code>fe_load</code>). This will be phased out since surface load elements associated with volume loads entries are more general.
<code>FVol</code>	volume loads defined on elements (handled by <code>fe_load</code>)
<code>info</code>	used to stored non standard entries
<code>KeepDof</code>	(obsolete) used to eliminated DOFs not specified by the stack data. These entries are less general than <code>FixDof</code> and should be avoided.
<code>map</code>	field of normals at nodes
<code>mpc</code>	multiple point constraints
<code>rbe3</code>	a flavor of MPC that enforce motion of a node a weighted average
<code>par</code>	are used to define physical parameters (see <code>upcom Par</code> commands)
<code>rigid</code>	linear constraints associated with rigid links
<code>SensDof</code>	(SDT) Sensor definitions

`fe_case` is called by the user to initialize (when `Case` is not provided as first argument) or modify cases (`Case` is provided).

Accepted commands are

`Get, Set, Remove, Reset ...`

- `[Case, CaseName]=fe_case(model, 'GetCase')` returns the current case. `GetCasei` returns case number *i* (order in the model stack). `GetCaseName` returns a case with name *Name* and creates it if it does not exist. Note that the Case name cannot start with `Case`.

- `data=fe_case(model,'GetData EntryName')` returns data associated with the case entry *EntryName*.
- `model=fe_case(model,'SetData EntryName',data)` sets data associated with the case entry *EntryName*.
- `[Case,NNode,ModelDOF]=fe_case(model,'GetT')`; returns a congruent transformation matrix which verifies constraints. Details are given in sermpc. `CaseDof=fe_case(model,'GetTDOF')` returns the case DOF (for model DOF use `feutil('getdof',model)`).
- `model=fe_case(model,'Remove','EntryName')` removes the entry with name *EntryName*.
- `Reset` empties all information in the case stored in a model structure `model = fe_case(model,'reset')`
- `SetCurve` has a load reference a curve in model Stack. For example `model=fe_case(model,load 1','input')`; associates `Point load 1` to curve `input`. See section 7.9 for more details.
- `stack_get` applies the command to the case rather than the model. For example `des = fe_case(model,'stack_get','par')`
- `stack_set` applies the command to the case rather than the model. For example `model = fe_case(model,'stack_set','info','Value',1)`
- `stack_rm` applies the command to the case rather than the model. For example `model = fe_case(model,'stack_rm','par')`

Commands for advanced constraint generation

AutoSPC

Analyses the rank of the stiffness matrix at each node and generates a `fixdof` case entry for DOFs found to be singular:

```
model = fe_case(model,'autospc')
```

Assemble

Calls used to assemble the matrices of a model. See `fe_mkn1 Assemble` and section 4.5.8 for optimized assembly strategies.

Build Sec epsl d

`model = fe_cyclic('build (N) epsl (d)',model,LeftNodeSelect)` is used to append a cyclic constraint entry in the current case.

ConnectionEqualDOF

`fe_caseg('Connection EqualDOF',model,'name',DOF1,DOF2)` generates a set of MPC connecting each DOF of the vector *DOF1* to corresponding DOF in *DOF2*. *DOF1* and *DOF2* can be a list of `NodeId`, in that case all corresponding DOF are connected, or only DOF given as a `-dof DOFs` command option.

Following example defines 2 disjointed cubes and connects them with a set of MPC between DOFs of the given nodes:

```
cf1=feplot; cf1.mdl=femesh('testhexa8');
cf1.mdl=feutil('repeatsel 2 0.0 0.0 1.5',cf1.mdl);
cf1.mdl=fe_caseg('Connection EqualDOF -id56',cf1.mdl, ...
    'link1',[5:8],[9:12]);
fecom(cf1,'promodelviewon');fecom(cf1,'curtab Cases','link1');
```

The option `-id i` can be added to the command to specify a MPC ID *i* for export to other software.

ConnectionPivot

This command generates a set of MPC defining a pivot connection between two sets of nodes. It is meant for use with volume or shell models with no common nodes. For beams the pin flags (columns 9:10 of the element row) are typically more appropriate, see `beam1` for more details.

The command specifies the DOFs constraint at the pivot (in the example DOF 6 is free), the local *z* direction and the location of the pivot node. One then gives the model, the connection name, and node selections for the two sets of nodes.

```
model=demosdt('demoTwoPlate');
model=fe_caseg('Connection Pivot 12345 0 0 1 .5 .5 -3 -id 1111', ...
    model,'pivot','group1','group2');
def=fe_eig(model);feplot(model,def)
```

The option `-id i` can be added to the command to specify a MPC ID *i* for export to other software.

ConnectionSurface

This command implements node to surface connections through constraints or penalty. `fe_caseg('ConnectionSurface DOFs',model,'name',NodeSel1,EltSel2)` generates a set of MPC connecting of *DOFs* of a set of nodes selected by `NodeSel1` (this is a node selection string) to a surface selected by `EltSel2` (this is an element selection string). The following example links *x* and *z* translations of two plates

```
model=demosdt('demoTwoPlate');
model=fe_caseg('Connection surface 13 -MaxDist0.1',model,'surface', ...
    'z==0', ... % Selection of nodes to connect
    'withnode {z==.1 & y<0.5 & x<0.5}'); % Selection of elements for matching
def=fe_eig(model);feplot(model,def)
```

Accepted command options are

- `-id i` can be added to the command to specify a MPC ID *i* for export to other software.
- `-Radiusval` can be used to increase the search radius for the `feutilb Match` operation.
- `-MaxDistval` eliminates matched node with distance to the matched point within the element higher than *val*. This is typically useful for matches on surfaces where the node can often be external. Using a `-MaxDist` is required for `-Dof`
- `-kpval` is used to give the stiffness for a penalty based implementation of the constraint. The stiffness matrix of the penalized bilateral connection is stored in a superelement with the constraint name.
- `-Dofval` can be used to build surface connections of non structural DOFs (thermal fields, ...).

It is also possible to define the `ConnectionSurface` implicitly, to let the constraint resolution be performed after full model assembly. The `ConnectionSurface` is then defined as an MPC, which `data` structure features fields `.type` equal to `ConnectionSurface` with possible command options, and field `.sel` giving in a cell array a sequence `{NodeSel1, EltSel2}`, as defined in the explicit definition. The following example presents the implicit `ConnectionSurface` definition equivalent to the above explicit one.

```

model=demosdt('demoTwoPlate');
  model=fe_case(model,'mpc','surface',...
  struct('type','Connection surface 13 -MaxDist0.1',...
  'sel',{{'z==0','withnode {z==.1 & y<0.5 & x<0.5}'}}));
def=fe_eig(model);feplot(model,def)

```

Warning volume matching requires that nodes are within the element. To allow exterior nodes, you should add a `& selface` at the end of the element selection string for matching.

ConnectionScrew

```
fe_caseg('Connection Screw',model,'name',data)
```

This command generates a set of RBE3 defining a screw connection. Nodes to be connected are defined in planes from their distance to the axis of the screw. The connected nodes define a master set enforcing the motion of a node taken on the axis of the screw with a set of RBE3 (plane type 1) or rigid links (plane type 0) ring for each plane.

In the case where rigid links are defined, the command appends a group of `rigid` elements to the model.

Real screw can be represented by beams connecting all the axis slave nodes.

`data` defining the screw is a data structure with following fields:

<code>Origin</code>	a vector [<code>x0 y0 z0</code>] defining the origin of the screw.
<code>axis</code>	a vector [<code>nx ny nz</code>] defining the direction of the screw axis.
<code>radius</code>	defines the radius of the screw.
<code>planes</code>	a matrix with as many lines as link rings. Each row is of the form [<code>z0 type ProId zTol rTol</code>] where <code>z0</code> is the distance to the origin along the axis of the screw, <code>type</code> is the type of the link (0 for rigid and 1 for <code>rbe3</code>), <code>ProId</code> is the ProId of the elements containing nodes to connect, and <code>zTol</code> and <code>rTol</code> are tolerance of the positions of these nodes respectively for distance along the axis and distance to the axis. One can also define more generally planes as a cell array whose each row defines a plane and is of the form <code>{z0 type st}</code> where <code>z0</code> and <code>type</code> are defined above and <code>st</code> is a FindNode string. <code>st</code> can contain <code>\$FieldName</code> tokens that will be replaced by corresponding <code>data.FieldName</code> value (for example <code>'cyl<= \$radius o \$Origin \$axis & inElt{ProId \$ProId}'</code> will select nodes in cylinder of radius <code>data.radius</code> , origin <code>data.Origin</code> and axis <code>data.axis</code> , and in elements of ProId <code>data.ProId</code>).
<code>MatProId</code>	Optional. If present beams are added to connect slave nodes at the center of each link ring. It is a vector [<code>MatId ProId</code>] defining the <code>MatId</code> and the <code>ProId</code> of the beams.
<code>MasterCelas</code>	Optional. It defines the <code>celas</code> element which is added if this field is present. It is of the form [<code>0 0 -DofID1 DofID2 ProID EltID Kv Mv Cv Bv</code>]. The first node of the <code>celas</code> is the slave node of the <code>rbe3</code> ring and the second is added at the same location. This can be useful to reduce a superelement keeping the center of the rings in the interface.
<code>NewNode</code>	Optional. If it is omitted or equal to 1 then a new slave node is added to the model at the centers of the link rings. If it equals to 0, existent model node can be kept.
<code>Nnode</code>	Optional. Gives the number of points to retain in each plane.

Following example creates a test model, and adds 2 `rbe3` rings in 2 planes.

```
model=demosdt('demoscrew layer 0 40 20 3 3 layer 0 40 20 4'); % create model
r1=struct('Origin',[20 10 0],'axis',[0 0 1],'radius',3, ...
         'planes',[1.5 1 111 1 3.1;
                  5.0 1 112 1 4;], ...
         'MasterCelas',[0 0 -123456 123456 10 0 1e14], ...
         'NewNode',0);
model=fe_caseg('ConnectionScrew',model,'screw1',r1);
```

```
cf=feplot(model); % show model
fecom('promodelviewon');fecom('curtab Cases','screw1');
```

Entries

The following paragraphs list available entries not handled by `fe_load` or `upcom`.

cyclic (SDT)

`cyclic` entries are used to define sector edges for cyclic symmetry computations. They are generated using the `fe_cyclic Build` command.

FixDof

`FixDof` entries correspond to rows of the `Case.Stack` cell array giving `{'FixDof', Name, Data}`. `Name` is a string identifying the entry. `data` is a column DOF definition vector (see section 7.10) or a string defining a node selection command. You can also use

```
data=struct('data',DataStringOrDof,'ID',ID) to specify a identifier.
```

You can now add DOF and ID specifications to the `findnode` command. For example `'x==0 -dof 1 2 -ID 101'` fixes DOFs x and y on the `x==0` plane and generates an `data.ID` field equal to 101 (for use in other software).

The following command gives syntax examples. An example is given at the end of the `fe_case` documentation.

```
model = fe_case(model,'FixDof','clamped dofs','z==0', ...
    'FixDof','SimpleSupport','x==1 & y==1 -DOF 3', ...
    'FixDof','DofList',[1.01;2.01;2.02], ...
    'FixDof','AllDofAtNode',[5;6], ...
    'FixDof','DofAtAllNode',[.05]);
```

map

`map` entries are used to define maps for normals at nodes. These entries are typically used by shell elements or by meshing tools. `Data` is a structure with fields

- `.normal` a N by 3 matrix giving the normal at each node or element
- `.ID` a N by 1 vector giving identifiers. For normals at integration points, element coordinates can be given as two or three additional columns.

- `.opt` an option vector. `opt(1)` gives the type of map (1 for normals at element centers, 2 for normals at nodes, 3 normals at integration points specified as additional columns of `Data.ID`).
- `.vertex` an optional N by 3 matrix giving the location of each vector specified in `.normal`. This can be used for plotting.

MPC

MPC (multiple point constraint) entries are rows of the `Case.Stack` cell array giving `{'MPC', Name, Data}`. `Name` is a string identifying the entry. `Data` is a structure with fields `Data.ID` positive integer for identification. `Data.c` is a sparse matrix whose columns correspond to DOFs in `Data.DOF`. `c` is the constraint matrix such that $[c] \{q\} = \{0\}$ for q defined on `DOF`.

`Data.slave` is an optional vector of slave DOFs in `Data.DOF`. If the vector does not exist, it is filled by `feutil FixMpcMaster`.

Note that the current implementation has no provision for using local coordinates in the definition of MPC (they are assumed to be defined using global coordinates).

par (SDT)

`par` entries are used to define variable coefficients in element selections. It is nominally used through `upcom Par` commands but other routines may also use it [46].

RBE3 (SDT)

`rbe3` constraints enforce the motion of a slave node as a weighted average of master nodes. Two definition strategies are supported in SDT, either direct or implicit. There are known robustness problems with the current implementation of this constraint.

The direct definition explicitly declares each node with coupled DOFs and weighting in a `data` field. Several `rbe3` constraints can be declared in `data.data`. Each row of `data.data` codes a set of constraints following the format

```
Rbe3ID NodeIdSlave DofSlave Weight1 DofMaster1 NodeId1 ...
```

`DofMaster` and `DofSlave` code which DOFs are used (123 for translations, 123456 for both translations and rotations).

The implicit definition handles *Node Selectors* described in section 7.11 to define the `rbe3`. The input is then a structure:

```

data=struct('SlaveSel','NodeSel',...
           'MasterSel','NodeSel',...
           'DOF', DofSlave,...
           'MasterDOF', DofMaster);

```

`SlaveSel` is the slave node selection (typically a single node), `MasterSel` is the master node selection, `DOF` is the declaration of the slave node coupling, `MasterDOF` is the declaration of the master nodes coupling (same for all master nodes).

Grounding or coupling the slave node movement is possible through the use of a `celas`, as shown in the example below featuring an implicit `rbe3` definition. In a practical approach, the slave node is duplicated and a `celas` element is generated between the two, which allows the definition of global movement stiffnesses. Constraining the rotation of a drilled block around its bore axis is considered using a global rotation stiffness.

```

% Definition of a drilled block around y
model=feutil('ObjectHoleInBlock 0 0 0 1 0 0 0 1 0 2 2 2 .5 4 4 4');
model=fe_mat('DefaultI1',model); % default material properties
model=fe_mat('defaultP1',model); % default element integration properties
% Generation of the bore surface node set
[i1,r1]=feutil('Findnode cyl ==0.5 o 0 0 0 0 1 0',model);
model=feutil('AddsetNodeId',model,'bolt',r1(:,1));
% Generation of the slave node driving the global bore movement
model.Node(end+[1:2],1:7)=[242 0 0 0 0 0 0;244 0 0 0 0 0 0];
% Addition of the celas element between the slave node and its duplicate
model.Elt(end+[1:2],1:7)=[inf abs('celas') 0;242 244 123456 0 0 0 1e11];
model=feutil('AddSetNodeId',model,'ref_rot',244);
% Definition of the RBE3 constraint
data=struct('SlaveSel','setname ref_rot',...
           'MasterSel','setname bolt',...
           'DOF',123456,... % Slave node constrained on 6 DOF
           'MasterDOF',123); % Master only use translation
model=fe_case(model,'rbe3','block_mov',data);
% Grounding the global y rotation (leaving the celas stiffness work)
model=fe_case(model,'fixdof','ClampBlockRot',242.05);
% 5 rigid body modes model obtained
def=fe_eig(model,[5 20 1e3]);
cf=feplot(model,def);fecom('curtabCases','rbe3');fecom('ProViewOn');

```

rigid

See details under `rigid` which also illustrates the `RigidAppend` command.

Sens ... (SDT)

`SensDof` entries are detailed in section 4.3. They are stored as rows of the `Case.Stack` cell array giving `{'SensDof', Name, data}`. `SensStrain` entries have been replaced with strain sensors in `SensDof`.

`R1=fe_case('sensobserve',model,'SensEntryName',def); iiplot(R1)` can be used to extract observations at sensors associated with a given response. The `SensEntryName` can be omitted.

un=0

`model=fe_case(model,'un=0','Normal motion',map);` where `map` gives normals at nodes generates an `mpc` case entry that enforces the condition $\{u\}^T \{n\} = 0$ at each node of the map.

Example

Here is an example combining various `fe_case` commands

```
femesh('reset');
model = femesh('test ubeam plot');
% specifying clamped dofs (FixDof)
model = fe_case(model,'FixDof','clamped dofs','z==0');
% creating a volume load
data = struct('sel','GroupAll','dir',[1 0 0]);
model = fe_case(model,'FVol','Volumic load',data);
% assemble active DOFs and matrices
model=fe_mkn1(model);
% assemble RHS (volumic load)
Load = fe_load(model,'Case1');
% compute static response
kd=ofact(model.K{2});def.def= kd\Load.def; ofact('clear',kd)
Case=fe_case(model,'gett'); def.DOF=Case.DOF;
% plot displacements
feplot('initdef',def);
fecom(';undef;triax;showpatch;promodelinit');
```


fe_ceig

Purpose Computation and normalization of complex modes associated to a second order viscously damped model.

Syntax

```
[psi,lambda] = fe_ceig( ... )
lambda       = fe_ceig(m,c,k)
def          = fe_ceig( ... )
...          = fe_ceig(m,c,k)
...          = fe_ceig({m,c,k,mdof},ceigopt)
...          = fe_ceig({m,c,k,T,mdof},ceigopt)
...          = fe_ceig(model,ceigopt)
```

Description Complex modes are solution of the second order eigenvalue problem (see section 5.5 for details)

$$[M]_{N \times N} \{\psi_j\}_{N \times 1} \lambda_j^2 + [C] \{\psi_j\} \lambda_j + [K] \{\psi_j\} = 0$$

where modeshapes `psi`= ψ and poles $\Lambda = [\lambda_j]$ are also solution of the first order eigenvalue problem (used in `fe_ceig`)

$$\begin{bmatrix} C & M \\ M & 0 \end{bmatrix}_{2N \times 2N} \begin{bmatrix} \psi \\ \psi \Lambda \end{bmatrix}_{2N \times 2N} [\Lambda]_{2N \times 2N} + \begin{bmatrix} K & 0 \\ 0 & -M \end{bmatrix} \begin{bmatrix} \psi \\ \psi \Lambda \end{bmatrix} = [0]_{2N \times 2N}$$

and verify the two orthogonality conditions

$$\psi^T C \psi + \Lambda \psi^T M \psi + \psi^T M \psi \Lambda = I \quad \text{and} \quad \psi^T K \psi - \Lambda \psi^T M \psi \Lambda = -\Lambda$$

`[psi,lambda] = fe_ceig(m,c,k)` is the old low level call to compute all complex modes. For partial solution you should use `def = fe_ceig(model,ceigopt)` where `model` can be replaced by a cell array with `{m,c,k,mdof}` or `{m,c,k,T,mdof}` (see the example below). Using the projection matrix `T` generated with `fe_case('gett')` is the proper method to handle boundary conditions.

Options give `[CeigMethod EigOpt]` where `CeigMethod` can be 0 (full matrices), 1 (real modes then complex ones on the same basis) 2 and 3 are refined solvers available with the VISCO extension. `EigOpt` are standard `fe_eig` options.

Here is a simple example of `fe_ceig` calls.

```
model=demosdt('demoubeam'); cf=feplot;
[Case,model.DOF]=fe_mkn1('init',model);
```

fe_ceig

```
m=fe_mkn1('assemble not',model,Case,2);
k=fe_mkn1('assemble not',model,Case,1);

kc=k*(1+i*.002); % with hysteretic damping
def1=fe_ceig({m,[],kc,model.DOF},[1 6 10 1e3]); % free modes
def2=fe_ceig({m,[],kc,Case.T,model.DOF},[1 6 10 1e3]); % fixed modes

cf.def=def1; % show def1 in feplot figure
```

See also [fe_eig](#), [fe_mk](#), [nor2ss](#), [nor2xf](#), section 5.3

fe_coor

Purpose	Coordinate transformation matrices for Component Mode Synthesis problems.
Syntax	<pre>[t] = fe_coor(cp) [t,nc] = fe_coor(cp,opt)</pre>
Description	<p>The different uses of <code>fe_coor</code> are selected by the use of options given in the argument <code>opt</code> which contains <code>[type method]</code> (with the default values <code>[1 3]</code>).</p> <p><code>type=1</code> (default) the output <code>t</code> is a basis for the kernel of the constraints <code>cp</code></p> $\text{range}([T]_{N \times (N-NC)}) = \ker([c]_{NS \times N})$ <p>$NC \leq NS$ is the number of independent constraints.</p> <p><code>type=2</code> the output argument <code>t</code> gives a basis of vectors linked to unit outputs followed by a basis for the kernel</p> $T = \begin{bmatrix} [TU]_{N \times NS} & [TK]_{N \times (N-NS)} \end{bmatrix} \quad \text{with} \quad [c]_{NS \times N} [T] = \begin{bmatrix} [I] \\ [0]_{NS \times (N-NS)} \end{bmatrix}$ <p>If $NC < NS$ such a matrix cannot be constructed and an error occurs.</p> <p><code>method</code> the kernel can be computed using: 1 a singular value decomposition <code>svd</code> (default) or 3 a <code>lu</code> decomposition. The <code>lu</code> has lowest computational cost. The <code>svd</code> is most robust to numerical conditioning problems.</p>
Usage	<p><code>fe_coor</code> is used to solve problems of the general form</p> $\begin{aligned} [Ms^2 + Cs + K] \{q(s)\} &= [b] \{u(s)\} & \text{with} & [c_{int}] \{q(s)\} = 0 \\ \{y(s)\} &= [c] \{q(s)\} \end{aligned}$ <p>which are often found in CMS problems (see section 6.2.6 and [39]).</p> <p>To eliminate the constraint, one determines a basis T for the kernel of $[c_{int}]$ and projects the model</p> $\begin{aligned} [T^T M T s^2 + T^T C T s + T^T K T] \{q_R(s)\} &= [T^T b] \{u(s)\} \\ \{y(s)\} &= [c T] \{q_R(s)\} \end{aligned}$
See also	Section 7.14, <code>fe_c</code> , the <code>d.cms</code> demo

fe_curve

Purpose Generic handling of curves and signal processing utilities

Syntax `out=fe_curve('command',MODEL,'Name',...);`

Commands

`fe_curve` is used to handle curves and do some basic signal processing. The format for curves is described in section 7.9. The `iiplot` interface may be used to plot curves and a basic call would be `iiplot(Curve)` to plot curve data structure `Curve`.

Accepted commands are

`bandpass Unit f_min f_max`

```
out=fe_curve('BandPass Unit f_min f_max',signals);
```

realizes a true bandpass filtering (i.e. using `fft()` and `ifft()`) of time signals contained in curves `signals`. `f_min` and `f_max` are given in units `Unit`, whether Hertz(Hz) or Radian(Rd). With no `Unit`, `f_min` and `f_max` are assumed to be in Hertz.

```
out=fe_curve('TestFrame');% 3 DOF oscillator response to noisy input
fe_curve('Plot',out{2}); % "unfiltered" response
filt_disp=fe_curve('BandPass Hz 70 90',out{2}); % filtering
fe_curve('Plot',filt_disp); title('filtered displacement');
```

`datatype [,cell]`

```
out=fe_curve('DataType',DesiredType);
```

returns a data structure describing the data type, useful to fill `.xunit` and `.yunit` fields for curves definition. `DesiredType` could be a string or a number corresponding to the desired type. With no `DesiredType`, the current list of available types is displayed. One can specify the unit with `out=fe_curve('DataType',DesiredType,'unit');`.

`DataTypeCell` returns a cell array rather than data structure to follow the specification for curve data structures.

`getcurve`

```
curve=fe_curve('getcurve',model,'curve_name');
```

extracts curve `curve_name` from `model.Stack` or the possible curves attached to a load case. If the user does not specify any name, all the curves are returned in a cell array.

`h1h2 input_channels`

```
FRF=fe_curve('H1H2 input_channels',frames,'window');
```

computes H1 and H2 FRF estimators along with the coherence from time signals contained in cell array `frames` using window `window`. The time vector is given in `frames1.X` while `input_channels` tells which columns of in `frames1.Y` are inputs. If more than one input channel is specified, true MIMO FRF estimation is done, and $H\nu$ is used instead of H2. When multiple frames are given, a mean estimation of FRF is computed.

Note: To ensure the proper assembly of H1 and $H\nu$ in MIMO FRF estimation case, a weighing based on maximum time signals amplitude is used. To use your own, use

```
FRF=fe_curve('H1H2 input_channels',frames>window,weighing);
```

where `weighing` is a vector containing weighing factors for each channel. To avoid weighing, use

```
FRF=fe_curve('H1H2 input_channels',frames>window,0);
```

`noise`

OBSOLETE : use `fe_curve TestNoise` instead

```
noise=fe_curve('Noise',Nw_pt,fs,f_max);
```

computes a `Nw_pt` points long time signal corresponding to a “white noise”, with sample frequency `fs` and a unitary power spectrum density until `f_max`. `fs/2` is taken as `f_max` when not specified. The general shape of noise power spectrum density, extending from 0 to `fs/2`, can be specified instead of `f_max`.

```
% computes a 2 seconds long white noise, 1024 Hz of sampling freq.
```

```
% with "rounded" shape PSD
```

```
fs=1024; sample_length=2;
```

```
Shape=exp(fe_curve('window 1024 hanning'))-1;
```

```
noise_h=fe_curve('noise',fs*sample_length,fs,Shape);
```

```
noise_f=fe_curve('fft',noise_h);
```

```
figure(1);
```

```
subplot(211);fe_curve('plot -gca',noise_h);axis tight;
```

```
subplot(212);fe_curve('plot -gca',noise_f);axis tight;
```

plot

```
fe_curve('plot',curve); plots the curve curve.
fe_curve('plot',fig_handle,curve); plots curve in the figure with handle fig_handle.
fe_curve('plot',model,'curve_name'); plots the curve of model.Stack named curve_name.
fe_curve('plot',fig_handle,model,curve_name); plots curve named curve_name
stacked in .Stack field of model model.

% computes a 2 seconds long white noise, 1024 Hz of sampling freq.
fs=1024; sample_length=2;
noise=fe_curve('noise',fs*sample_length,fs);
noise.xunit=fe_curve('DataType','Time');
noise.yunit=fe_curve('DataType','Excit. force');
noise.name='Input force';

fe_curve('Plot',noise);
```

resspectrum [*True, Pseudo*] [*Abs., Rel.*] [*Disp., Vel., Acc.*]

```
out=fe_curve('ResSpectrum',signal,freq,damp);
computes the response spectrum associated to the time signal given in signal. Time
derivatives can be obtained with option -v or -a. Time integration with option +v
or +a. Pseudo derivatives with option PseudoA or PseudoV. freq and damp are
frequencies (in Hz) and damping ratios vectors of interest for the response spectra.
For example
```

```
wd=fileparts(which('d_ubeam'));
% read the acceleration time signal
bagnol_ns=fe_curve(['read' fullfile(wd,'bagnol_ns.cyt')]);

% read reference spectrum
bagnol_ns_rspec_pa= fe_curve(['read' fullfile(wd,'bagnol_ns_rspec_pa.cyt')]);
% compute response spectrum with reference spectrum frequencies
% vector and 5% damping
RespSpec=fe_curve('ResSpectrum PseudoA',...
                  bagnol_ns,bagnol_ns_rspec_pa.X/2/pi,.05);

fe_curve('plot',RespSpec); hold on;
plot(RespSpec.X,bagnol_ns_rspec_pa.Y,'r');
legend('fe\_curve','cyberquake');
```

returny

If curve has a `.Interp` field, this interpolation is taken in account. If `.Interp` field is not present or empty, it uses a degree 2 interpolation by default.

To force a specific interpolation (over passing `.interp field`, one may insert the `-linear`, `-log` or `-stair` option string in the command.

To extract a curve `curve_name` and return the values `Y` corresponding to the input `X`, the syntax is

```
y = fe_curve('returny',model,curve_name,X);
```

Given a `curve` data structure, to return the values `Y` corresponding to the input `X`, the syntax is

```
y = fe_curve('returny',curve,X);
```

set

This command sets a curve in the model. 3 types of input are allowed:

- A data structure, `model=fe_curve(model,'set',curve_name,data_structure)`
- A string to interpret, `model=fe_curve(model,'set',curve_name,string)`
- A name referring to an existing curve (for load case only), `model=fe_curve(model,'set LoadCurve',load_case,chanel,curve_name)`

When you want to associate a curve to a load for time integration it is preferable to define formally the time dependence (if not curve can be interpolated or extrapolated).

The following example illustrates the different calls.

```
model=fe_time('demo bar'); q0=[];

% curve defined by a by-hand data structure:
c1=struct('ID',1,'X',linspace(0,1e-3,100), ...
        'Y',linspace(0,1e-3,100),'data',[],...
        'xunit',[],'yunit',[],'unit',[],'name','curve 1');
model=fe_curve(model,'set','curve 1',c1);
```

```
% curve defined by a string to evaluate (generally test fcn):
model=fe_curve(model,'set','step 1','TestStep 1e-4*10');
% curve defined by a reference curve:
c2=fe_curve('test -ID 100 ricker 10e-4 100 1 100e-4');
c3=fe_curve('test sin 10e-4 100 1 100e-4');

model=fe_curve(model,'set','ricker 1',c2);
model=fe_curve(model,'set','sin 1',c3);

% define Load with curve definition
LoadCase=struct('DOF',[1.01;2.01],'def',1e6*eye(2),...
               'curve',{fe_curve('test ricker 20e-4 1'),...
                        'ricker 1'}));
model = fe_case(model,'DOFLoad','Point load 1',LoadCase);
% modify a curve in the load case
model=fe_curve(model,'set LoadCurve','Point load 1',2,'TestStep 1e-4*10');
```

Test ...

The `test` command handles a large array of analytic and tabular curves. In OpenFEM all parameters of each curve must be given in the proper order. In SDT you can specify only the ones that are not the default using their name.

When the abscissa vector (time, frequency, ...) is given as shown in the example, a tabular result is returned.

Without output argument the curve is simply plotted.

```
fe_curve('test') % lists curenly implemented curves

t=linspace(0,3,1024); % Define abscissa vector
% OpenFEM format with all parameters
C1=fe_curve('test ramp 0.6 2.5 2.3',t);
C2=fe_curve('TestRicker 2 2',t);

% SDT format non default parameters given with their name
C3=fe_curve('Test CosHan f0=5 A=3');
C4=fe_curve('testEval 3*cos(2*pi*5*t)');

% Now display result
C3=fe_curve(C3,t);C4=fe_curve(C4,t)
```



```
figure(1);plot(t,[C1.Y C2.Y C4.Y C3.Y]);
legend(C1.name,C2.name,C4.name,C3.name)
```

A partial list of accepted test curves follows

- `Testsin`, `Testcos`, `TestTan`, `TestExp`, accept parameters `T` period and `A` amplitude. `-stoptime Tf` will truncate the signal.
- `TestRamp t0 t1 Yf` has a ramp starting at zero until t_0 and going up to Yf at t_1 . The number of intermediate value can be controled with the abscissa vector.
- `TestRicker dt A t0` generates a Ricker function typically used to represent impacts of duration dt and amplitude A , starting from time t_0 .
- `TestSweep f0 f1 t0 t1` generates a sweep cosine from t_0 to t_1 , with linear frequency sweeping from f_0 to f_1 .

$$Y = \cos(2 * \pi * (f_0 + \frac{f_1 - f_0}{NStep} * t) * t)$$

Note that f_1 is not the final instant frequency of the signal (which will be $f_0 + 2 * \frac{f_1 - f_0}{NStep}$ for ascending sweep).
- `TestStep t1` generates a step which value is one from time 0 to time t_1 .
- `TestNoise "window"` computes a time signal corresponding to a white noise , with the power spectrum density specified as the `window` parameter. For example `TestNoise "Box 1 0 100"` defines a unitary power spectrum density until 100 Hz.
- `TestBox A min max` generates a samble box signal from `min` to `max` abscissa, with an amplitude `A`.
- `TestEval str` generates the signal obtained by evaluating the string `str` function of `t`.

testframe

`out=fe_curve('TestFrame');` computes the time response of a 3 DOF oscillator to a white noise and fills the cell array `out` with noise signal in cell 1 and time response in cell 2. Use `sdtweb('fe_curve.m#TestFrame')` to open the function at this example.

timefreq

```
out=fe_curve('TimeFreq',Input,xf);
computes response of a system with given transfer functions FRF to time input
Input. Sampling frequency and length of time signal Input must be coherent with
frequency step and length of given transfer FRF.

fs=1024; sample_length=2; % 2 sec. long white noise
noise=fe_curve('noise',fs*sample_length,fs);% 1024 Hz of sampling freq.
[t,f,N]=fe_curve('getXTime',noise);

% FRF with resonant freq. 50 100 200 Hz, unit amplitude, 2% damping
xf=nor2xf(2*pi*[50 100 200].',.02,[1 ; 1 ; 1],[1 1 1],2*pi*f);

Resp=fe_curve('TimeFreq',noise,xf); % Response to noisy input
fe_curve('Plot',Resp); title('Time response');
```

Window ...

Use `fe_curve window` to list implemented windows. The general calling format is `win=fe_curve('Window Nb_pts Type Arg')`; which computes a *Nb_pts* points window. The default is a symmetric window (last point at zero), the command option `-per` clips the last point of a $N + 1$ long symmetric window.

For the exponential window the arguments are three doubles. `win = fe_curve('Window 1024 Exponential 10 20 10')`; returns an exponential window with 10 zero points, a 20 point flat top, and a decaying exponential over the 1004 remaining points with a last point at `exp(-10)`.

`win = fe_curve('Window 1024 Hanning')`; returns a 1024 point long hanning window.

See also

`fe_load`, `fe_case`

fe_cyclic

Purpose	Support for cyclic symmetry computations.
Syntax	<pre>model=fe_cyclic('build NSEC',model,LeftNodeSelect) def=fe_cyclic('eig NDIAM',model,EigOpt)</pre>
Description	<code>fe_cyclic</code> groups all commands needed to compute responses assuming cyclic symmetry. For more details on the associated theory you can refer to [47].

Assemble [-struct]

This command supports the computations linked to the assembly of gyroscopic coupling, gyroscopic stiffness and tangent stiffness in geometrically non-linear elasticity. The input arguments are the model and the rotation vector (in rad/s)

```
model=demosdt('demo sector all');
[K,model,Case]=fe_case('assemble -matdes 2 1 NoT -cell',model);
SE=fe_cyclic('assemble -struct',model,[0 0 1000]); %

def=fe_eig({K{1:2},Case.T,model.DOF},[6 20 0]);% Non rotating modes

def2=fe_eig({K{1},SE.K{4},Case.T,model.DOF},[6 20 0]); % Rotating mode s

[def.data def2.data]
```

Note that the rotation speed can also be specified using a stack entry `model=stack_set(model,'info','Omega',[0 0 1000])`.

Build ...

`model=fe_cyclic('build nsec epsl len',model,'LeftNodeSelect')` adds a `cyclic` symmetry entry in the model case. It automatically rotates the nodes selected with `LeftNodeSelect` by $2\pi/nsec$ and finds the corresponding nodes on the other sector face. The default for `LeftNodeSelect` is `'GroupAll'` which selects all nodes.

The alternate command `model=fe_cyclic('build nsec epsl len -intersect',model)` is much faster but does not implement strict node tolerancing and may thus need an adjustment of `epsl` to higher values.

Command options are

- `nsec` is the optional number of sectors. An automatic determination of the number of angular sectors is implemented from the angle between the left and right interface nodes with the minimum radius. This guess may fail in some situations so that the argument may be necessary.
- `nsec=-1` is used for periodic structures and you should then provide the translation step. For periodic solutions, `model=fe_cyclic('build -1 tx ty tz epsl len -intersect',model,'LeftNodeSelect')` specifies 3 components for the spatial periodicity.
- `Fix` will adjust node positions to make the left and right nodes sets match exactly.
- `epsl len` gives the tolerance for edge node matching.
- `-equal` can be used to build a simple periodicity condition for use outside of `fe_cyclic`. This option is not relevant for cyclic symmetry.
- `-ByMat` is used to allow matching by `MatId` which allows for proper matching of coincident nodes.

```
model=demosdt('demo sector 5');  
cf.model=fe_cyclic('build epsl 1e-6',model);
```

LoadCentrifugal

The command is used to build centrifugal loads based on an `info,Omega` stack entry in the form

```
data=struct('data',[0 0 1000],'unit','RPM');  
model=stack_set(model,'info','Omega',data);  
model=fe_cyclic('LoadCentrifugal',model);
```

Eig

`def=fe_cyclic('eig ndiam',model,EigOpt)` computes `ndiam` diameter modes using the cyclic symmetry assumption. For `ndiam>0` these modes are complex to account for the inter-sector phase shifts. `EigOpt` are standard options passed to `fe_eig`.

This example computes the two diameter modes of a three bladed disk also used in the `d_cms2` demo.

```
model=demosdt('demo sector');  
model=fe_cyclic('build 3',model,'groupall');  
fe_case(model,'info')  
def=fe_cyclic('eig 2',model,[6 20 0 11]);  
fe_cyclic('display 3',model,def)
```

The basic functionality of this command is significantly extended in [fe_cyclicb](#) [ShaftEig](#) that is part of the SDT/Rotor toolbox.

See also

Section 7.14

fe_def

Purpose Utilities for FEM related data structures.

Syntax `... = fe_def(def, 'command', ...)`
`... = fe_def('command', ...)`

Description Accepted commands are

SubDef, SubDof, SubCh

`def=fe_def('SubDef',def,ind)`; keeps deformations associated with `ind`, which a vector of indices or a logical vector (for example `ind=def.data(:,1)<500` can be used to select frequencies below 500). Other fields of the `def` structure are truncated consistently.

`def=fe_def('SubDof',def,DOF)` is extracts a subset of DOFs based on defined DOF or with `def=fe_def('subdofind',def,ind)` indices (again either values or logicals). This command is partially redundant with `feutilb PlaceInDof` called with `def2 = feutilb('PlaceInDof',DOF,def)`. The main difference is the ability to add zeros (use `DOF` larger than `def.DOF`) and support `sens` structures.

`fe_def('SubDofInd-Cell',def,ind_dof,ind_def)` returns a clean cell array listing selected DOFs and responses. This is typically used to generate clean tables.

`fe_def('SubCh',def,ind)` is similar to `SubDof` but allows but supports more advanced selection for multi-dimensional curves. This command is not fully documented.

DefFreq

`w=fe_def('DefFreq',DISK)` returns frequencies defined in the `info,Freq` entries using Hz units.

Range

`Range` commands are used to manipulate experiment (series of design points) specifications. A range is a data structure with fields

- `.val` numeric array containing one design point per row and one design parameter per column.

- `.lab` cell array of strings giving a parameter label for each column. These labels should be acceptable fieldnames (no spaces, braces, ...)
- `.param` optional structure with fields associated with parameter labels used for formatting and analysis. Accepted values are detailed below. It is not necessary to define a `.param` field for each design parameter.
- `.edge` optional connectivity matrix used to define lines connecting different design points of the experiment

Acceptable `.param` field entries are

- `.type` a string. Typically `double` or `pop`.
- `.choices`, for `.type='pop'`, contains a cell array of strings. The parameter value then gives the index within `.choices`
- `.LabFcn` a command to be evaluated with `st1=eval(r2.LabFcn)` to generate the proper label. For example `'sprintf('%.1f ms',val/1000)'` is used to generate a label in a different unit.
- `.level` is an integer specifying the computational step at which a given parameter can be modified. This is used to generate experiment trees.

RangeGrid

```
Range=fe_def('RangeGrid',par);
```

`Range` is defined by a grid of all the parameter values defined in `par`. `par` is expected as a cell array with as many elements as parameters. Each cell can be

- a string `'lab "label" min min max max cur cur scale "scale" NPoints NPoints'`. `"label"` is the parameter name. Then the minimum, maximum and nominal values are defined. Scale can be `"lin"` for linear scale or `"log"` for logarithmic scale. `NPoints` defines the number of point for the parameter vector.
- a range data structure
- a numeric vector in the old `upcom` format `[type cur min max scale]` with `type` defining the matrix type (unused here), `scale==2` indicates a logarithmic variation.

`par` can also be a matrix following the model of the visco tools parameters definition (see `fevisco Range` for more details).

As an illustration, following example defines a grid 6x7 of 2 parameters named length and thickness:

```
par={'lab "length" min 10 max 20 cur 10 scale "lin" NPoints 6',...
     'lab "thickness" min 1e-3 max 2e-3 cur 0 scale "log" NPoints 7'};
Range=fe_def('RangeGrid',par);
```

RangeVect

```
Range=fe_def('RangeVect',par);
```

Simply concatenate all parameter ranges (they must have the same length).

RangeRes

```
R1=fe_def('RangeRes',R1,Range);
```

This command reshapes the last dimension of the result curve `R1` according to the `Range`. For a grid DOE last dimension is split in as many dimensions as parameters. For a vector DOE, last dimension is only redefined by a cell array of labels defining each design point.

fe_eig

Purpose Computation of normal modes associated to a second order undamped model.

Syntax

```
def = fe_eig(model,EigOpt)
def = fe_eig({m,k,mdof},EigOpt)
def = fe_eig({m,k,T,mdof},EigOpt)
[phi, wj] = fe_eig(m,k)
[phi, wj, kd] = fe_eig(m,k,EigOpt,imode)
```

Description The normal modeshapes $\mathbf{phi}=\phi$ and frequencies $\mathbf{wj}=\text{sqrt}(\text{diag}(\Omega^2))$ are solution of the undamped eigenvalue problem (see section 5.2)

$$- [M] \{\phi_j\} \omega_j^2 + [K] \{\phi_j\} = \{0\}$$

and verify the two orthogonality conditions

$$[\phi]^T [M]_{N \times N} [\phi]_{N \times N} = [I]_{N \times N} \quad \text{and} \quad [\phi]^T [K] [\phi] = \begin{bmatrix} \Omega_j^2 \\ \backslash \end{bmatrix}$$

The outputs are the data structure `def` (which is more appropriate for use with high level functions `feplot`, `nor2ss`, ... since it keeps track of the signification of its content, frequencies in `def.data` are then in **Hz**) or the modeshapes (columns of `phi`) and frequencies `wj` in **rad/s**. Note how you provide `{m,k,mdof}` in a cell array to obtain a `def` structure without having a model.

The optional output `kd` corresponds to the factored stiffness matrix. It should be used with methods that do not renumber DOFs.

`fe_eig` implements various algorithms to solve this problem for modes and frequencies. Many options are available and it is important that you read the notes below to understand how to properly use them. The option vector `EigOpt` can be supplied explicitly or set using `model=stack_set(model, 'info', 'EigOpt', EigOpt)`. Its format is

`[method nm Shift Print Thres]` (default values are `[2 0 0 0 1e-5]`)

- `method`
 - **2 default** full matrix solution. Cannot be used for large models.
 - **6** IRA/Sorensen solver. Faster than **5** but less robust.

- **5** Lanczos solver allows specification of frequency band rather than number of modes. To turn off convergence check add **2000** to the option (**2105**, **2005**, ...).
 - **50** Callback to let the user specify an external solver method using `setpref('SDT', 'External')`.
 - **106**, **104** same as **6,4** methods but no initial DOF renumbering. This is useless with the default `ofact('methodspfmex')` which renumbers at factorization time.
 - **0** SVD based full matrix solution
 - **1** subspace iteration which allows to compute the lowest modes of a large problem where sparse mass and stiffness matrices are used.
 - **3** Same as **5** but using `ofact('methodlu')`.
 - **4** Same as **5** but using `ofact('methodchol')`.
- **nm** number of modes to be returned. A non-integer or negative **nm**, is used as the desired **fmax** in **Hz** for iterative solvers (method 5 only).
 - **shift** value of mass shift (should be non-zero for systems with **rigid body modes**, see notes below). The subspace iteration method supports iterations without mass shift for structures with rigid body modes. This method is used by setting the shift value to **Inf**.
 - **print** level of printout (**0** none, **11** maximum)
 - **thres** threshold for convergence of modes (default **1e-5** for the subspace iteration and Lanczos methods)

Finally, a set of vectors **imode** can be used as an initial guess for the subspace iteration method (**method 1**).

Notes

- The default full matrix algorithm (**method=2**) cleans results of the MATLAB **eig** function. Computed modes are mass normalized and complex parts, which are known to be spurious for symmetric eigenvalue problems considered here, are

eliminated. The alternate algorithm for full matrices (`method=0`) uses a singular value decomposition to make sure that all frequencies are real. The results are thus wrong, if the matrices are not symmetric and positive definite (semi-positive definite for the stiffness matrix).

- The preferred partial solver is `method 6` which calls `eigs` (ARPACK) properly and cleans up results. This solver sometimes fails to reach convergence, use `method 5` then.
- The subspace iteration and Lanczos algorithms are rather free interpretation of the standard algorithms (see Ref. [36] for example).
- The Lanczos algorithm (methods `3,4,5`) is much faster than the subspace iteration algorithm (method `1`). A double Orthogonalization scheme and double restart usually detects multiple modes.
- For systems with rigid body modes, you must specify a mass-shift. A good value is about one tenth of the first flexible frequency squared, but the Lanczos algorithm tends to be sensitive to this value (you may occasionally need to play around a little). If you do not find the expected number of rigid body modes, this is can be reason.
- DOFs with zero values on the stiffness diagonal are eliminated by default. You can bypass this behavior by giving a shift with unit imaginary value (`eigopt(3)=1e3+1i` for example).

Example

Here is an example containing a high level call

```
model =demosdt('demo gartfe');
cf=feplot;cf.model=model;
cf.def=fe_eig(model,[6 20 1e3 11]);
fecom chc10
```

and the same example with low level commands

```
model =demosdt('demo gartfe');
[m,k,mdof] = fe_mknl(model);
cf=feplot;cf.model=model;
cf.def=fe_eig({m,k,mdof},[5 20 1e3]);fecom chc10
```

See also [fe_ceig](#), [fe_mk](#), [nor2ss](#), [nor2xf](#)

fe_exp

Purpose Expansion of experimental modeshapes.

Syntax

```
emode = fe_exp(yTest,c,T)
emode = fe_exp(yTest,sens,T,opt)
yExp = fe_exp(yTest,fTest,sens,m,k,mdof,freq,opt)
```

Description A unified perspective on interpolation and the more advanced finite element based expansion methods are discussed in the tutorial 3.3. An example is treated in detail in the [gartco](#) demonstration. This section gives a list of available methods with a short discussion of associated trade-offs.

Interpolation

Interpolation methods can be implemented easily by constructing a basis **t** of possible displacements and minimizing the test error as discussed in section 3.3.2.

For example, if node 2 is placed at a quarter of the distance between nodes 1 and 3 whose motion is observed. A linear interpolation for translations in the *x* direction is built using

```
ndof = [1.01;2.01;3.01]; T= [1 0;3/4 1/4;0 1];
yExp = fe_exp(yTest,fe_c(ndof,[1.01;3.01])*T,T)
```

For expansion of this form, **T** must contain at most as many vectors as there are sensors. In other cases, a solution is still returned but its physical significance is dubious.

Modal, Serep

Modal or SEREP expansion is a subspace based expansion using the subspace spanned by low frequency target modes. With a sensor configuration defined (**sens** defined using [fe_sens](#)), a typical call would be

```
[phi,wj] = fe_eig(m,k,[105 50 1e3]);
TargetModes = phi(:,[IndicesOfTargetModes]);
mdex = fe_exp(Iires.',sens,TargetModes);
```

This method is very easy to implement. Target modes can be imported from an external code. A major limitation is the fact that results tend to be sensitive to target mode selection.

You can impose that an orthogonal linear combination of the modes is used for the expansion using `mdex = fe_exp(IIres.',sens,TargetModes,2);`. This is motivated for cases where both test and analysis modeshapes are mass normalized and will provide mass orthonormal expanded modeshapes [48]. In practice it is rare that test results are accurately mass normalized and the approach is only implemented for completeness.

Static

Static expansion can be obtained using two different approaches. If constraint modes (or attachment modes for structures without rigid body modes) are imported (or computed using `[T] = fe_reduc('static',m,k,mdof,sdof)`), static expansion can be considered as a subspace method and the solution is found with

```
mdex = fe_exp(IIres.',sens,T);
```

The subspace can also be computed by `fe_exp`, using

```
mdex = fe_exp(IIres.',0,sens,m,k,mdof);
```

which will handle arbitrary sensor configurations defined in `sens`.

The main limitation with static expansion is the existence of a frequency limit (first frequency found when all sensors are fixed). `[mdex,phi_fixed] = fe_exp(IIres.',0,sens,m,k,mdof)`; returns an estimate of the first 10 fixed sensor modes. If the first frequency is close to your test bandwidth, you should consider using dynamic expansion.

Dynamic, RBDE

Dynamic expansion is supported at a single frequency or at one frequency for each deformation to be expanded using

```
mdex = fe_exp(yTest,fExp*2*pi,sens,m,k,mdof);
```

Reduced basis dynamic expansion (RBDE) is supported using

```
mdex = fe_exp(yTest,fExp*2*pi,sens,m,k,mdof,T);
```

where `T` typically contains normal and attachment modes (see `gartco` for an example). Note that, when using reduced bases, you can provide `T'*m*T` and `T'*k*T` instead of `m` and `k` which saves a lot of time if these matrices have already been computed.

MDRE, MDRE-WE

Minimum dynamic residual expansion (MDRE) is currently only implemented for normal modeshape expansion. Furthermore, computational times are generally only acceptable for the reduced basis form of the algorithm. A typical call would be

```
mdex = fe_exp(yTest,fExp*2*pi,sens,m,k,mdof,T,'mdre');
```

where **T** contains normal and attachment modes but often needs to be renormalized using **T = fe_norm(T,m,k)** to prevent numerical conditioning problems (see [gartco](#) for an example). Note that, when using reduced bases, you can provide **T'*m*T** and **T'*k*T** instead of **m** and **k** which saves a lot of time if these matrices have already been computed.

MDRE-WE (Minimum dynamic residual expansion with measurement error) iteratively adjusts the relative weighting γ_j between model and test error in (3.9). Input arguments specify a starting value for γ_j and a relative error bound. The initial value for γ_j is increased (multiplied by 2) until $\epsilon_j / \|\{y_{jTest}\}\|$ is below the given bound. A typical call would be $\epsilon_j / \|\{y_{jTest}\}\|$

```
opt=struct('type','mdrewe','gamma',1,'MeasErr',.1);  
yTest=IIres.';wTest=IIpo(:,1)*2*pi;  
[mdex,opt,mdexr,err]=fe_exp(yTest,wTest,sens,m,k,mdof,T,opt);
```

where the **opt** in the output gives the adjusted values of γ_j , **mdexr** is the expanded vector expressed in the generalized coordinates associated with **T**, and **err** gives the objective function value (first row) and relative error (second row).

See also [fe_sens](#), [fe_reduc](#), section 3.3, [gartco](#) demo.

fe_gmsh

- Purpose** Information about GMSH can be found at <http://www.geuz.org/gmsh/>
To call the GMSH mesher from SDT.
- Syntax** `model=fe_gmsh(command,model,...);`
- Description** The main operation is the automatic meshing of surfaces.

Example

This example illustrates the automatic meshing of a plate

```
FNode = [1 0 0 0 0 0 0; 2 0 0 0 1 0 0; 3 0 0 0 0 2 0];
femesh('objectholeinplate 1 2 3 .5 .5 3 4 4');
model=femesh('model0');
model.Elt=feutil('selelt seledge ',model);
model.Node=feutil('getnode groupall',model);
model=fe_gmsh('addline',model,'groupall');
model.Node(:,4)=0; % reset default length
mo1=fe_gmsh('write temp.geo -lc .3 -run -2 -v 0',model);
delete('temp.msh');delete('temp.geo');feplot(mo1)
```

This other example makes a circular hole in a plate

```
% Hole in plate :
model=feutil('Objectquad 1 1',[0 0 0; 1 0 0;1 1 0;0 1 0],1,1); %
model=fe_gmsh('addline -loop1',model,[1 2; 2 4]);
model=fe_gmsh('addline -loop1',model,[4 3; 3 1]);
model=fe_gmsh('AddFullCircle -loop2',model,[.5 .5 0; .4 .5 0; 0 0 1]);

model.Stack{end}.PlaneSurface=[1 2];
mo1=fe_gmsh('write tmp.geo -lc .02 -run -2 -v 0',model)
delete('temp.msh');delete('temp.geo');feplot(mo1)
```

To allow automated running of GMSH from MATLAB, this function uses a `info,GMSH` stack entry with the following fields

<code>.Line</code>	one line per row referencing <code>NodeId</code> . Can be defined using <code>addline</code> commands.
<code>.Circle</code>	define properties of circles.
<code>.LineLoop</code>	rows define a closed line as combination of elementary lines. Values are row indices in the <code>.Line</code> field. One can also define <code>LineLoop</code> from circle arcs (or mixed arcs and lines) using a cell array whose each row describes a line loop as <code>{'LineType',LineInd,...}</code> where <code>LineType</code> can be <code>Circle</code> or <code>Line</code> and <code>LineInd</code> row indices in corresponding <code>.Line</code> or <code>.Circle</code> field.
<code>.TransfiniteLines</code>	Defines lines which seeding is controlled.
<code>.PlaneSurface</code>	rows define surfaces as a combination of line loops, values are row indices in the <code>.LineLoop</code> field. Negative values are used to reverse the line orientation. 1st column describes the exterior contour, and following the interiors to be removed. As <code>.PlaneSurface</code> is a matrix, extra columns can be filled by zeros.
<code>.EmbeddedLines</code>	define line indices which do not define mesh contours but add additional constraints to the final mesh (see Line In Surface in the <code>gmsh</code> documentation).
<code>.SurfaceLoop</code>	rows define a closed surface as combination of elementary surfaces. Values are row indices in the <code>.PlaneSurface</code> field.

The local mesh size is defined at nodes by GMSH. This is stored in column 4 of the `model.Node`. Command option `-lval` in the command resets the value `val` for all nodes that do not have a prior value.

Add...

Typical calls are of the form `[mdl,RO]=fe_gmsh('Add Cmd',mdl,data)`. The optional second output argument can be used to obtain additional information like the `LoopInfo`. Accepted command options are

- `-loop i` is used to add the given geometries and append the associated indices into the `LineLoop(i)`.
- `FullCircle` defines a circle defined using `data` with rows giving center coordinates, an edge node coordinates and the normal in the last row. 4 arcs of circle are added. In the `LineLoop` field the entry has the form `{'Circle',[ind1 ind2 ind3 ind4]}` where `indi` are the row indices of the 4 arcs of circle created in `.Circle` field.

- `CircleArc` defines a circle arc using `data` with rows giving center coordinates, second and third rows are respectively the first and second edges defined by node coordinates. One can also give 3 `NodeId` as a column instead of x y z.
- `Disk` ...
- `Line` accepts multiple formats. `data` can be a 2 column matrix which each row defines a couple of points from their `NodeId`.
`data` can also be a 2 by 3 matrix defining the coordinates of the 2 extremities.
`data` can also be a string defining a line selection.
 - It is possible to specify a seeding on the line for further meshing operation using additional arguments `seed` and the number of nodes to seed on the line. *E.g.*: `mdl=fe_gmsh('AddLine',mdl,data,'seed',5);` will ask `gmsh` to place 5 nodes on each line declared in `data`.
 - It is possible to define line constrains in mesh interiors using embedded lines (depending on the `gmsh` version). `mdl=fe_gmsh('AddLine',mdl,data,'c` will thus declare the edges found in `data` not as line loops defining surfaces, but as interior mesh constrains. This feature is only supported for lines specified as selections.
- `AddLine3` can be used to declare splines instead of lines in the geometry. For this command to work, `beam3` elements must be used, so that a middle node exists to be declared as the spline control point. For this command, `data` can only be an element string selection.

config

The `fe_gmsh` function uses the `OpenFEM` preference to launch the GMSH mesher.

```
setpref('OpenFEM','gmsh','$HOME_GMSH/gmsh.exe')
```

Ver

Command `Ver` returns the version of `gmsh`, the version is transformed into a double to simplify hierarchy handling (*e.g.* version 2.5.1 is transformed into 251). This command also provides a good test to check your `gmsh` setup as the output will be empty if `gmsh` could not be found.

Read

`fe_gmsh('read FileName.msh')` reads a mesh from the GMSH output format.

Write

`fe_gmsh('write FileName.geo',model);` writes a model (`.Node`, `.Elt`) and geometry data in `model.Stack'info'`, 'GMSH' into a `.geo` file which root name is specified as `FileName`.

- Command option `-lc` allows specifying a characteristic length.
- Command option `-multiple` can be used for automated meshing of several closed contours. The default behavior will define a single Plane Surface combining all contours, while `-mutliple` variant will declare each contour as a single Plane Surface.
- Command option `-keepContour` can be used to force `gmsh` not to add nodes in declared line objects (`Transfinite Line` feature).
- Command option `-spline` can be used (when lines have been declared using command `AddLine3` from `beam3` elements) to write spline objects instead of line objects in the `.geo` file
- `.stl` writing format is also supported, by using extension `.stl` instead of `.geo` in the command line.
- Command option `-run` allows to run `gmsh` on the written file for meshing. All characters in the command after `-run` will be passed to the `gmsh` batch call performed. `fe_gmsh` then outputs the model processed by `gmsh`, which is usually written in `.msh` file format.

All text after the `-run` is propagated to GMSH. In particular (`-1` or `-2` or `-3`) specifies the meshing dimension. `-order 2` uses quadratic elements. `-v 0` makes a silent run.

It also possible to add a different output file name `NewFile.msh`, using `model=fe_gmsh('write NewFile.msh -run', 'FileName.stl')`.

- Conversion of files through `fe_gmsh` into `.msh`, or SDT/OpenFEM format is possible, for all input files readable by `gmsh`. Use command option `-run` and specify in second argument the file name.
For example: `model=fe_gmsh('write -run', 'FileName.stl')` convert `.stl` to `.mesh` then open into SDT/OpenFem. Some warning can occur if no

`FileName.mesh` is given, but without effect on the result.

See also `missread`

fe_load

Purpose Interface for the assembly of distributed and multiple load patterns

Syntax

```
Load = fe_load(model)
Load = fe_load(model,Case)
Load = fe_load(model,'NoT')
Load = fe_load(model,Case,'NoT')
```

Description `fe_load` is used to assemble loads (left hand side vectors to FEM problems). Simple point loads are easily built using `fe_c` and reciprocity (transpose of output shape matrix) but `fe_load` is needed for more complex cases.

Loads are associated with cases which are structures with at least `Case.DOF` and `Case.Stack` fields.

```
Case1.DOF = model.DOF; % default is model.DOF
Case1.Stack = [{'LoadType','Name',TypeSpecificData}];
```

Taking the example of a point load with type specific data given by

```
data=struct('DOF',365.03,'def',1);
```

you can create a case using low level commands

```
Case1=struct('DOF',model.DOF,'Stack',{'DofLoad','PointLoad',data});
```

or with the easier case creation format (using *SDT* function `fe_case`)

```
Case1=fe_case('DofLoad','PointLoad',data);
```

or add a new load to a case defined in the `model.Stack` field

```
model=fe_case(model,'DofLoad','PointLoad',data);
```

To compute the load, the model (a structure with fields `.Node`, `.Elt`, `.pl`, `.il`) must generally be provided with the syntax `Load=fe_load(model,Case)`. If the case is not provided, `fe_load` uses the first case in `model.Stack`.

The optional `'NoT'` argument is used to require loads defined on the full list of DOFs rather than after constraint eliminations computed using `Case.T'*Load.def`.

The rest of this manual section describes supported load types and the associated type specific data.

DofLoad, DofSet

Loads at DOFs and *DofLoad* and prescribed displacements *DofSet* entries are described by the following data structure

data.name name of the case
data.DOF column vector containing a DOF selection
data.def matrix of load/set for each DOF (each column is a load/set case and the rows are indexed by **Case.DOF**). With two DOFs, **def=[1;1]** is a single input at two DOFs, while **def=eye(2)** corresponds to two inputs.
data.lab cell array giving label, unit label , and unit info (see **fe_curve DataType**) for each load (column of **data.def**)
data.curve can specify a curve data structure (or a string referring to an existing curve) to describe frequency or time dependence of loads. Units for the load are defined through the **.lab** field (in $\{F\} = [B]\{u\}$ one assumes u to be unitless thus F and B have the same unit systems).

```
femesh('reset');  
model = femesh('testubeam plot');  
data=struct('DOF',365.03,'def',1.1); % 1.1 N at node 365 direction z  
data.lab=fe_curve('datatype',13);  
model=fe_case(model,'DofLoad','PointLoad',data);  
% alternate format to declare unit inputs  
model=fe_case(model,'DofLoad','ShortTwoInputs',[362.01;258.02]);  
Load = fe_load(model);  
feplot(model,Load); fecom(';scaleone;undefline;ch1 2') % display
```

FVol

FVol entries use **data** is a structure with fields

- `data.sel` an element selection (or a model description matrix but this is not acceptable for non-linear applications).
- `data.dir` a 3 by 1 cell array specifying the value in each global direction x, y, z. Alternatives for this specification are detailed below. The field can also be specified using `.def` and `.DOF` fields.
- `data.lab` cell array giving label, unit label, and unit info (see `fe_curve DataType`) for each load (column of `data.def`)
- `data.curve` can specify a curve data structure (or a string referring to an existing curve) to describe frequency or time dependence of loads. Units for the load are defined through the `.lab` field (in $\{F\} = [B] \{u\}$ one assumes u to be unitless thus F and B have the same unit systems).

Each cell of `Case.dir` can give a constant value, a position dependent value defined by a string `FcnName` that is evaluated using `fv(:,jDir)=eval(FcnName)` or `fv(:,jDir)=feval(FcnName,node)` if the first fails. Note that `node` corresponds to nodes of the model in the global coordinate system and you can use the coordinates `x,y,z` for your evaluation. The transformation to a vector defined at `model.DOF` is done using `vect=elem0('VectFromDir',model,r1,model.DOF)`, you can look the source code for more details.

For example

```
femesh('reset');model = femesh('testubeam');
data=struct('sel','groupall','dir',[0 32 0]);
data2=struct('sel','groupall','dir',{0,0,'(z-1).^3.*x'});
model=fe_case(model,'FVol','Constant',data, ...
              'FVol','Variable',data2);
Load = fe_load(model);
feplot(model,Load);fecom(';colordataz;ch2'); % display
```

Volume loads are implemented for all elements, you can always get an example using the elements self tests, for example `[model,Load]=beam1('testload')`.

Gravity loads are not explicitly implemented (care must be taken considering masses in this case and not volume). You should use the product of the mass matrix with the rigid body mode corresponding to a uniform acceleration.

FSurf

FSurf entries use `data` a structure with fields

`data.sel` a vector of `NodeId` in which the faces are contained (all the nodes in a loaded face/edge must be contained in the list). `data.sel` can also contain any valid node selection (using string or cell array format).

the optional `data.eltsel` field can be used for an optional element selection to be performed before selection of faces with `feutil('selelt innode',model,data.sel)`. The surface is obtained using

```
if isfield(data,'eltsel');
    mo1.Elt=feutil('selelt',mo1,data.eltsel);
end
elt=feutil('seleltinnode',mo1, ...
    feutil('findnode',mo1,r1.sel));
```

`data.set` Alternative specification of the loaded face by specifying a face `set` name to be found in `model.Stack`

`data.def` a vector with as many rows as `data.DOF` specifying a value for each DOF.

`data.DOF` DOF definition vector specifying what DOFs are loaded. Note that pressure is DOF `.19`. Uniform pressure can be defined using wild cards as show in the example below.

`data.lab` cell array giving label, unit label ,and unit info (see `fe_curve DataType`) for each load (column of `data.def`)

`data.curve` can specify a curve data structure (or a string referring to an existing curve) to describe frequency or time dependence of loads. Units for the load are defined through the `.lab` field (in $\{F\} = [B] \{u\}$ one assumes u to be unitless thus F and B have the same unit systems).

Surface loads are defined by surface selection and a field defined at nodes. The surface can be defined by a set of nodes (`data.sel` and possibly `data.eltsel` fields. One then retains faces or edges that are fully contained in the specified set of nodes. For example

```
femesh('reset');
model = femesh('testubeam plot');
data=struct('sel','x==-.5', ...
           'eltsel','withnode {z>1.25}','def',1,'DOF',.19);
model=fe_case(model,'Fsurf','Surface load',data);
Load = fe_load(model); feplot(model,Load);
```

Or an alternative call with the cell array format for `data.sel`

```
data=struct('eltsel','withnode {z>1.25}','def',1,'DOF',.19);
NodeList=feutil('findnode x==-.5',model);
data.sel={'','NodeId','==',NodeList};
model=fe_case(model,'Fsurf','Surface load',data);
Load = fe_load(model); feplot(model,Load);
```

Alternatively, one can specify the surface by referring to a `set` entry in `model.Stack`, as shown in the following example

```
femesh('reset');
model = femesh('testubeam plot');

% Define a face set
[eltid,model.Elt]=feutil('eltdifix',model);
i1=feutil('findelt withnode {x==-.5 & y<0}',model);i1=eltid(i1);
i1(:,2)=2; % fourth face is loaded
data=struct('ID',1,'data',i1);
model=stack_set(model,'set','Face 1',data);

% define a load on face 1
data=struct('set','Face 1','def',1,'DOF',.19);
model=fe_case(model,'Fsurf','Surface load',data);
Load = fe_load(model); feplot(model,Load)
```

The current trend of development is to consider surface loads as surface elements and transform the case entry to a volume load on a surface.

See also [fe_c](#), [fe_case](#), [fe_mk](#)

fe_mat

Purpose Material / element property handling utilities.

Syntax

```
out = fe_mat('convert si ba',pl);
typ=fe_mat('m_function',UnitCode,SubType)
[m_function',UnitCode,SubType]=fe_mat('type',typ)
out = fe_mat('unit')
out = fe_mat('unitlabel',UnitSystemCode)
[o1,o2,o3]=fe_mat(ElemP,ID,pl,il)
```

Description Material definitions can be handled graphically using the **Material** tab in the model editor (see section 4.2.1). For general information about material properties, you should refer to section 7.3. For information about element properties, you should refer to section 7.4.

The main user accessible commands in `fe_mat` are listed below

Convert,Unit

The `convert` command supports unit conversions to `unit1` to `unit2` with the general syntax

```
pl_converted = fe_mat('convert unit1 unit2',pl);
il_converted = fe_mat('convertil unit1 unit2',il);
```

For example convert from SI to BA and back

```
mat = m_elastic('default')
% convert mat.pl from SI unit to BA unit
pl=fe_mat('convert SIBA',mat.pl);
% check that conversion is OK
pl2=fe_mat('convert BASI',pl);mat.pl-pl2(1:6)
```

`out=fe_mat('unitsystem')` returns a **struct** containing the information characterizing standardized unit systems supported in the universal file format.

Code	Identifier	Length and Force
1	SI	Meter, Newton
2	BG	Foot, Pound f
3	MG	Meter, kilogram f
4	BA	Foot, poundal
5	MM	Millimeter, milli-newton
6	CM	Centimeter, centi-newton
7	IN	Inch, Pound force
8	GM	Millimeter, kilogram force
9	TM	Millimeter, Newton
9	US	User defined

Unit codes 1-8 are defined in the universal file format specification and thus coded in the material/element property type (column 2). Other unit systems are considered user types and are associated with unit code 9. With a unit code 9, `fe_mat convert` commands must give both the initial and final unit systems.

`out=fe_mat('unitlabel',UnitSystemCode)` returns a standardized list of unit labels corresponding in the unit system selected by the `UnitSystemCode` shown in the table above.

When defining your own properties you only need to implement the `p_fun PropertyUnitType` command to allow support of unit conversion.

Get[pl,il]

`pl = fe_mat('getpl',model)` is used to robustly return the material property matrix `pl` (see section 7.3) independently of the material input format.

Similarly `il = fe_mat('getil',model)` returns the element property matrix `il`.

Get[Mat,Pro]

`r1 = fe_mat('GetMat Param',model)` This command can be used to extract given parameter `Param` value in the model properties. For example one can retrieve density of matid 111 as following

```
rho=fe_mat('GetMat 111 rho',model);
```

Set[Mat,Pro]

```
r1 = fe_mat('SetMat MatId Param=value',model)
```

```
r1 = fe_mat('SetPro ProId Param=value',model)
```

This command can be used to set given parameter *Param* at the value *value* in the model properties. For example one can set density of matid 111 at 5000 as following `rho=fe_mat('SetMat 111 rho=5000',model);`

Type

The type of a material or element declaration defines the function used to handle it.

`typ=fe_mat('m_function',UnitCode,SubType)` returns a real number which codes the material function, unit and sub-type. Material functions are `.m` or `.mex` files whose name starts with `m_` and provide a number of standardized services as described in the `m_elastic` reference.

The `UnitCode` is a number between 1 and 9 giving the unit selected. The `SubType` is also a number between 1 and 9 allowing selection of material subtypes within the same material function (for example, `m_elastic` supports subtypes : 1 isotropic solid, 2 fluid, 3 anisotropic solid).

Note : the code type `typ` should be stored in column 2 of material property rows (see section 7.3).

```
[m_function,UnitCode,SubType]=fe_mat('typem',typ)
```

Similarly, element properties are handled by `p_` functions which also use `fe_mat` to code the type (see `p_beam`, `p_shell` and `p_solid`).

ElemP

Calls of the form `[o1,o2,o3]=fe_mat(ElemP,ID,p1,i1)` are used by element functions to request constitutive matrices. This call is really for developers only and you should look at the source code of each element.

See also `m_elastic`, `p_shell`, element functions in chapter 8

fe_mknl, fe_mk

Purpose Assembly of finite element model matrices.

Syntax

```
[m,k,mdof] = fe_mknl(model);  
[Case,model.DOF]=fe_mknl('init',model);  
mat=fe_mknl('assemble',model,Case,def,MatType);
```

Description The exact procedure used for assembly often needs to be optimized in detail to avoid repetition of unnecessary steps. SDT typically calls an internal procedure implemented in `fe_case Assemble` and detailed in section 4.5.8. This documentation is meant for low level calls.

`fe_mknl` (and the obsolete `fe_mk`) take models and return assembled matrices and/or right hand side vectors.

Input arguments are

- `model` a model data structure describing nodes, elements, material properties, element properties, and possibly a case.
- `Case` a data structure describing loads, boundary conditions, etc. This may be stored in the model and be retrieved automatically using `fe_case(model, 'GetCase')`.
- `def` a data structure describing the current state of the model for model/residual assembly using `fe_mknl`. `def` is expected to use model DOFs. If `Case` DOFs are used, they are reexpanded to model DOFs using `def=struct('def',Case.T*def.def, 'DOF')`. This is currently used for geometrically non-linear matrices.
- `MatType` or `Opt` describing the desired output, appropriate handling of linear constraints, etc.

Output formats are

- `model` with the additional field `model.K` containing the matrices. The corresponding types are stored in `model.Opt(2,:)`. The `model.DOF` field is properly filled.
- `[m,k,mdof]` returning both mass and stiffness when `Opt(1)==0`

- `[Mat,mdof]` returning a matrix with type specified in `Opt(1)`, see `MatType` below.

`mdof` is the DOF definition vector describing the DOFs of output matrices.

When fixed boundary conditions or linear constraints are considered, `mdof` is equal to the set of master or independent degrees of freedom `Case.DOF` which can also be obtained with `fe_case(model,'gettdof')`. Additional unused DOFs can then be eliminated unless `Opt(2)` is set to 1 to prevent that elimination. To prevent constraint elimination in `fe_mkn1` use `Assemble NoT`.

In some cases, you may want to assemble the matrices but not go through the constraint elimination phase. This is done by setting `Opt(2)` to 2. `mdof` is then equal to `model.DOF`.

This is illustrated in the example below

```
femesh('reset');
model =femesh('testubeam');
model.DOF=[];% an non empty model.DOF would eliminate all other DOFs
model =fe_case(model,'fixdof','Base','z==0');
model = fe_mk(model,'Options',[0 2]);
[k,mdof] = fe_mk(model,'options',[0 0]);
fprintf('With constraints %i DOFs\n',size(k,1));
fprintf('Without          %i DOFs',size(model.K{1},1));
Case=fe_case(model,'gett');
isequal(Case.DOF,mdof) % mdof is the same as Case.DOF
```

For other information on constraint handling see section 7.14.

Assembly is decomposed in two phases. The initialization prepares everything that will stay constant during a non-linear run. The assembly call performs other operations.

Init

The `fe_mkn1 Init` phase initializes the `Case.T` (basis of vectors verifying linear constraints see section 7.14), `Case.GroupInfo` fields (detailed below) and `Case.MatGraph` (preallocated sparse matrix associated with the model topology for optimized (re)assembly). `Case.GroupInfo` is a cell array with rows giving information about each element group in the model (see section 7.15.3 for details).

Command options are the following

- `NoCon Case = fe_mknl('initNoCon', model)` can be used to initialize the case structure without building the matrix connectivity (sparse matrix with preallocation of all possible non zero values).
- `Keep` can be used to prevent changing the `model.DOF` DOF list. This is typically used for submodel assembly.
- `-NodePos` saves the `NodePos` node position index matrix for a given group in its `EltConst` entry.
- `-gstate` is used force initialization of group stress entries.

The initialization phase is decomposed into the following steps

1. Generation of a complete list of DOFs using the `feutil('getdof',model)` call.
2. get the material and element property tables in a robust manner (since some data can be replicated between the `pl,il` fields and the `mat,pro` stack entries. Generate node positions in a global reference frame.
3. For each element group, build the `GroupInfo` data (DOF positions).
4. For each element group, determine the unique pairs of `[MatId ProId]` values in the current group of elements and build a separate `integ` and `constit` for each pair. One then has the constitutive parameters for each type of element in the current group. `pointers` rows 6 and 7 give for each element the location of relevant information in the `integ` and `constit` tables.

This is typically done using an `[integ,constit,ElMap]=ElemF('integinfo')` command, which in most cases is really being passed directly to a `p_fun('BuildConstit')` command.

`ElMap` can be a structure with fields beginning by `RunOpt_`, `Case_` and `eval` which allows execution of specific callbacks at this stage.

5. For each element group, perform other initializations as defined by evaluating the callback string obtained using `elem('GroupInit')`. For example, initialize integration rule data structures `EltConst`, define local bases or normal maps in `InfoAtNode`, allocate memory for internal state variables in `gstate`, ...
6. If requested (call without `NoCon`), preallocate a sparse matrix to store the assembled model. This topology assumes non zero values at all components of element matrices so that it is identical for all possible matrices and constant during non-linear iterations.

Assemble [, NoT]

The second phase, assembly, is optimized for speed and multiple runs (in non-linear sequences it is repeated as long as the element connectivity information does not change). In `fe_mk` the second phase is optimized for robustness. The following example illustrates the interest of multiple phase assembly

```
femesh('reset');
model =femesh('test hexa8 divide 100 10 10');
% traditional FE_MK assembly
tic;[m1,k1,mdof] = fe_mk(model);toc

% Multi-step approach for NL operation
tic;[Case,model.DOF]=fe_mknl('init',model);toc
tic;
m=fe_mknl('assemble',model,Case,2);
k=fe_mknl('assemble',model,Case,1);
toc
```

MatType

Matrix types are numeric indications of what needs to be computed during assembly. Currently defined types for OpenFEM are

- **0** mass and stiffness assembly. **1** stiffness, **2** mass, **3** viscous damping, **4** hysteretic damping, **5** tangent stiffness in geometric non-linear mechanics.
- **3** viscous damping. Uses `info,Rayleigh` case entries if defined, see example in section 5.3.2.
- **4** hysteretic damping. Weighs the stiffness matrices associated with each material with the associated loss factors. These are identified by the key word `Eta` in `PropertyUnitType` commands.
- **7** gyroscopic coupling in the body fixed frame, **70** gyroscopic coupling in the global frame. **8** centrifugal softening.
- **9** is reserved for non-symmetric stiffness coupling (fluid structure, contact/friction, ...);
- **100** volume load, **101** pressure load, **102** inertia load, **103** initial stress load. Note that some load types are only supported with the `mat_og` element family;

- 200 stress at node, 201 stress at element center, 202 stress at gauss point
- 251 energy associated with matrix type 1 (stiffness), 252 energy associated with matrix type 2 (mass), ...
- 300 compute initial stress field associated with an initial deformation. This value is set in `Case.GroupInfo{jGroup,5}` directly (be careful with the fact that such direct modification INPUTS is not a MATLAB standard feature).
301 compute the stresses induced by a thermal field. For pre-stressed beams, 300 modifies `InfoAtNode=Case.GroupInfo{jGroup,7}`.

NodePos

`NodePos=fe_mknl('NodePos',NNode,elt,cEGI,ElemF)` is used to build the node position index matrix for a given group. `ElemF` can be omitted. `NNode` can be replaced by `node`.

OrientMap

This command is used to build the `InfoAtNode` entry. The `RunOpt.EltOrient` field is a possibly empty stack containing appropriate information before step 5 of the `init` command.

A first mechanism to fill in the orientation information is to define `MAP,Groupi` stack entries in the model. This can lead to errors if element groups are modified.

fe_mk (obsolete)

Syntax

```
model      = fe_mk(model,'Options',Opt)
[m,k,mdof] = fe_mk( ... ,[0      OtherOptions])
[mat,mdof] = fe_mk( ... ,[MatType OtherOptions])
```

`fe_mk` options are given by calls of the form `fe_mk(model,'Options',Opt)` or the obsolete `fe_mk(node,elt,pl,il,[],adof,opt)`.

- `opt(1)` `MatType` see above
- `opt(2)` if active DOFs are specified using `model.DOF` (or the obsolete call with `adof`), DOFs in `model.DOF` but not used by the model (either linked to no element or with a zero on the matrix or both the mass and stiffness diagonals) are eliminated unless `opt(2)` is set to `1` (but case constraints are then still considered) or `2` (all constraints are ignored).
- `opt(3)` Assembly method (0 default, 1 symmetric mass and stiffness (OBSOLETE), 2 disk (to be preferred for large problems)). The disk assembly method creates temporary files using the `sdtdef tempname` command. This minimizes memory usage so that it should be preferred for very large models.
- `opt(4)` `0` (default) nothing done for less than 1000 DOF method 1 otherwise. `1` DOF numbering optimized using current `ofact SymRenumber` method. Since new solvers renumber at factorization time this option is no longer interesting.

`[m,k,mdof]=fe_mk(node,elt,pl,il)` returns mass and stiffness matrices when given nodes, elements, material properties, element properties rather than the corresponding model data structure.

`[mat,mdof]=fe_mk(node,elt,pl,il,[],adof,opt)` lets you specify DOFs to be retained with `adof` (same as defining a `Case` entry with `{'KeepDof', 'Retained', adof}`).

These formats are kept for backward compatibility but they do not allow support of local coordinate systems, handling of boundary conditions through cases, ...

Notes

`fe_mk` no longer supports complex matrix assembly in order to allow a number of speed optimization steps. You are thus expected to assemble the real and imaginary parts successively.

See also

Element functions in chapter 8, `fe_c`, `feplot`, `fe_eig`, `upcom`, `fe_mat`, `femesh`, etc.

fe_norm

Purpose

Mass-normalization and stiffness orthonormalization of a set of vectors.

Syntax

```
To = fe_norm(T,m)
[rmode,wr] = fe_norm(T,m,k,NoCommentFlag)
[rmode,wr] = fe_norm(T,m,k,tol)
```

Description

With just the mass **m** (**k** not given or empty), **fe_norm** orthonormalizes the **T** matrix with respect to the mass **m** using a preconditioned Cholesky decomposition. The result **To** spans the same vector space than **T** but verifies the orthonormal condition

$$[To]^T [M]_{N \times N} [To]_{N \times NM} = [I]_{NM \times NM}$$

If some vectors of the basis **T** are collinear, these are eliminated. This elimination is a helpful feature of **fe_norm**.

When both the mass and stiffness matrices are specified a reanalysis of the reduced problem is performed (eigenvalue structure of model projected on the basis **T**). The resulting reduced modes **rmode** not only verify the mass orthogonality condition, but also the stiffness orthogonality condition (where $[\Omega_{j\setminus}^2] = \text{diag}(wr.^2)$)

$$[\phi]^T [K] [\phi] = [\Omega_{j\setminus}^2]_{NM \times NM}$$

The verification of the two orthogonality conditions is not a sufficient condition for the vectors **rmode** to be the modes of the model. Only if $NM = N$ is this guaranteed. In other cases, **rmode** are just the best approximations of modes in the range of **T**.

When the fourth argument **NoCommentFlag** is a string, no warning is given if some modes are eliminated.

When a tolerance is given, frequencies below the tolerance are truncated. The default tolerance (value given when **tol=0**) is product of **eps** by the number of modes by the smallest of **1e3** and the mean of the first seven frequencies (in order to incorporate at least one flexible frequency in cases with rigid body modes). This truncation helps prevent poor numerical conditioning from reduced models with a dynamic range superior to numerical precision.

See also

[fe_reduc](#), [fe_eig](#)

fe_quality

Purpose Mesh quality measurement tools

Description This function provides mesh quality measurement, visualization and report tools. Badly shaped elements are known to cause computation error and imprecision, and basic geometric tests can help to acknowledge such property. Every element cannot be tested the same way therefore the `lab` command presents the tests available for each kind. The geometric measurements performed are described in the following section.

An integrated call is provided for `fepplot`,

```
fe_quality(cf.mdl);
```

This call performs all test available and opens a GUI allowing the user to customize the views.

Available tests

Degenerate

Degenerated elements have overlaying nodes which is generally unwanted. The set is automatically generated when such elements are detected.

Jacobian

This test computes the minimum jacobian for each element and detects negative values. It is directly related to the element volume so that a wrapped element would show such pattern. The set is generated only if elements with negative jacobians are detected.

AspectRatio

This test can be applied to any kind of element. It computes the ratio of the longest edge length to the shortest one. Thus a good element will have an aspect ratio close to one while a badly shaped element will have a greater aspect ratio. The Default tolerance for visualization is set to 2.

MaxIntAng

This test can be applied to triangle and quadrangle elements ([tria3](#), [tria6](#), [quad4](#), [quad8](#), [quadb](#)). It measures the greatest angle in an element which is an indication of element distortion. The default tolerance is set to 110 degrees.

GammaK

This test is applied to triangle elements ([tria3](#), [tria6](#)). It computes the ratio between the radius of the inscribed circle and the circumcircle. This indicator is named γ_K and is bounded between 0 and 1. Well shaped elements will have a γ_K coefficient close to one. Degenerated triangles show $\gamma_K = 0$. The default tolerance is set to 0.5.

MidNodeEgde

This test is applied to quadratic triangles ([tria6](#)). It measures the distance of the middle nodes to the edge nodes. The ratio between the distance from the middle node to the first edge node (l_1) and the distance from the middle node to the second edge node (l_2) is computed for each element as $MNE = \max_{i=1...3} \left(\frac{\max(l_{1i}/l_{2i}, l_{2i}/l_{1i})}{\min(l_{1i}/l_{2i}, l_{2i}/l_{1i})} \right)$. The default tolerance is set to 1.5.

MaxAngleMid2Edge

This test is applied to quadratic triangles ([tria6](#)). It measures the distortion of the edges by computing the maximum angle between the straight edge (between both edge extreme nodes) and the actual edges through the middle node. The maximum over the whole triangle is output, the default tolerance is set to 30 degrees.

Taper

This test is applied to 2D quadrangle elements ([quadb](#)). It compares the areas of the 4 triangles formed by the diagonals and each edge to the area of the full quadrangle. The exact computation is $\max(\frac{2A_i}{A_K})$. Thus a well shaped element will show a taper ratio close to 0.5, while a badly shaped element can have taper ratios over 1. The default tolerance is set to 0.8.

Skew

This test is applied to quadrangle elements ([quad4](#), [quad8](#), [quadb](#)). It evaluates the element distortion by measuring the angle formed by the diagonals (the maxi-

mum angle is taken). A square will then show a skew angle of 90 degrees, while a distorted element will show angles over 150 degrees. The default tolerance is set to 110 degrees.

Wrap

This test is applied to quadrangle elements ([quad4](#), [quad8](#), [quadb](#)). It measures the coplanarity of the 4 vertices by comparing the height of the 4th point to the plan generated by the first three points (H), relatively to the element dimension. The exact formulation is $W = \frac{H}{l(D_1)+l(D_2)}$. Perfectly planar elements will have a null wrap coefficient. The default tolerance is set to 10^{-2}

RadiusEdge

This test is applied to tetrahedron elements ([tetra4](#), [tetra10](#)). It measures the ratio between the radius of the circumsphere to the minimum edge length of a tetrahedron. Well shaped elements will show a small value while badly shaped elements will show far greater values. The radius edge coefficient is lower bounded by the radius edge ratio of the regular tetrahedron: $RE \geq \frac{\sqrt{6}}{4}$. The default maximum value is set to 2, which usually is sufficient to have a quality mesh. Sliver elements may not be detected by this measure.

Sliver

This test is applied to tetrahedron elements ([tetra4](#), [tetra10](#)). A sliver element is a nearly flat tetrahedron, such pathology can lead to bad conditioning due to the very small volumes that can be engendered by these particular elements. This is well detected by computing the ratio between the maximum edge length to the minimum altitude (from a vertice to the opposed face). Sliver elements will have large values and possibly infinite if degenerated. The degenerated elements are set to a value of 10^5 for visualization, the default tolerance is set to 10.

FaceAspect

This can be applied to hexahedron and pentahedron elements ([hexa8](#), [hexa20](#), [penta6](#), [penta15](#)). It measures the aspect ratio of each face of the elements. The default tolerance is set to 2.

Unstraight

This can be applied to any element with middle nodes. It measures the euclidean distance between the edge middle (if the edge were straight) and the actual position of the middle edge node. Tolerance is set at 0.1.

RadiusCircum

This measure can only be accessed separately, with an explicit specification in the `meas` command. It measures the circum radiuses of triangle elements. This is applicable to `tria3` and `tria6` elements.

Commands

lab[...]

Outputs or prints the tests available and their default tolerance. If no output is asked this is printed to the prompt. `fe_quality('lab')` outputs the list of element tested with the command for detailed information. `fe_quality('lab EltName)` prints the tests available for the element *EltName* and the default tolerances associated.

meas[...]

Computes the mesh quality measurements. For a `feplot` model, the results are stored in the stack under the entry `'info','MeshQual'`. The results are given by element groups unless a specific element selection is given as a third argument. Accepted calls are `MQ = fe_quality('meas',model)`; Computes all available tests per element group.

`MQ = fe_quality('meas -view MName',model)`; Computes the *MName* test and visualize it.

`MQ = fe_quality('meas',model,'EltSel')` Computes all measurement tests for the specified *EltSel* element selection.

`MQ` is the mesh quality output. It is a structure of fields `eltid`, `data` and `lab`. All fields are cell arrays of the same size related to the measures described in the lab entry as `MName_ElemF_EGID` for which corresponding `EltId` and measurement values (`data`) are given. Direct visualization of the results can be obtained with the `-view` option.

`view[...]`

Performs a visualization of the quality measurements of a `feplot` model. The stack entry `'info', 'MeshQual'` must exist (created by `meas`). Two `feplot` selections are generated. First the elements are face colored in transparency with a colored ranking. Second, the elements outside the measurement tolerance are plotted in white patches of full opacity. Both plots generate an `EltSet`, the elements plotted are stored in `'set', 'MeshQual_eltset'`, the elements outside tolerance are stored in `'set', 'MeshQual_MName_tol_val'` with `MName` the test considered and `val` the tolerance value.

The tolerance can be defined using the command option `-tol val`. A positive (resp. negative) tolerance `val` defines pathologic elements over (resp. under) the threshold.

Command option `-noGlobalMesh` customizes the selection so that the global mesh in transparency is not displayed.

It is possible to plot a sub selection of the elements measured by specifying an `EltSel` as third argument. The curve `colordataelt` plot can also be output.

`fe_quality('view');` Default visualization, `AspectRatio` is plotted as it is available for every element.

`fe_quality('view MName -tol val', cf);` `feplot` pointer, `MName` and tolerance `val` test are specified.

`fe_quality('view', cf, EltSel);` An additional element selection `EltSel` to restrict the mesh quality measurement plot.

MeshDim

`fe_quality('MeshDim', model)` returns a line vector `[weight average min max]` giving an indication on the mesh dimensions. The mesh edge lengths of all elements are computed, and the average, min and max data are output.

Command option `-print` allows printing this data in a human readable format to the output display.

print

Prints out the mesh quality report sorted in `'info', 'MeshQual'` of a model or a `feplot` figure. By default the results are printed to the prompt, a specific file can be given in the print command. *E.g.*

`fe_quality('print myMeshQualityReport', model);`

`clear[...]`

This command clears the element quality visualization and can also clean up the stack of any element sets created during the `view` procedures. All entries created by `fe_quality` in the model Stack are of the `'info'` or `'set'` type with a name starting by `MeshQual`.

`fe_quality('clear')` clears the `feplot` selection and visualization.

`fe_quality('clearall')` clears the visualization and removes every stack entry concerning mesh quality.

`fe_quality('clear MName')` removes from the stack a specified `MName` measurement visualization.

fe_reduc

Purpose Utilities for finite element model reduction.

Syntax

```
SE = fe_reduc('command options',model)
TR = fe_reduc('command options',model)
```

Description `fe_reduc` provides standard ways of creating and handling bases (rectangular matrix **T**) of real vectors used for model reduction (see details in section 6.2). Input arguments are a command detailed later and a model (see section 7.6). Obsolete low level calls are detailed at the end of this section. Generic options for the command are

- `-matdes` can be used to specify a list of desired matrices. Default values are `-matdes 2 1` for mass and stiffness, see details in section 4.5.8.
- `-SE` is used to obtain the output (reduced model) as a superelement SE. Details about the fields of superelement data-structures are given section section 6.3.2.
- `model.Dbfile` can be used to specify a `-v7.3 .mat` file to be used as database for out of core operations.
- `-hdf` is used to request the use of out of core operations.

Accepted `fe_reduc` commands are

Static, CraigBampton

`Static` computes *static* or *Guyan condensation*. `CraigBampton` appends fixed interface modes to the static condensation.

Given a set of interface DOFs, indexed as I , and other DOFs C , the static responses to unit displacements are given by

$$[T] = \begin{bmatrix} T_I \\ T_C \end{bmatrix} = \begin{bmatrix} I \\ -K_{CC}^{-1}K_{CI} \end{bmatrix}$$

which is the static basis (also called *constraint modes* in the Component Mode Synthesis literature). For Craig-Bampton (6.94), one appends fixed interface modes (with $q_I = 0$). Note that you may get an error if the interface DOFs do not constrain rigid body motion so that K_{CC} is singular.

The interface DOFs should be specified using a `DOFSet` case entry. The interface DOFs are defined as those used in the `DofSet`. The complementary DOF are determined by exclusion of the interface DOF from the remaining active DOFs.

```
model=demosdt('volbeam');
% Define interface to be xyz DOF at nodes 2,3
model=fe_case(model,'DofSet','IN', ...
    feutil('getdof',[2;3],[.01;.02;.03]));
% statically reduced model
ST=fe_reduc('Static',model);
% For Craig Bampton specify eigenvalue options
model=stack_set(model,'info','EigOpt',[5 10 0]);
CB=fe_reduc('CraigBampton',model);
```

Available command options are

- `NM` is the number of desired modes, which *should be specified* in an `info,EigOpt` stack entry which allow selection of the eigenvalue solver (default is 5, Lanczos). Note that using `NM=0` corresponds to static or Guyan condensation.
- `-SE` is used to obtain the output as a superelement SE. Without this argument, outputs are the rather obsolete list `[T,sdof,f,mr,kr]` where `f` is the frequency of fixed interface modes.
- `-shift` allows the use of a non-zero shift in the eigenvalue solution for the fixed interface modes. The interior matrix K_{cc} is only factored once, so using a shifted matrix may result in poor estimates of rigid body modes.
- `-useDOF` recombines the fixed interface modes to associate shape with a specific interior DOF. This can ease the manipulation of the resulting model as a superelement.
- `-drill`. Shell elements may not always use drilling stiffness (5 DOF rather than 6), which tends to cause problems when using 6 DOF interfaces. The option calls `model.il=p_shell('SetDrill 0',model.il)` to force the default 6 DOF formulations.
- `-Load` appends static correction for defined loads to the model.

```
mdl=fesuper(mdl,'setTR',name,'fe_reduc command') calls fe_reduc to assemble and reduce the superelement. For example
mdl=fesuper(mdl,'SetTR','SE1','CraigBampton -UseDof -drill');
```

Free [, Float] [, -dterm] EigOpt

The standard basis for modal truncation with static correction discussed in section 6.2.3 (also known as McNeal reduction). Static correction is computed for the loads defined in the model case (see `fe_case`). Accepted options are

- *EigOpt* should be specified in an `info,EigOpt` stack entry. For backward compatibility these `fe_eig` options can be given in the command and are used to compute the modeshapes. In the presence of rigid body modes you must provide a mass shift.
- `Float` is used to obtain the standard attachment modes (6.90) in the presence of rigid body modes. Without this option, `fe_reduc` uses shifted attachment modes (6.91), when a non zero shift is given in *EigOpt*. This default is typically much faster since the shifted matrix need not be refactored, but may cause problem for relatively large negative shifts.

dynamic w

`[T,rbdof,rb]=fe_reduc('dynamic freq', ...)` computes the dynamic response at frequency w to loads \mathbf{b} . This is really the same as doing $(-w^2\mathbf{m}+\mathbf{k})\backslash\mathbf{b}$ but can be significantly faster and is more robust.

flex [,nr]

`[T,rbdof,rb]=fe_reduc('flex', ...)` computes the static response of flexible modes to load \mathbf{b} (which can be given as `bdof`)

$$\left[K_{Flex}^{-1} \right] [b] = \sum_{j=NR+1}^N \frac{\{\phi_j\} \{\phi_j\}^T}{\omega_j^2}$$

where NR is the number of rigid body modes. These responses are also called static flexible responses or **attachment modes** (when forces are applied at interface DOFs in CMS problems).

The flexible response is computed in three steps:

- Determine the flexible load associated to b that does not excite the rigid body modes $b_{Flex} = ([I] - [M\phi_R] [\phi_R^T M \phi_R]^{-1} [\phi_R^T]) [b]$

- Compute the static response of an isostatically constrained model to this load

$$[q_{Iso}] = \begin{bmatrix} 0 & 0 \\ 0 & K_{Iso}^{-1} \end{bmatrix} [b_{Flex}]$$

- Orthogonalize the result with respect to rigid body modes

$$q_{Flex} = ([I] - [\phi_R] [\phi_R^T M \phi_R]^{-1} [\phi_R^T M]) [q_{Iso}]$$

where it clearly appears that the knowledge of rigid body modes and of an isostatic constraint is required, while the knowledge of all flexible modes is not (see [36] for more details).

By definition, the set of degrees of freedom R (with other DOFs noted Iso) forms an isostatic constraint if the vectors found by

$$[\phi_R] = \begin{bmatrix} \phi_{RR} \\ \phi_{IsoR} \end{bmatrix} = \begin{bmatrix} I \\ -K_{Iso}^{-1} K_{IsoR} \end{bmatrix}$$

span the full range of rigid body modes (kernel of the stiffness matrix). In other words, displacements imposed on the DOFs of an isostatic constraint lead to a unique response with no strain energy (the imposed displacement can be accommodated with a unique rigid body motion).

If no isostatic constraint DOFs `rdof` are given as an input argument, a `lu` decomposition of `k` is used to find them. `rdof` and rigid body modes `rb` are always returned as additional output arguments.

The command `flexnr` can be used for faster computations in cases with no rigid body modes. The static flexible response is then equal to the static response and `fe_reduc` provides an optimized equivalent to the MATLAB command `k\b`.

rb

`[rb,rbdof]=fe_reduc('rb',m,k,mdof,rbdof)` determines rigid body modes (rigid body modes span the null space of the stiffness matrix). The DOFs `rbdof` should form an isostatic constraint (see the `flex` command above). If `rbdof` is not given as an input, an `LU` decomposition of `k` is used to determine a proper choice.

If a mass is given (otherwise use an empty `[]` mass argument), computed rigid body modes are mass orthonormalized ($\phi_R^T M \phi_R = I$). Rigid body modes with no mass are then assumed to be computational modes and are removed.

`m,k,mdof` (obsolete format)

Low level calling formats where matrices are provided are still supported but should be phased out since they do not allow memory optimization needed for larger models.

- `m` mass matrix (can be empty for commands that do not use mass)
- `k` stiffness matrix and
- `mdof` associated DOF definition vector describing DOFs in `m` and `k`. When using a model with constraints, you can use `mdof=fe_case(model,'gettdof')`.
- `b` input shape matrix describing unit loads of interest. Must be coherent with `mdof`.
- `bdof` alternate load description by a set of DOFs (`bdof` and `mdof` must have different length)
- `rdof` contains definitions for a set of DOFs forming an iso-static constraint (see details below). When `rdof` is not given, it is determined through an LU decomposition done before the usual factorization of the stiffness. This operation takes time but may be useful with certain elements for which geometric and numeric rigid body modes don't coincide.

For `CraigBampton`, the calling format was `fe_reduc('CraigBampton NM Shift Option`

See also `fe2ss`, `fe_eig`, section 6.2

fe_sens

Purpose	Utilities for sensor/shaker placement and sensor/DOF correlation.
Syntax	Command dependent syntax. See sections on placement and correlation below.
Placement	In cases where an analytical model of the structure is available before a modal test, you can use it for test preparation, see section 3.1.3 and the associated gartsens demo. fe_sens provides sensor/shaker placement methods.

indep

`sdof=fe_sens('indep',DEF)` uses the effective independence algorithm [14] to sort the selected sensors in terms of their ability to distinguish the shapes of the considered modes. The output `sdof` is the DOF definition vector `cdof` sorted according to this algorithm (the first elements give the best locations).

See example in the [gartsens](#) demo. The `mseq` algorithm is much faster and typically gives better results.

mseq

`sdof = fe_sens('mseq Nsens target',DEF,sdof0)` places *Nsens* sensors, with an optional initial set *sdof0*. The maximum response sequence algorithm used here can only place meaningfully NM (number of modes in DEF) sensors, for additional sensors, the algorithm tries to minimize the off-diagonal auto-MAC terms in modes in `DEF.def` whose indices are selected by *target*.

```
[FEM,def]=demosdt('demo gartfe');
def=fe_def('subdef',def,6:15); % Keep ten modes
d1=fe_def('subdof',def,[.01;.02;.03]) % Keep translations
% Select subpart as target location
d1=fe_def('subdof',d1,feutil('findnode group 4:6',FEM));
sdof= fe_sens('mseq 10',def);
FEM=fe_case(FEM,'sensdof','Test',sdof);
feplot(FEM);fecom('curtabCase:Test');fecom('previewOn');
% see also garsens demo
```

[[ma](#),[mmif](#)]

```
[sdof,load] = fe_sens('ma val',po,cphi,IndB,IndPo,Ind0)
```

Shaker placement based on most important components for force appropriation of a mode. The input arguments are poles `po`, modal output shape matrix `cphi`, indices `IndB` of sensor positions where a collocated force could be applied, `IndPo` tells which mode is to be appropriated with the selected force pattern. `Ind0` can optionally be used to specify shakers that must be included.

`sdof(:,1)` sorts the indices `IndB` of positions where a force can be applied by order of importance. `sdof(:,2)` gives the associated MMIF. `load` gives the positions and forces needed to have a MMIF below the value `val` (default 0.01). The value is used as a threshold to stop the algorithm early.

`ma` uses a sequential building algorithm (add one position a time) while `mmif` uses a decimation strategy (remove one position at a time).

Correlation

`fe_sens` provides a user interface that helps obtaining test/analysis correlation for industrial models. To get started you can refer to the following sections

- defining a wire-frame with translation sensors in section 2.2.1 and section 2.2.2
- adding sensors to a FEM as a `SensDof` entry is illustrated in the topology correlation tutorial section 3.1.

Commands supported by `fe_sens` are

basis

These commands are used to handle cases where the test geometry is defined in a different frame than the FEM. An example is detailed in section 3.1.2.

`BasisEstimate` guesses a local coordinate system for test nodes that matches the FEM model reasonably and displays the result in a fashion that lets you edit the estimated basis. Arguments are the model, and the name of the `SensDof` entry containing a test frame.

```
model = fe_sens('basisEstimate',model,'Test');
```

A list of node pairs in the FEM and test frames can be provided as an additional argument to improve results. The list is a two columns matrix containing FEM (resp. test) `NodeId` in the first (resp. second) column. If four nodes are provided, the estimation is an exact triplet positioning, the first node being the origin and the

3 other being directions (must be non colinear). For shorter or longer node lists, the positioning is based on global distance minimization between paired nodes.

`Basis` is used to set the local test basis in a script (see example in section 3.1.2).

`BasisToFEM` is used to transform the `SensDof` entry to FEM coordinates. This transformation is done after basis adjustment and makes verification easier by clarifying the fact that the `sens.tdof` uses the 5 column format with measurement directions given in the FEM format. The only reference to test is the identifier in `sens.tdof(:,1)` which is kept unchanged and thus where a 1.01 will refer to test direction x which may be another direction in the FEM.

`SensMatch, sens, ...`

The building of observation matrices for `SensDof` entries is now described under `sensor SensMatch` (building topology correlation to locate test nodes in the FEM model) and `sensor Sens` (building of the observation matrix after matching). Please read section 4.3.4 for more details.

The obsolete `near,rigid,arigid` commands are supported through `SensMatch` calls.

`tdof`

`tdof = fe_sens('tdof',sens.tdof)` returns the 5 column form of `tdof` if `sens.tdof` is defined as a DOF definition vector. For more details see `sens.tdof` and section 4.3 for general sensor definitions.

`links`

`fecom('ShowLinks Sensors')` generates a plot with the mode wire-mesh associated with the `SensDof` entry `Sensors`.

For older models where the wire frame is included in the model with a negative `EGID`, `fecom('ShowLinks')` still generates a standard plot showing the FEM as a gray mesh, the test wire-frame as a red mesh, test/FEM node links as green lines with end circles, and rotation interpolation links as blue lines with cross markers.

`stick`

The `stick` command can be used to find an orthonormal projection of the test nodes onto the nearest FEM surface. The projected nodes are found in the `match.StickNode` field.


```
[sens,def]=demosdt('demo gartte cor');
match=fe_sens('stick sensors',sens,'selface');
```

WireExp

`def = fe_sens('wireexp',sens)` uses the wire-frame topology define in `sens` to create an interpolation for un-measured directions. For a tutorial on this issue see section 3.3.2.

The following example applies this method for the GARTEUR example. You can note that the in-plane bending mode (mode 8) is clearly interpolated with this approach (the drums of the green deformation have global motion rather than just one point moving horizontally).

```
[TEST,test_mode]=demosdt('demo gartte wire');
TR=fe_sens('wireexp',TEST);
cf=feplot;cf.model=TEST;fe_sens('WireExpShow',cf,TR)
pause %Use +/- to scan trough deformations as a verification

cf.def(1)=test_mode;
cf.def(2)={test_mode,TR};
fecom(';show2def;ScaleEqual;ch8;view2');
legend(cf.o(1:2),'Nominal','Wire-exp')
```

The command builds default properties associated with the wire frame (beams properties for segments, shells properties for surfaces, elastic properties for volumes). In some cases you may get better properties by defining properties yourself (see section 7.4 and section 7.3).

Section 4.3, `femesh`, `fe_exp`, `fe_c,ii_mac`, `ii_comac`

fe_simul

Purpose High level access to standard solvers.

Syntax `[Result,model] = fe_simul('Command',MODEL,OPT)`

Description `fe_simul` is the generic function to compute various types of response. It allows an easy access to specialized functions to compute static, modal (see `fe_eig`) and transient (see `fe_time`) response. A tutorial may be found in section 4.5.

Once you have defined a FEM model (section 4.2), material and elements properties (section 4.2.1), loads and boundary conditions (section 4.2.3), calling `fe_simul` assembles the model (if necessary) and computes the response using the dedicated algorithm.

Note that you may access to the `fe_simul` commands graphically with the simulate tab of the feplot GUI. See tutorial (section 4.5) on how to compute a response.

Input arguments are :

- **MODEL** a standard FEM model data structure with loads, boundary conditions, ... defined in the case. See section 4.2 (tutorial), `fe_case` for boundary conditions, `fe_load` for loads, ...
- **OPT** is an option vector or data structure used for some solutions. These may also be stored as `model.Stack` entries.

Accepted commands are

- **Static**: computes the static response to loads defined in the Case. no options are available for this command

```
model = demosdt('demo ubeam');cf=feplot;cf.model=model;
data = struct('sel','GroupAll','dir',[1 0 0]);
model = fe_case(model,'FVol','Volume load',data);
[cf.def,model]=fe_simul('static',model);
```

- **Mode** : computes normal modes, `fe_eig` options can be given in the command string or as an additional argument. For modal computations, `opt=[method nm Shift Print Thres]` (it is the same vector option as for `fe_eig`). This an example to compute the first 10 modes of a 3D beam :

```

model = demosdt('demo ubeam');cf=fepplot;cf.model=model;
model=stack_set(model,'info','EigOpt',[6 10 0 11]);
[cf.def,model]=fe_simul('mode',model);

```

- **DFRF**: computes the direct response to a set of input/output at the frequencies defines in Stack.

```

femesh('reset'); model = femesh('testubeamt');
model=fe_case(model,'FixDof','Clamped end','z==0');
r1=struct('DOF',365.03,'def',1.1); % 1.1 N at node 365 direction z
model=fe_case(model,'DofLoad','PointLoad',r1);
model= stack_set(model,'info','Freq',1:10);
def=fe_simul('DFRF',model);

```

One can define a frequency dependence of the load using a curve (see section 7.9 for more detail). For example:

```

model=fe_curve(model,'set','input','Testeval (2*pi*t).^2');
model=fe_case(model,'setcurve','PointLoad','input');

```

- **Time** : computes the time response. You must specify which algorithm is used (**Newmark**, Discontinuous Galerkin **dg**, **Newton**, **Theta**, or **NLNewmark**). For transient computations, **opt= [beta alpha t0 deltaT Nstep]** (it is the same vector option as for **fe_time**). Calling time response with **fe_simul** does not allow initial condition. This is an example of a 1D bar submitted to a step input :

```

model=demosdt('demo bar');
[def,model]=fe_simul('time newmark',model,[.25 .5 0 1e-4 50]);
def.DOF=def.DOF+.02;
cf=fepplot;cf.model=model;cf.def=def;
fecom(';view1;animtime;ch20');

```

See also [fe_eig](#), [fe_time](#), [fe_mk](#)

fe_stress

Purpose Computation of stresses and energies for given deformations.

Syntax

```
Result = fe_stress('Command',MODEL,DEF)
... = fe_stress('Command',node,elt,pl,il, ...)
... = fe_stress( ... ,mode,m dof)
```

Description You can display stresses and energies directly using `fecom ColorDataEner` commands and use `fe_stress` to analyze results numerically. `MODEL` can be specified by four input arguments `node`, `elt`, `pl` and `il` (those used by `fe_mk`, see also section 7.1 and following), a data structure with fields `.Node`, `.Elt`, `.pl`, `.il`, or a database wrapper with those fields.

The deformations `DEF` can be specified using two arguments: `mode` and associated DOF definition vector `mdof` or a structure array with fields `.def` and `.DOF`.

Ener [m,k] *ElementSelection*

Element energy computation. For a given shape, the levels of strain and kinetic energy in different elements give an indication of how much influence the modification of the element properties may have on the global system response. This knowledge is a useful analysis tool to determine regions that may need to be updated in a FE model. Accepted command options are

- `-MatDesval` is used to specify the matrix type (see `MatType`). `-MatDes 5` now correctly computes energies in pre-stressed configurations.
- `-curve` should be used to obtain energies in the newer curve format. `Ek.X{1}` gives as columns `EltId`, `vol`, `MatId`, `ProId`, `GroupId` so that passage between energy and energy density can be done dynamically.
- `ElementSelection` (see the element selection commands) used to compute energies in part of the model only. The default is to compute energies in all elements.

Obsolete options are

- `m`, `k` specify computation of kinetic or strain energies. For backward compatibility, `fe_stress` returns `[StrainE,KinE]` as two arguments if no element selection is given.

- `dens` changes from the default where the element energy and **not** energy density is computed. This may be more appropriate when displaying energy levels for structures with uneven meshes.
- Element energies are computed for deformations in `DEF` and the result is returned in the data structure `RESULT` with fields `.data` and `.EltId` which specifies which elements were selected. A `.vol` field gives the volume or mass of each element to allow switching between energy and energy density.

The strain and kinetic energies of an element are defined by

$$E_{strain}^e = \frac{1}{2}\phi^T K_{element}\phi \quad \text{and} \quad E_{kinetic}^e = \frac{1}{2}\phi^T M_{element}\phi$$

For complex frequency responses, one integrates the response over one cycle, which corresponds to summing the energies of the real and imaginary parts and using a factor 1/4 rather than 1/2.

feplot

`feplot` allows the visualization of these energies using a color coding. You should compute energies once, then select how it is displayed. Energy computation clearly require material and element properties to be defined in `InitModel`.

The earlier high level commands `fecom ColorDataK` or `ColorDataM` don't store the result and thus tend to lead to the need to recompute energies multiple times. The preferred strategy is illustrated below.

```
demosdt('LoadGartFe'); % load model,def
cf=feplot(model,def);cf.sel='elname quad4';fecom ch7
% Compute energy and store in Stack
Ek=fe_stress('ener -MatDes 1 -curve',model,def)
cf.Stack{'info','Ek'}=Ek;
% Color is energy density by element
feplot('ColorDataElt -dens -ColorBarTitle "Ener Dens"',Ek);
% Color by group of elements
cf.sel={'elname quad4', ... % Just the plates
'ColorDataElt -ColorBarTitle "ener" -bygroup -edgealpha .1', ...
Ek}; % Data with no need to recompute
```

Accepted `ColorDataElt` options are

- `-dens` divides by element volume. Note that this can be problematic for mixed element types (in the example above, the volume of `celas` springs is defined as its length, which is inappropriate here).
- `-frac` divides the result by the total energy (equal to the square of the modal frequency for normal modes).
- `-byGroup` sums energies within the same element group. Similarly `-byProId` and `-byMatId` group by property identifier. When results are grouped, the `fecom('InfoMass')` command gives a summary of results.

The color animation mode is set to `ScaleColorOne`.

Stress

`out=fe_stres('stress CritFcn Rest',MODEL,DEF,EltSel)` returns the stresses evaluated at elements of `Model` selected by `EltSel`.

The `CritFcn` part of the command string is used to select a criterion. Currently supported criteria are

- `sI, sII,` principal stresses from max to min. `sI` is the default.
- `sIII`
- `mises` Returns the von Mises stress (note that the plane strain case is not currently handled consistently).
- `-comp i` Returns the stress components of index `i`. This component index is giving in the engineering rather than tensor notation (before applying the `TensorTopology` transformation).

The `Rest` part of the command string is used to select a restitution method. Currently supported restitutions are

- `AtNode` average stress at each node (default). Note this is not currently weighted by element volume and thus quite approximate. Result is a structure with fields `.DOF` and `.data`.
- `AtCenter` stress at center or mean stress at element stress restitution points. Result is a structure with fields `.EltId` and `.data`.
- `AtInteg` stress at integration points (`*b` family of elements).
- `Gstate` returns a case with `Case.GroupInfo{jGroup,5}` containing the group `gstate`. This will be typically used to initialize stress states for non-linear computations. For multiple deformations, `gstate` the first `nElt` columns correspond to the first deformation.

The `fecom ColorDataStress` directly calls `fe_stress` and displays the result. For example, run the basic element test `q4p testsurstress`, then display various stresses using

```
q4p('testsurstress')
fecom('ColorDataStress atcenter')
fecom('ColorDataStress mises')
fecom('ColorDataStress sII atcenter')
```

To obtain strain computations, use the strain material as shown below.

```
[model,def]=hexa8('testload stress');
model.pl=m_elastic('dbval 100 strain','dbval 112 strain');
model.il=p_solid('dbval 111 d3 -3');
data=fe_stress('stress atcenter',model,def)
```

CritFcn

For stress processing, one must often distinguish the raw stress components associated with the element formulation and the desired output. `CritFcn` are callback functions that take a local variable `r1` of dimensions (stress components \times nodes \times deformations) and to replace this variable with the desired stress quantity(ies). For example

```
function out=first_comp(r1)
    out=squeeze(r1(1,:,:,:));
```

would be a function taking the first component of a computed stress. `sdtweb fe_stress('Principal')` provides stress evaluations classical for mechanics.

Redefining the `CritFcn` callback is in particular used in the `StressCut` functionality, see section 4.4.

See also `fe_mk`, `feplot`, `fecom`

fe_time,of_time

Purpose Computation of time and non linear responses.

Syntax

```
def=fe_time(model)
def=fe_time(TimeOpt,model)
[def,model,opt]=fe_time(TimeOpt,model)
model=fe_time('TimeOpt...',model)
TimeOpt=fe_time('TimeOpt...')
```

Description `fe_time` groups static non-linear and transient solvers to compute the response of a FE model given initial conditions, boundary conditions, load case (section 4.2.3) and time parameters. Note that you may access to the `fe_time` commands graphically with the simulate tab of the feplot GUI. See tutorial (section 4.5) on how to compute a response.

Solvers and options

Three types of time integration algorithm are possible: the Newmark schemes, the Theta-method, and the time Discontinuous Galerkin method. Implicit and explicit methods are implemented for the Newmark scheme, depending on the Newmark coefficients β and γ , and non linear problems are supported.

The parameters of a simulation are stored in a time option data structure `TimeOpt` given as input argument or in a `model.Stack` entry `info,TimeOpt`. Initial conditions are stored as a `curve,q0` entry.

The solvers selected by the string `TimeOpt.Method` are

- `newmark`, linear Newmark
- `NLNewmark`, non linear Newmark (with Newton iterations)
- `staticNewton`, static Newton
- `Theta`, Theta-Method (linear)
- `Euler` method for first order time integration.
- `dg`, Discontinuous Galerkin

Here is a simple example to illustrate the common use of this function.


```

model=fe_time('demo bar'); % build the model

% set the time options in model.Stack
model=fe_time('TimeOpt Newmark .25 .5 0 1e-4 100',model);

def=fe_time(model); % compute the response

```

`fe_time` can also be called with `TimeOpt` as the first argument. This is often more convenient when the user modifies options fields by hand

```
def=fe_time(TimeOpt,model);
```

The `TimeOpt` data structure has fields

<code>Method</code>	selection of the solver
<code>Opt</code>	if any, for example for Newmark [beta gamma t0 deltaT Nstep]
<code>OutInd</code>	DOF output indices (see 2D example). This selection is based on the state DOFs which can be found using <code>fe_case(model,'GetDof')</code> .
<code>MaxIter</code>	maximum number of iterations.
<code>nf</code>	optional value of the first residual norm.
<code>IterInit,IterEnd</code>	see the non linear solvers.
<code>Jacobian</code>	string to be evaluated to generate a factored jacobian matrix in matrix or <code>ofact</code> object <code>ki</code> . The default jacobian matrix is <code>'ki=ofact(model.K{3}+2/dt*model.K{2}+4/(dt*dt)*model.K{1});'</code> for the dynamic case and <code>'ki=ofact(model.K{3});'</code> for the static case.
<code>JacobianUpdate</code>	controls the update of Jacobian in Newton and quasi-Newton loops. Use 1 for updates and 0 for a fixed Jacobian (default).
<code>Residual</code>	Callback evaluated for residual. The default residual is method dependent.
<code>InitAcceleration</code>	optional field to be evaluated to initialize the acceleration field.
<code>OutputFcn</code>	string to be evaluated for post-processing or time vector containing the output time steps
<code>FinalCleanupFcn</code>	string to be evaluated for final post-processing of the output time steps

<code>c_u, c_v, c_a</code>	optional observation matrices for displacement, velocity and acceleration outputs.
<code>lab_u, lab_v, lab_a</code>	optional cell array containing labels describing each output (lines of observation matrices)
<code>NeedUVA</code>	<code>[NeedU NeedV NeedA]</code> , if <code>NeedU</code> is equal to 1, output displacement, etc.
<code>OutputInit</code>	optional string to be evaluated to initialize the output (before the time loop)
<code>SaveTimes</code>	optional time vector, saves time steps on disk
<code>TimeVector</code>	optional value of computed time steps, if exists <code>TimeVector</code> is used instead of time parameters
<code>RelTol</code>	threshold for convergence tests. The default is the OpenFEM preference <code>getpref('OpenFEM','THRESHOLD',1e-6);</code>

Input and output options

This section details the applicable input and the output options.

Initial conditions may be provided in a `model.Stack` entry of type `info` named `q0` or in an input argument `q0`. `q0` is a data structure containing `def` and `DOF` fields as in a FEM result data structure (section 4.5). If any, the second column gives the initial velocity. If `q0` is empty, zero initial conditions are taken. In this example, a first simulation is used to determine the initial conditions of the final simulation.

```

model=fe_time('demo bar');
TimeOpt=fe_time('TimeOpt Newmark .25 .5 0 1e-4 100');
TimeOpt.NeedUVA=[1 1 0];
% first computation to determine initital conditions
def=fe_time(TimeOpt,model);

% no input force
model=fe_case(model,'remove','Point load 1');

% Setting initial conditions
q0=struct('def',[def.def(:,end) def.v(:,end)],'DOF',def.DOF);
model=stack_set(model,'curve','q0',q0);

def=fe_time(TimeOpt,model);

```

An alternative call is possible using input arguments

```

def=fe_time(TimeOpt,model,Case,q0)

```

In this case, it is the input argument `q0` which is used instead of an eventual stack entry.

You may define the time dependence of a load using curves as illustrated in section 7.9.

You may specify the time steps by giving the `'TimeVector'`

```
TimeOpt=struct('Method','Newmark','Opt',[.25 .5 ],...
               'TimeVector',linspace(0,100e-4,101));
```

This is useful if you want to use non constant time steps. There is no current implementation for self adaptive time steps.

To illustrate the output options, we use the example of a 2D propagation. Note that this example also features a time dependent `DofLoad` excitation (see `fe_case`) defined by a curve, (see `fe_curve`), here named `Point load 1`.

```
model=fe_time('demo 2d');
TimeOpt=fe_time('TimeOpt Newmark .25 .5 0 1e-4 50');
```

You may specify specific output by selecting DOF indices as below

```
i1=fe_case(model,'GetDof'); i2=feutil('findnode y==0',model)+.02;
TimeOpt.OutInd=fe_c(i1,i2,'ind');
model=stack_set(model,'info','TimeOpt',TimeOpt);
def=fe_time(model); % Don't animate this (only bottom line)
```

You may select specific output time step using `TimeOpt.OutputFcn` as a vector

```
TimeOpt.OutputFcn=[11e-4 12e-4];
TimeOpt=feutil('rmfield',TimeOpt,'OutInd');
model=stack_set(model,'info','TimeOpt',TimeOpt);
def=fe_time(model); % only two time steps saved
```

or as a string to evaluate. The output is the `out` local variable in the `fe_time` function and the current step is `j1+1`. In this example the default output function (for `TimeOpt.NeedUVA=[1 1 1]`) is used but specified for illustration

```
TimeOpt.OutputFcn=['out.def(:,j1+1)=u;' ...
                  'out.v(:,j1+1)=v;out.a(:,j1+1)=a;'];
model=stack_set(model,'info','TimeOpt',TimeOpt);
def=fe_time(model); % full deformation saved
```

This example illustrates how to display the result (see `feplot`) and make a movie

```

cf=feplot(model,def);
fecom('ColorDataEvalA');
fecom(cf,'SetProp sel(1).fsProp','FaceAlpha',1,'EdgeAlpha',0.1);
cf.ua.clim=[0 2e-6];fecom(';view2;AnimTime;ch20;scd1e-2;');
st=fullfile(getpref('SDT','tempdir'),'test.avi');
fecom(['animavi ' st]);fprintf('\nGenerated movie %s\n',st);

```

Note that you must choose the `Anim:Time` option in the `feplot` GUI.

You may want to select outputs using observations matrix

```

model=fe_time('demo bar'); Case=fe_case('gett',model);
i1=feutil('findnode x>30',model);
TimeOpt=fe_time('TimeOpt Newmark .25 .5 0 1e-4 100');
TimeOpt.c_u=fe_c(Case.DOF,i1+.01);           % observation matrix
TimeOpt.lab_u=fe_c(Case.DOF,i1+.01,'dofs'); % labels

```

```
def=fe_time(TimeOpt,model);
```

If you want to specialize the output time and function you can specify the `SaveTimes` as a time vector indicating at which time the `SaveFcn` string will be evaluated. A typical `TimeOpt` would contain

```

TimeOpt.SaveTimes=[0:Ts:TotalTime];
TimeOpt.SaveFcn='My_function(''Output'',u,v,a,opt,out,j1,t)';

```

Cleanup

The field `FinalCleanupFcn` of the `TimeOpt` can be used to specify what is done just after the time integration.

```
fe_simul provides a generic clean up function which can be called using
opt.FinalCleanupFcn='fe_simul(''fe_timeCleanup'',model)';
```

Some command options can be used.

- `-cf i` stores the result of time integration in the stack of `iiplot` or `feplot` figure number `i`. `i=-1` can be specified to use current `iiplot` figure and `i=-2` for current `feplot` figure. Displacements are stored in `curve,disp` entry of the stack. Velocities and accelerations (if any) are respectively stored in the `curve,vel` and `curve,acc` stack entries. If command option `-reset` is present, existent stack entries (`disp`, `vel`, `acc`, etc. ...) are lost whereas if not stack entries name are incremented (`disp(1)`, `disp(2)`, etc. ...).

- Command option `'-ExitFcn"AnotherCleanupFcn"'` can be used to call an other clean up function just after `fe_simul('fe_timeCleanup')` is performed.
- Command option `fullDOF` performs a restitution of the output on the unconstrained DOF of the model used by `fe_time`. this option is exclusive to `restitFeplot`
- Command option `-rethrow` allows outputting the cross reference output data from `iiplot` or `feplot` if the option `-cf-1` or `-cf-2` is used.
- Command option `-restitFeplot` adds a `.TR` field to the output to allow deformation on the fly restitution in `feplot`, this option is exclusive to `fullDOF`.

newmark

For the Newmark scheme, `TimeOpt` has the form

```
TimeOpt=struct('Method','Newmark','Opt',Opt)
```

where `TimeOpt.Opt` is defined by

```
[beta gamma t0 deltaT Nstep]
```

`beta` and `gamma` are the standard Newmark parameters [36] ([0 0.5] for explicit and default at [.25 .5] for implicit), `t0` the initial time, `deltaT` the fixed time step, `Nstep` the number of steps.

The default residual is `r = (ft(j1,:)*fc'-v'*c-u'*k)'`; (notice the sign change when compared to `NLNewmark`).

This is a simple 1D example plotting the propagation of the velocity field using a Newmark implicit algorithm. Rayleigh damping is declared using the `info,Rayleigh` case entry.

```
model=fe_time('demo bar');
data=struct('DOF',2.01,'def',1e6,...
           'curve',fe_curve('test ricker 10e-4 1'));
model = fe_case(model,'DOFLoad','Point load 1',data);
TimeOpt=struct('Method','Newmark','Opt',[.25 .5 3e-4 1e-4 100],...
              'NeedUVA',[1 1 0]);
def=fe_time(TimeOpt,model);

% plotting velocity (propagation of the signal)
```

```
def_v=def;def_v.def=def_v.v; def_v.DOF=def.DOF+.01;
feplot(model,def_v);
if sp_util('issdt'); fecom(';view2;animtime;ch30;scd3');
else; fecom(';view2;scaledf3'); end
```

dg

The time discontinuous Galerkin is a very accurate time solver [49] [50] but it is much more time consuming than the Newmark schemes. No damping and no non linearities are supported for Discontinuous Galerkin method.

The options are [unused unused t0 deltaT Nstep Nf], `deltaT` is the fixed time step, `Nstep` the number of steps and `Nf` the optional number of time step of the input force.

This is the same 1D example but using the Discontinuous Galerkin method:

```
model=fe_time('demo bar');
TimeOpt=fe_time('TimeOpt DG Inf Inf 0. 1e-4 100');
TimeOpt.NeedUVA=[1 1 0];
def=fe_time(TimeOpt,model);

def_v=def;def_v.def=def_v.v; def_v.DOF=def.DOF+.01;
feplot(model,def_v);
if sp_util('issdt'); fecom(';view2;animtime;ch30;scd3'); ...
else; fecom(';view2;scaledf3'); end
```

NLNewmark

For the non linear Newmark scheme, `TimeOpt` has the same form as for the linear scheme (method `Newmark`). Additional fields can be specified in the `TimeOpt` data structure

Jacobian	string to be evaluated to generate a factored jacobian matrix in matrix or <code>ofact</code> object <code>ki</code> . The default jacobian matrix is <code>'ki=ofact(model.K{3}+2/dt*model.K{2}'+4/(dt*dt)*model.K{1});'</code>
Residual	Defines the residual used for the Newton iterations of each type step. It is typically a call to an external function. The default residual is <code>'r = model.K{1}*a+model.K{2}*v+model.K{3}*u-fc;'</code> where <code>fc</code> is the current external load, obtained using <code>(ft(j1,:)*fc)'</code> at each time step.
IterInit	evaluated when entering the correction iterations. This can be used to initialize tolerances, change mode in a co-simulation scheme, etc.
IterEnd	evaluated when exiting the correction iterations. This can be used to save specific data, ...
IterFcn	Correction iteration algorithm function, available are <code>iterNewton</code> (default when ommitted) or <code>iterNewton.Sec</code> . Details of the implementation are given in the <code>staticNewton</code> below.
MaxIterSec	for <code>iterNewton.Sec</code> applications (see <code>staticNewton</code>).
ResSec	for <code>iterNewton.Sec</code> applications (see <code>staticNewton</code>).

`staticNewton`

For non linear static problems, the Newton solver `iterNewton` is used. `TimeOpt` has a similar form as with the `NLNewmark` method but no parameter `Opt` is used.

An increment control algorithm `iterNewton.Sec` can be used when convergence is difficult or slow (as it happens for systems showing high stiffness variations). The Newton increment Δq is then the first step of a line search algorithm to optimize the corective displacement increment $\rho\Delta q, \rho \in \mathbf{R}$ in the iteration. This optimum is found using the secant iteration method. Only a few optimization iterations are needed since this does not control the mechanical equilibrium but only the relevance of the Newton increment. Each secant iteration requires two residual computations, which can be costly, but more efficient when a large number of standard iterations (matrix inversion) is required to obtain convergence.

Fields can be specified in the `TimeOpt` data structure

Jacobian	string to be evaluated to generate a factored jacobian matrix in matrix or ofact object ki . The default jacobian matrix is <code>'ki=ofact(model.K{3});'</code>
Residual	Defines the residual used for the Newton iterations of each type step. It is typically a call to an external function. The default residual is <code>'r = model.K{3}*u-fc;'</code>
IterInit	evaluated when entering the Newton solver. This can be used to initialize tolerances, change mode in a co-simulation scheme, etc.
IterEnd	evaluated when exiting the Newton solver. This can be used to save specific data, ...
IterFcn	Correction iteration function. It is a string naming an fe_time iteration subfunction available, or an external function. When performing the time simulation initialization, the string will be replaced by the function handle (e.g. <code>@iterNewton</code>). The available iteration algorithms in fe_time are either iterNewton , which is defaulted when the IterFcn field is omitted and performs basic Newton iterations, either iterNewton_Sec which is the Newton increment control algorithm.
MaxIterSec	Maximum secant iterations for the iterNewton_Sec iteration algorithm. The default is 3 when omitted.
ResSec	Residual evaluation for the secant iterations of the iterNewton_Sec iteration algorithm. When omitted, fe_time tries to interpret the Residual field. The function must fill in the secant residual evaluation r1 which two columns will contain the residual for solution rho(1)*dq and rho(2)*dq . The default ResSec field will be then <code>'r1(:,1) = model.K{3}*(u-rho(1)*dq)-fc; r1(:,2) = model.K{3}*(u-rho(2)*dq)-fc;'</code>

Below is a trivial demonstration of formats and call for non-linear bar statically loaded in 5 steps.

```
mdl=femesh('testbar1');

mdl=fe_case(mdl,'DofLoad','right',struct('DOF',2.01,'def',1));
mdl=fe_case(mdl,'FixDof','left',1);
mdl=fe_case(mdl,'setcurve','right', ...
    fe_curve('testramp 5 1')); % 5 steps gradual load
```



```

mdl=stack_set(mdl,'info','TimeOpt', ...
    struct('Opt',[],'Method','staticnewton',...
        'Jacobian','ki=basic_jacobian(model,ki,0.,0.,opt.Opt);',...
        'Residual','r = model.K{3}*u-fc-model.K{3}*u.^1.5;'));

def=fe_time(mdl);feplot(mdl,def);fecom('showdefarrow');

```

numerical damping for Newmark, HHT-alpha schemes

You may want to use numerical damping in a time integration scheme, the first possibility is to tune the Newmark parameters using a coefficient α such that $\beta = \frac{(1+\alpha)^2}{4}$ and $\gamma = \frac{1}{2} + \alpha$. This is known to implement too much damping at low frequencies and is very depending on the time step [36].

A better way to implement numerical damping is to use the HHT- α method which applies the Newmark time integration scheme to a modified residual balancing the forces with the previous time step.

For the HHT- α scheme, `TimeOpt` has the form

```

TimeOpt=struct('Method','nlnewmark','Opt',Opt,...
    'HHTalpha',alpha)

```

where `TimeOpt.Opt` is defined by

```
[beta gamma t0 deltaT Nstep]
```

`beta` and `gamma` are the standard Newmark parameters [36] with numerical damping, `t0` the initial time, `deltaT` the fixed time step, `Nstep` the number of steps.

The automatic `TimeOpt` generation call takes the form `[alpha unused t0 deltaT Nstep]` and will compute the corresponding β , γ parameters.

This is a simple 1D example plotting the propagation of the velocity field using the HHT- α implicit algorithm:

```

model=fe_time('demo bar');
TimeOpt=fe_time('TimeOpt hht .05 Inf 3e-4 1e-4 100');
TimeOpt.NeedUVA=[1 1 0];
def=fe_time(TimeOpt,model);

```

The call

```
TimeOpt=fe_time('TimeOpt hht .05 Inf 3e-4 1e-4 100');
```

is strictly equivalent to

```

TimeOpt=struct('Method','nlnewmark',...
    'Opt',[.275625 .55 3e-4 1e-4 100],...
    'HHTalpha',.05);

```

Theta

The θ -method is a velocity based solver, whose formulation is given for example in [51, 52]. It considers the acceleration as a distribution, thus relaxing discontinuity problems in non-smooth dynamics. Only a linear implementation is provided in `fe_time`. The user is nevertheless free to implement a non-linear iteration, through his own `IterFcn`.

This method takes only one integration parameter for its scheme, θ set by default at 0.5. Any values between 0.5 and 1 can be used, but numerical damping occurs for $\theta > 0.5$.

The `TimeOpt.Opt` takes the form `[theta unused t0 deltaT Nstep]`.

This is a simple 1D example plotting the propagation of the velocity field using the θ -Method:

```
model=fe_time('demo bar');
TimeOpt=fe_time('TimeOpt theta .5 0 3e-4 100');
def=fe_time(TimeOpt,model);
```

Euler

This method can be used to integrate first order problem of the form $M\dot{q} + Kq = F$. One can use it to solve transient heat diffusion equation (see `p_heat`).

Integration scheme is of the form $q_{n+1} = q_n + (1 - \theta)h\dot{q}_n + \theta h\dot{q}_{n+1}$. θ can be define in `opt.Opt(1)`. Explicit Euler ($\theta = 0$) is not implemented at this time. Best accuracy is obtained with $\theta = \frac{1}{2}$ (Crank-Nicolson).

of_time

The `of_time` function is a low level function dealing with CPU and/or memory consuming steps of a time integration.

The **case sensitive** commands are

<code>lininterp</code>	linear interpolation.
<code>storelaststep</code>	pre-allocated saving of a time step.
<code>interp</code>	Time scheme interpolations (low level call).

The `lininterp` command which syntax is

```
out = of_time ('lininterp',table,val,last) ,
```

computes `val` containing the interpolated values given an input `table` which first column contains the abscissa and the following the values of each function. Due to performance requirements, the abscissa must be in ascending order. The variable `last` contains `[i1 xi si]`, the starting index (beginning at 0), the first abscisse and coordinate. The following example shows the example of 2 curves to interpolate:

```
out=of_time('lininterp',[0 0 1;1 1 2;2 2 4],linspace(0,2,10)',[0 0 0])
```

Warning : this command modifies the variable `last` within a given function this may modify other identical constants in the same m-file. To avoid any problems, this variable should be generated using `zeros` (the Matlab function) to assure its memory allocation independence.

The `storelaststep` command makes a deep copy of the displacement, velocity and acceleration fields (stored in each column of the variable `uva.uva` in the preallocated variables `u`, `v` and `a` following the syntax:

```
of_time('storelaststep',uva,u,v,a);
```

The `interp` command is used by `fe_time` when the user gives a `TimeVector` in the command. For time `t1` and the `uva` matrix containing in each column, displacement, velocity and acceleration at the preceding time step `t0`, the interpolation strategy depends on the arguments given to `of_time`.

Warning : this command modifies `out.def` at very low level, `out.def` thus cannot be initialized by simple numerical values, but by a non trivial command (use `zeros(1)` instead of `0` for example) to ensure the unicity of this data in memory.

For a `Newmark` or `HHT-alpha` scheme, the low level call command is

```
of_time ('interp', out, beta,gamma,uva,a, t0,t1,model.FNL);
```

where `beta` and `gamma` are the coefficients of the Newmark scheme, first two values of `opt.Opt`.

Thus the displacement (u_1) and velocity (v_1) at time `t1` will be computed from the displacement (u_0), velocity (v_0), acceleration (a_0) stored in `uva`, the new acceleration `a` (a_1), and the time step ($h = t1 - t0$) as

$$\begin{cases} v_1 = v_0 + h(1 - \gamma)a_0 + h\gamma a_1 \\ u_1 = u_0 + hv_0 + h^2(\frac{1}{2} - \beta)a_0 + h^2\beta a_1 \end{cases} \quad (9.4)$$

NL force (`model.FNL`) is linearly interpolated.

For the `Theta-Method` scheme, the low level command is

```
of_time ('interp', out, opt.Opt(1), [],uva,v, t0,t1,model.FNL);
```

Thus the displacement (u_1) at time `t1` will be computed from the displacement (u_0), velocity (v_0), stored in `uva`, the new velocity `v` (v_1), and the time step ($h = t1 - t0$) as

$$u_1 = u_0 + h(1 - \theta)v_0 + h\theta v_1 \quad (9.5)$$

For the `staticnewton` method, it is possible to use the same storage strategy (since it is optimized for performance), using

```
of_time ('interp', out, [],[], [],u, t0,t1,model.FNL);
```

In this case no interpolation is performed.

Please note that this low-level call uses the internal variables of `fe_time` at the state where it is evaluated. It is then useful to know that inside `fe_time`:

- at time `tc=t(j1+1)` using time step `dt`, values are `t0=tc-dt` and `t1=tc`.
- `uva` is generally stored in `Case.uva`.
- the current acceleration, velocity or displacement values when interpolation is performed are always `a`, `v`, and `u`.
- The `out` data structure must be preallocated and is modified by low level C calls. Expected fields are

<code>def</code>	must be preallocated with size <code>length(OutInd) x length(data)</code>
<code>data</code>	column vector of output times
<code>OutInd</code>	<code>int32</code> vector of output indices, must be given
<code>cur</code>	<code>[Step dt]</code> , must be given
<code>FNL</code>	possibly preallocated data structure to store non-linear loads. <code>FNL.def</code> must be <code>length(model.FNL)</code> by <code>size(out.data,1)</code> (or possibly <code>size(out.FNL.data,1)</code> , in this case fieldnames must be <code>def,DOF,data,cur</code>)

- non linear loads in `model.FNL` are never interpolated.

See also [fe_mk](#), [fe_load](#), [fe_case](#)

idcom

Purpose UI command functions for standard operations in identification.

Syntax `idcom('CommandString');`

Description `idcom` provides a simple access to standard operations in identification. The way they should be sequenced is detailed in section 2.3 which also illustrates the use of the associated GUI.

`idcom` is always associated with an `iiplot` figure. Information on how to modify standard plots is given under `iicom`. The datasets used by `idcom` are described in section 2.3. Methods to access the data from the command line are described in section 2.1.2. Identification options stored in the figure are detailed under the `idopt` function.

`idcom(ci)` turns the environment on, `idcom(ci,'Off')` removes options but not datasets.

The information given below details each command (see the `commode` help for hints on how to build commands and understand the variants discussed in this help). Without arguments `idcom` opens or refreshes the current `idcom` figure.

Commands

`e [,i w]`

Single pole narrow-band model identification. `e` calls `ii_poest` to determine a single pole narrow band identification for the data set `ci.Stack{'test'}`.

A bandwidth of two percent of w is used by default (when i is not given). For $i < 1$, the i specifies the half bandwidth as a fraction of the central frequency w . For i an integer greater than 5, the bandwidth is specified as a number of retained frequency points.

The selected frequency band is centered around the frequency w . If w is not given, `ii_poest` will wait for you to pick the frequency with your mouse.

If the local fit does not seem very good, you should try different bandwidths (values of i).

The results are stored in `ci.Stack{'IdAlt'}` with a pole `.po` and residue `.res` field. FRFs are resynthesized into `ci.Stack{'IdFrfr'}` (which is overlaid to `ci.Stack{'Test'}`)

in `iipplot`). If, based on the plot(s), the estimate seems good it should be added to the current pole set `ci.Stack{'IdMain'}` using `ea`.

`ea`

Add alternate poles to the main set. If appropriate modes are present in `ci.Stack{'IdAlt'}` (after using the `e` or `f` commands for example), they should be added to the main pole set `ci.Stack{'IdMain'}` using the `ea` command. These poles can then be used to identify a multiple pole broadband model with `idcom est` and `idcom eup` commands.

If all poles in `ci.Stack{'IdAlt'}` are already in `ci.Stack{'IdMain'}`, the two are only combined when using the `eaf` command (this special format is used to prevent accidental duplication of the nodes).

`er [num i, f w]`

Remove poles from `ci.Stack{'IdMain'}`. The poles to be removed can be indicated by number using `'er num i'` or by frequency using `'er f w'` (the pole with imaginary part closest to `w` is removed). The removed pole is placed in `ci.Stack{'IdAlt'}` so that an `ea` command will undo the removal.

`est[,local,localpole]`

Broadband multiple pole identification without pole update. `est` uses `id_rc` to identify a model based on the complete frequency range. This estimate uses the current pole set `ci.Stack{'IdMain'}` but does not update it. The results are a residue matrix `ci.Stack{'IdMain'}.res`, and corresponding FRF `ci.Stack{'IdFrf'}` (which is overlaid to `ci.Stack{'Test'}` in `iipplot`). In most cases the estimate can be improved by optimizing the poles using the `eup` or `eopt` commands.

`estLocal` only estimates residues of poles in the range selected by `ci.IDopt`. You perform a series of local estimates over selected bands by providing these bands or using narrow band around each pole with `estLocalPole`.

`gartid`

```
idcom('w0');idcom est
def_global=ci.Stack{'IdMain'}; % broadband estimate

idcom('estlocal',{[6 7],[15 17],[31 38],[48 65]});
def_local=ci.Stack{'IdMain'}; % estimate by multiple local bands
```

```
eup dstep fstep [local, num i , iter j ]
```

Update of poles. `eup` uses `id_rc` to update the poles of a multiple pole model based data within `ci.IDopt.SelectedRange`. This update is done through a non-linear optimization of the pole locations detailed in section 2.3.3. The results are updated modes `ci.Stack{'IdMain'}` (the initial ones are stored in `ci.Stack{'IdAlt'}`), and corresponding FRF `ci.Stack{'IdFrf'}` (which is overlaid in `iiplot`).

In most cases, `eup` provides significant improvements over the initial pole estimates provided by the `e` command. In fact the only cases where you should not use `eup` is when you have a clearly incomplete set of poles or have reasons to suspect that the model form used by `id_rc` will not provide an accurate broadband model of your response.

Default values for damping and frequency steps are `0.05` and `0.002`. You may specify other values. For example the command `'eup 0.05 0.0'` will only update damping values.

It is often faster to start by optimizing over small frequency bands while keeping all the poles. Since some poles are not within the selected frequency range they should not be optimized. The option `local` placed after values of `dstep` and `fstep` (if any) leads to an update of poles whose imaginary part are within the retained frequency band.

When using local update, you may get warning messages about conditioning. These just tell you that residues of modes outside the band are poorly estimated, so that the message can be ignored. While algorithms that by-pass the numerical conditioning warning exist, they are slower and don't change results so that the warning was left.

In some cases you may want to update specific poles. The option `num i` where `i` gives the indices in `IdMain` of the poles you want to update. For example `'eup 0.0 0.02 num 12'` will update the frequency of pole `12` with a step of 2%.

- The poles in `ci.Stack{'IdMain'}.po` are all the information needed to obtain the full model estimate. You should save this information in a text file (use `idcom('TableIdMain')` to generate a clean output) to be able to restart/refine your identification.
- You can get a feel for the need to further update your poles by showing the error and quality plots (see `iiplot` and section 2.3.2).

`eopt [local, num i]`

Update of poles. `eopt` is similar to `eup` but uses `id_rcopt` to optimize poles. `eopt` is often more efficient when updating one or two poles (in particular with the `eopt local` command after selecting a narrow frequency band). `eopt` is guaranteed to improve the quadratic cost (3.3) so that using it rarely hurts.

`find`

Find a pole. This command detects minima of the MMIF that are away from poles of the current model `ci.Stack{'IdMain'}.po` and calls `ii_poest` to obtain a narrow band single pole estimate in the surrounding area. This command can be used as an alternative to indicating pole frequencies with the mouse (`e` command). More complex automated model initialization will be introduced in the future.

`f i`

Graphical input of frequencies. `f i` prompts the user for mouse input of `i` frequencies (the abscissa associated with each click is taken to be a frequency). The result is stored in the pole matrix `ci.Stack{'IdAlt'}.po` assuming that the indicated frequencies correspond to poles with 1% damping. This command can be used to create initial pole estimates but the command `e` should be used in general.

`dspl nm`

Direct system parameter identification. `dspl` uses `id_dspl` to create a `nm` pole state space model of `Test`. `nm` must be less than the number of sensors. The results are transformed to the residue form which gives poles and residues in `IdMain`, and corresponding FRF `IdFrf` (which is overlaid to `Test` in `iiplot`).

`mass i`

*Computes the generalized mass at address *i*.* If the identified model contains complex residues (`ci.IDopt.Fit='Pos'` or `'Complex'`), `res2nor` is used to find a real residue approximation. For real residues, the mass normalization of the mode is given by the fact that for collocated residues reciprocity implies

$$c_{Col}\phi_j = \phi_j^T b_{Col} = \sqrt{R_{jCol}} = (m_{jCol})^{-1/2}$$

The mass at a given sensor `i` is then related to the modal output `$c_i\phi_j$` of the mass normalized mode by `$m_{ij} = (c_i\phi_j)^{-2}$` . This command can only be used when col-

located transfer functions are specified and the system is assumed to be reciprocal (see `idopt`).

`poly nn nd`

Orthogonal polynomial identification. `poly` uses `id_poly` to create a polynomial model of `Test` with numerators of degree `nn` and denominators of degree `nd`. The corresponding FRFs are stored in `IdFrf` (which is overlaid to `Test` in `iipplot`).

[Table,TeX] `IIpo`

Formatted printout of pole variables `IIpo` or `IIpo1`. With the `Tex` command the printout is suitable for inclusion in LATEX.

This command is also accessible from the `idcom` figure context menu.

See also `idcom`, `iicom`, `iipplot`, `id_rc`, section 2.3

idopt

Purpose handling of options used by the identification related routines.

Description `idopt` is the function handling identification options. Identification options associated with `idcom` figures are used when generating new identifications. They should be modified using the `ci.IDopt` pointer or the `IDopt` tab in the figure. In the text output below

```
>> ci=idcom; ci.IDopt
(ID options in figure(2)) =
  ResidualTerms : [ 0 | 1 (1) | 2 (s^-2) | {3 (1 s^-2)} | 10 (1 s)]
  DataType : [ {disp./force} | vel./force | acc./force ]
  AbscissaUnits : [ {Hz} | rd/s | s ]
  PoleUnits : [ {Hz} | rd/s ]
  SelectedRange : [ 1-3124 (4.0039-64.9998) ]
  FittingModel : [ Posit. cpx | {Complex modes} | Normal Modes]
  NSNA : [ 0 sensor(s) 0 actuator(s) ]
  Reciprocity : [ {Not used} | 1 FRF | MIMO ]
  Collocated : [ none declared ]
```

currently selected value are shown between braces `{ }` and alternatives are shown.

After performing an identification, the options used at the time are copied to the result. Thus the `ci.Stack{'IdMain'}.idopt` is a copy of the figure options when the identification was performed. Some manipulations possible with the `res2nor`, `res2ss`, `id_nor`, ... functions may require modifications of these options (which are different from the `idcom` figure options).

The `SDT handle` object used to store options is very permissive in the way to change values from the command line (for GUI operation use the `IDopt` tab). `ci.IDopt.OptName=OptValue` sets the option. `OptName` need only specify enough characters to allow a unique option match. Thus `ci.IDopt.res` and `ci.IDopt.ResidualTerms` are equivalent. Here are a few examples

```
demosdt('demoGartIdEst');ci=idcom;
ci.IDopt.Residual=0; % modify estimation default
ci.IDopt.Selected=[100 2000];
ci.IDopt.Po='Hz';
ci.IDopt % changed
ci.Stack{'IdMain'}.idopt % not changed until new identification
```

The following is a list of possible options with indications as to where they are stored. Thus `ci.IDopt.res=2` is simply a user friendly form for the old call `ci.IDopt(6)=2` which you can still use.

Res	Residual terms (stored in <code>ci.IDopt(1)</code>)
0	none
1	Static correction (high frequency mode correction)
2	Roll-off (s^{-2} , low frequency mode correction).
3	Static correction and roll-off (default)
10	1 and s, this correction is only supported by <code>id_rc</code> and should be used for identification in narrow bandwidth (see <code>ii_poest</code> for example)
-i	An alternate format uses negative numbers with decades indicating powers (starting at s^{-2}). Thus <code>Ass=-1101</code> means an asymptotic correction with terms in $s^{-2}, 1, s$
Data	type (stored in <code>ci.IDopt(2)</code>)
0	displacement/force (default)
1	velocity/force
2	acceleration/force
Abscissa	units for vector w can be Hz, rad/s or seconds
Pole	units can be Hz or rad/s
	units are actually stored in <code>ci.IDopt(3)</code> with units giving abscissa units (<code>01 w</code> in Hertz, <code>02 w</code> in rad/s, <code>03 w</code> time seconds) and tens pole units (<code>10 po</code> in Hertz, <code>20 po</code> in rad/s). Thus <code>ci.IDopt(3)=12</code> gives w in rad/sec and po in Hz.
Selected	frequency range indices of first and last frequencies to be used for identification or display (stored in <code>ci.IDopt(4:5)</code>)
Fitting	model (see res page 165, stored in <code>ci.IDopt(6)</code>)
0	positive-imaginary poles only, complex mode residue
1	complex mode residue, pairs of complex-conjugate poles (default)
2	normal mode residue
ns,na	number of sensors/actuators (outputs/inputs) stored in <code>ci.IDopt(7:8)</code>

idopt

- Recip** method selection for the treatment of reciprocity (stored in `ci.IDopt(12)`)
- 1** means that only **iC1** (`ci.IDopt(13)`) is declared as being collocated. `id_rm` assumes that only this transfer is reciprocal even if the system has more collocated FRFs
 - na** (number of actuators) is used to create fully reciprocal (and minimal of course) MIMO models using `id_rm`. **na** must match non-zero values declared in **iCi**.
 - nc** (with **nc** the number of collocated FRFs) is used to declare collocated FRFs while not enforcing reciprocity when using `id_rm`.
- iC1** indices of collocated transfer functions in the data matrix (see the `xf` ... format page 167)

To make a copy of the data, and no longer point to the figure, use `ci.IDopt.GetData`. `iop2 = idopt` returns a *SDT handle* to a set options that may differ from those of used by `idcom`.

See also `xfopt`, `idcom`, `iipplot`

id_dspi

Purpose Direct structural system parameter identification.

Syntax `[a,b,c,d] = id_dspi(y,u,w,idopt,np)`

Description The direct structural system parameter identification algorithm [53] considered here, uses the displacement frequency responses $y(s)$ at the different sensors corresponding to the frequency domain input forces $u(s)$ (both given in the `xf` format). For example in a SIMO system with a white noise input, the input is a column of ones `u=ones(size(w))` and the output is equal to the transfer functions `y=xf`. The results of this identification algorithm are given as a state-space model of the form

$$\begin{Bmatrix} \dot{p} \\ \ddot{p} \end{Bmatrix} = \begin{bmatrix} 0 & I \\ -K_T & -C_T \end{bmatrix} \begin{Bmatrix} p \\ \dot{p} \end{Bmatrix} + \begin{bmatrix} 0 \\ b_T \end{bmatrix} \{u\} \quad \text{and} \quad \{y\} = \begin{bmatrix} c_T & 0 \end{bmatrix} \begin{Bmatrix} p \\ \dot{p} \end{Bmatrix}$$

where the pseudo-stiffness K_T and damping C_T matrices are of dimensions `np` by `np` (number of normal modes). The algorithm, only works for cases where `np` is smaller than the number of sensors (`ci.IDopt.ns`).

```
ci=iicom('curveload sdt_id');
R1=ci.Stack{'Test'};
[a,b,c,d] = id_dspi(R1.xf,ones(size(R1.w)),R1.w,R1.idopt,4);
```

For SIMO tests, normal mode shapes can then be obtained using `[mode,freq] = eig(-a(np+[1:np],1:np))` where it must be noted that the modes **are not** mass normalized as assumed in the rest of the *Toolbox* and thus cannot be used directly for predictions (with `nor2xf` for example). Proper solutions to this and other difficulties linked to the use of this algorithm (which is provided here mostly for reference) are not addressed, as the main methodology of this *Toolbox* (`id_rc`, `id_rm`, and `id_nor`) was found to be more accurate.

For MIMO tests, `id_dspi` calls `id_rm` to build a MIMO model.

The identification is performed using data within `ci.IDopt.SelectedRange`. `y` is supposed to be a displacement. If `ci.IDopt.DataType` gives `y` as a velocity or acceleration, the response is integrated to displacement as a first step.

See also `idopt`, `id_rc`, `id_rm`, `psi2nor`, `res2nor`

id_nor

Purpose

Identification of normal mode model, with optimization of the complex mode output shape matrix.

```
NOR                = id_nor(ci.Stack{'IdMain'})
NOR                = id_nor( ... )
[om,ga,phib,cphi] = id_nor( ... )
[new_res,new_po]  = id_nor( ... )
[ ... ]           = id_nor(IdResult,ind,opt,res_now)
```

Description

`id_nor` is meant to provide an optimal transformation (see details in [12] or section 2.4.3) between the residue (result of `id_rc`) and non-proportionally damped normal mode forms

$$\{y(s)\} = \sum_{j=1}^{2N} \frac{\{c\psi_j\} \{\psi_j^T b\}}{s - \lambda_j} \{u\} \quad \text{and} \quad [Is^2 + \Gamma s + \Omega^2] \{p\} = [\phi^T b] \{u\}$$
$$\{y\} = [c\phi] \{p\}$$

The output arguments are either

- the standard normal mode model `freq,ga,phib,cphi` (see `nor`) when returning 4 outputs.
- the associated normal model data structure `NOR` when returning one output.
- or the residues of the associated model `new_res` and poles `po` (see `res` page 165) when returning 2 outputs. With this output format, the residual terms of the initial model are retained.

The algorithm combines `id_rm` (which extracts complex mode output shape matrices $c\psi$ from the residues `res` and scales them assuming the system reciprocal) and `psi2nor` (which provides an optimal second order approximation to the set of poles `po` and output shape matrices $c\psi$).

Since the results of `psi2nor` can quite sensitive to small errors in the scaling of the complex mode outputs $c\psi$, an optimization of all or part (using the optional argument `ind` to indicate the residues of which poles are to be updated) collocated residues can be performed. The relative norm between the identified residues `res` and those of the normal mode model is used as a criterion for this optimization.

Three optimization algorithms can be selected using `opt` (1: `id_min` of the *Structural Dynamics Toolbox*, 2: `fmins` of MATLAB, 3: `fminu` of the *Optimization Toolbox*).

You can also restart the optimization using the residues `old_res` while still comparing the result with the nominal `res` using the call

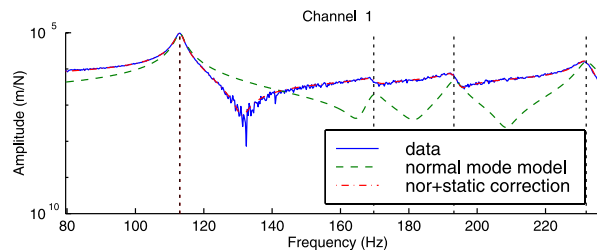
```
[new_res,po] = id_nor(res,po,idopt,ind,opt,old_res)
```

Notes

`id_nor` is only defined if `IDopt.Reciprocity` is 1 FRF or MIMO (12) and for cases with more sensors than modes (check `IDopt.NSNA`). `id_nor` may not work for identifications that are not accurate enough to allow a proper determination of normal mode properties.

In cases where `id_nor` is not applicable, normal mode residues can be identified directly using `id_rc` with `IDoptFit='Normal'` or an approximate transformation based on the assumption of proportional damping can be obtained with `res2nor`.

`id_nor` does not handle cases with more poles than sensors. In such cases `res2nor` can be used for simple approximations, or `id_nor` can be used for groups of modes that are close in frequency.



Residual terms can be essential in rebuilding FRFs (see figure above taken from `demo_id`) but are not included in the normal mode model (`freq`, `ga`, `phib`, `cphi`). To include these terms you can use either the residues `new_res` found by `id_nor`

```
xf = res2xf(new_res,po,w,idopt)
```

or combine calls to `nor2xf` and `res2xf`

```
xf = nor2xf(om,ga,phib,cphi,w) + ...
    res2xf(res,po,w,idopt,size(po,1)+1:size(res,1))
```

Example

```
ci=demosdt('demo_gartidest')
if ci.Stack{'Test'}.dof(4,2)~=1012.03;% Needed to have positive driving point I
    ci.Stack{'Test'}.xf=-ci.Stack{'Test'}.xf;
    ci.Stack{'Test'}.dof(:,2)=1012.03; idcom('est');
end
nor = id_nor(ci.Stack{'IdMain'});
ci.Stack{'IIxh'}=nor2xf(nor,ci.Stack{'Test'}.w,'hz struct acc');
iicom('iixhon')
```


See also [id_rc](#), [res2nor](#), [id_rm](#), [psi2nor](#), [demo_id](#)

id_poly

Purpose Parametric identification using **xf**-orthogonal polynomials.

Syntax

```
[num,den] = id_poly(xf,w,nn,nd)
[num,den] = id_poly(xf,w,nn,nd,idopt)
```

Description A fit of the provided frequency response function **xf** at the frequency points **w** is done using a rational fraction of the form $H(s) = num(s)/den(s)$ where **num** is a polynomial of order **nn** and **den** a polynomial of order **nd**. The numerically well conditioned algorithm proposed in Ref. [11] is used for this fit.

If more than one frequency response function is provided in **xf**, the numerator and denominator polynomials are stacked as rows of **num** and **den**. The frequency responses corresponding to the identified model can be easily evaluated using the command `qbode(num,den,w)`.

The identification is performed using data within `IDopt.SelectedRange`. The `idcom poly` command gives easy access to this function.

See also [id_rc](#), [invfreqs](#) of the *Signal Processing Toolbox*.

id_rc, id_rcopt

Purpose Broadband pole/residue model identification with the possibility to update an initial set of poles.

```
[res,po,xe]      = id_rc    (xf,po,w,idopt)
[res,new_po,xe] = id_rc    (xf,po,w,idopt,dst,fst)
[res,new_po,xe] = id_rcopt(xf,po,w,idopt,step,indpo)
```

Description This function is typically accessed using the `idcom` GUI figure as illustrated in section 2.3.

For a given set of poles `po` (see `ii_pof` for the format), `id_rc(xf,po,w,idopt)` identifies the residues of a broadband model, with poles `po`, that matches the FRFs `xf` at the frequency points `w`. (This is implemented as the `idcom est` command).

As detailed in section 2.3, the poles can (and should) be tuned [8] using either `id_rc` (ad-hoc dichotomy algorithm, accessible through the `idcom eup` command) or `id_rcopt` (gradient or conjugate gradient minimization, accessible through the `idcom eopt` command). `id_rc` performs the optimization when initial step sizes are given (see details below).

After the identification of a model in the residue form with `id_rc`, other model forms can be obtained using `id_rm` (minimal/reciprocal residue model), `res2ss` (state-space), `res2xf` (FRF) and `res2tf` (polynomial), `id_nor` (normal mode model).

The different input and output arguments of `id_rc` and `id_rcopt` are

`xf`

Measured data stored in the `xf` format where each row corresponds to a frequency point and each column to a channel (actuator/sensor pair).

Although it may work for other types of data, `id_rc` was developed to identify model properties based on *transfer functions from force actuators to displacement sensors*. `IDopt(2)` lets you specify that the data corresponds to velocity or acceleration (over force always). An integration (division by $s = j\omega$) is then performed to obtain displacement data and a derivation is performed to output estimated FRFs coherent with the input data (the residue model always corresponds to force to displacement transfer functions).

The phase of your data should loose 180° phase after an isolated lightly damped

but stable pole. If phase is gained after the pole, you probably have the complex conjugate of the expected data.

If the experimental set-up includes time-delays, these are not considered to be part of the mechanical system. They should be removed from the data set `xf` and added to the final model as sensor dynamics or actuator dynamics. You can also try to fit a model with a real poles for Pade approximations of the delays but the relation between residues and mechanical modes shapes will no longer be direct.

W

Measurement frequencies are stored as a column vector which indicates the frequencies of the different rows of `xf`. `IDopt(3)` is used to specify the frequency unit. By default it is set to `11` (FRF and pole frequencies in Hz) which differs from the *SDT* default of *rad/s* used in functions with no frequency unit option. It is assumed that frequencies are sorted (you can use the MATLAB function `sort` to order your frequencies).

po, new_po

Initial and updated pole sets. `id_rc` estimates residues based on a set of poles `po` which can be updated (leading to `new_po`). Different approaches can be used to find an initial pole set:

- create narrow-band single pole models (`ii_poest` available as the `idcom e` command).
- pick the pole frequencies on plots of the FRF or MMIF and use arbitrary but realistic values (e.g. 1%) for damping ratios (`ii_fin` available as the `idcom f` command).
- use pole sets generated by any other identification algorithm (`id_poly` and `id_dspi` for example).

Poles can be stored using different formats (see `ii_pof`) and can include both conjugate pairs of complex poles and real poles. (`id_rc` uses the frequency/damping ratio format).

The `id_rc` algorithms are meant for iterations between narrow-band estimates, used to find initial estimates of poles, and broadband model tuning using `id_rc` or `id_rcopt`. To save the poles to a text file, use `idcom Table`. If these are your best poles, `id_rc` will directly provide the optimal residue model. If you are still

iterating you may replace these poles by the updated ones or add a pole that you might have omitted initially.

IDopt

Identification options (see `idopt` for details). Options used by `id_rc` are `Residual`, `DataType`, `AbscissaUnits`, `PoleUnits`, `SelectedRange` and `FittingModel`.

The definition of channels in terms of actuator/sensor pairs is only considered by `id_rm` which should be used as a post-treatment of models identified with `id_rc`.

`dstep`, `fstep` (for `id_rc`)

Damping and frequency steps. To update pole locations, the user must specify initial step sizes on the frequency and damping ratio (as fractions of the initial values). `id_rc` then uses the gradient of the quadratic FRF cost to determine in which direction to step and divides the step size by two every time the sign changes. This approach allows the simultaneous update of all poles and has proved over the years to be extremely efficient.

For lightly damped structures, typical step values (used by the `idcom` command `eup`) are 10% on all damping ratios (`dstep = 0.1`) and 0.2% on all frequencies (`fstep = 0.002`). If you only want to update a few poles `fstep` and `dstep` can be given as vectors of length the number of poles in `po` and different step values for each pole.

`idcom('eup 0.05 0.002 local')` can be used to specify `dstep` and `fstep`. The optional `local` at the end of the command specifies that zero steps should be used for poles whose resonance is outside the selected frequency band.

`step`, `indpo` (for `id_rcopt`)

Methods and selected poles. `step` specifies the method used for step length, direction determination method, line search method, reference cost and pole variations. You should use the default values (empty `step` matrix). `indpo` gives the indices of poles to be updated (`po(indpo,:)` for poles in format 2 are the poles to be updated, by default all poles are updated).

The `idcom` `eopt` command can be used to access `id_rcopt`. `eoptlocal` calls `id_rcopt` with `indpo` set to only update poles whose resonance is within the selected frequency band.

res

Residues are stored in the `res` format (see section 5.6). If the options `IDopt` are properly specified this model corresponds to force to displacement transfer functions (even if the data is acceleration or velocity over force). Experts may want to mislead `id_rc` on the type of data used but this may limit the achievable accuracy.

xe

Estimated FRFs correspond to the identified model with appropriate derivation if data is acceleration or velocity over force.

See also

[idcom](#), [id_rm](#), [res2xf](#), [res2ss](#)

Tutorial section section 2.3

[gartid](#) and [demo_id](#) demonstrations

id_rm

Purpose

Create minimal models of MIMO systems and apply reciprocity constraints to obtain scaled modal inputs and outputs.

```
OUT = id_rm(IN,multi)
[psib,cpsi,new_res,new_po] = id_rm(res ,po,ci.IDopt)
[phib,cphi,new_res,new_po] = id_rm(Rres,po,ci.IDopt)
[psib,cpsi,new_res,new_po] = id_rm(res ,po,ci.IDopt,multi)
```

Description

`id_rm` is more easily called using the `idcom` GUI figure `Postprocessing` tab, see section 2.4.

`IN` is a data structure (see `xfopt` shapes at DOFs). Required fields are `IN.res` residues, `IN.po` poles, and `IN.idopt` identification options. Options used by `id_rm` are `.FittingModel` (Posit, Complex or Normal modes), `.NSNA` (number of sensors/actuators), `.Reciprocity` (not used, 1 FRF or true MIMO), `.Collocated` (indices of colloc. FRF when using reciprocity).

`multi` is an optional vector giving the multiplicity for each pole in `IN.po`.

`OUT` is a structure with fields (this format is likely to change in the future)

<code>.po</code>	poles with appropriate multiplicity
<code>.def</code>	output shape matrix (CPSI)
<code>.DOF</code>	Sensor DOFs at which <code>.DEF</code> is defined
<code>.psib</code>	input shape matrix (PSIB)
<code>.CDOF</code>	indices of collocated FRFs
<code>.header</code>	header (5 text lines with a maximum of 72 characters)

The low level calls giving `res`, `po` and `ci.IDopt` as arguments are obsolete and only maintained for backward compatibility reasons.

As shown in more detail in section 2.4, the residue matrix R_j of a single mode is the product of the modal output by the modal input. For a model in the residue form (residue `res`, poles `po` and options `IDopt` identified using `id_rc` for example), `id_rm` determines the modal input `psib` and output `cpsi` matrices such that

$$[\alpha(s)] = \sum_{j=1}^{2N} \frac{\{c\psi_j\} \{\psi_j^T b\}}{s - \lambda_j} \approx \sum_{j=1}^{2N} \frac{[R_j]}{s - \lambda_j} \quad (9.6)$$

The residues can be either complex mode residues or normal mode residues. In that case the normal mode input `phib` and output `cphi` matrices are real.

The `new_res` matrix is the minimal approximation of `res` corresponding to the computed input and output matrices. `id_rm` uses the number of sensors `IDopt(7)` and actuators `IDopt(8)`.

For MIMO systems (with the both the number of sensors `IDopt(7)` and actuators `IDopt(8)` larger than 1), a single mode has only a single modal output and input which implies that the residue matrix should be of rank 1 (see section 2.4.1). Residue matrices identified with `id_rc` do not verify this rank constraint. A minimal realization is found by singular value decomposition of the identified residue matrices. The deviation from the initial model (introduced by the use of a minimal model with isolated poles) is measured by the ratio of the singular value of the first deleted dyad to the singular value of the dyad kept. For example the following output of `id_rm`

```
Po #   freq      mul   Ratio of singular values to maximum
  1  7.10e+02  2   : 0.3000 k  0.0029
```

indicates that the ratio of the second singular value to the first is significant (0.3) and is kept, while the second dyad can be neglected (0.0029).

For a good identification, the ratios should be small (typically below 0.1). Large ratios usually indicate poor identification and you should update the poles using `id_rc` in a broad or narrow band update. Occasionally the poles may be sufficiently close to be considered as multiple and you should keep as many dyads as the modal multiplicity using the input argument `multi` which gives the multiplicity for each pole (thus the output shown above corresponds to a multiplicity of 2).

`id_rm` also enforces **reciprocity** conditions in two cases

- `IDopt(12)=1`. One transfer function is declared as being collocated. Reciprocity is only applied on the input and output coefficients linked to the corresponding input/output pair.
- `IDopt(12)=na`. As many collocated transfer functions as actuators are declared. The model found by `id_rm` is fully reciprocal (and minimal of course).
- in other cases `IDopt(12)` should be either 0 (no collocated transfer) or equal to `-nc` (`nc` collocated transfers but reciprocal scaling is not desired).

It is reminded that for a reciprocal system, input and output shape matrices linked to collocated inputs/outputs are the transpose of each other ($b = c^T$). Reciprocal

scaling is a requirement for the determination of non-proportionally damped normal mode models using `id_nor`.

In MIMO cases with reciprocal scaling, the quality indication given by `id_rm` is

Po#	freq	mul	sym.	rel.e.
1	7.10e+02	2 :	0.0038	0.0057

which shows that the identified residue was almost symmetric (relative norm of the anti-symmetric part is 0.0038), and that the final relative error on the residue corresponding to the minimal and reciprocal MIMO model is also quite small (0.0057).

Warnings

- `id_rm` is used by the functions: `id_nor`, `res2nor`, `res2ss`
- Collocated force to displacement transfer functions have phase between 0 and -180 degrees, if this is not true you cannot expect the reciprocal scaling of `id_rm` to be appropriate and should not use `id_nor`.
- `id_rm` only handles complete MIMO systems with NS sensors and NA actuators.

See also `idcom`, `id_rc`, `id_nor`, the `demo_id` demonstration

iicom

Purpose *UI command* function for FRF data visualization.

Syntax

```
iicom CommandString  
iicom(ci,'CommandString') % specify target figure with pointer  
out = iicom('CommandString')
```

Description `iicom` is a standard *UI command* function which performs operations linked to the data visualization within the `iipplot` interface. A tutorial can be found in section 2.1. Commands are text strings telling `iicom` what to do. If many `iipplot` figures are open, one can define the target giving an `iipplot` figure handle `ci` as a first argument. `iicom` uses data stored in a stack (see section 2.1.2). `iicom` does not modify data. A list of commands available through `iicom` is given below. These commands provide significant extensions to capabilities given by the menus and buttons of the `iipplot` *command figure*.


Commands `;`, `command`

The `commode` help details generic command building mechanisms. Commands with no input (other than the command) or output argument, can be chained using a call of the form `iicom(';Com1;Com2')`. `commode` is then used for command parsing.

`cax i`, `ca+`

Change current axes. `cax i` makes the axis `i` (an integer number) current. `ca+` makes the next axis current. For example, `iicom(';cax1;show rea;ca+;show ima')` displays the real part of the current FRFs in the first axis and their imaginary part in the second. (See also the `iicom` `Sub` command). The button indicates the number of the current axis. Pressing the button executes the `ca+` command.

`ch+`, `ch-`, `ch[+,-]i` : `next/previous`

Next/Previous . These commands/buttons are used to scan through plots of the same kind. For `iipplot` axes, this is applied to the current data sets. For `fepplot` axes, the current deformation is changed. You can also increment/decrement channels using the `+` and `-` keys when the current axis is a plot axis or increment by more than 1 using `iicom('ch+i')`.

`ch i, chc i, chall, ... select channel`

Display channels/poles/deformations *i*. Channels refer to columns of datasets, poles or deformations. `ch` / `chc` respectively define the indices of the channels to be displayed in all /the current drawing axes. The vector of indices is defined by evaluating the string *i*. For example `iicom ch[1:3]`, displays channels 1 to 3 in all axes.

For `Multi-dim curve` with dimension labels in the `.Xlab` field, `ChAllMyLabel` selects all channels associated with dimension `MyLabel`. This can be used to show responses at multiple operating conditions (typically stored as third or fourth dimension of `curve.Y`).

For multi-channel curves one can define the dimension name referring to the `Xlab` field in a cell array `iicom(ci,'ch','Xlabname',i)`. For this to work properly note that all `Xlabname` entries must be different (*e.g.* several `Unknown` entries must thus be avoided).

```
% Build a multi-dim curve, see sdtweb('demosdt.m#DemoGartteCurve')
r1=demosdt('demoGartteCurve')
ci=iiplot;ci.Stack={};ci=iiplot(r1);
iicom('ChAllzeta') % All channels that correspond to 'zeta' r1.Xlab{4}
% Cell selection with Xlab string and indices
iicom('ch',{'Input DOFs',[1,2]}) % Accessible with 'pick' button
iicom('curtabChannel')
```

Cursor, ods

The cursor is usually started with the axes context menu (right click on a given axis).

`iicom CursorOnFeplot` shows a cursor on the `iiplot` curve that let you show corresponding time deformation in `feplot`. (See `fecom CursorNodeIiplot` command for more details).

`iicom('ods')` provides an *operational deflection shape* cursor.

Curve[Init,Load,Save,Reset, ...]

These commands are used to manipulate datasets.

Most of them are of the form `iicom('Curve...',CurveNames)`. Then `CurveNames` can be a string with a curve name, a cell array of string with curve names or a regular expression (beginning by `#`) to select some curve names. If `CurveNames` is omitted,

a curve a dialog box is opened to select targeted curves. Otherwise these commands can be accessed through the GUI, in the **Stack** tab of the **iiplot** properties figure.

- **CurveInit** is used to initialize a display with a new dataset. `iicom('CurveInit', 'Name', C)` adds a **'curve', 'Name'** entry and displays this set in a new tab. To add multiple curves use `iicom('CurveInit', {'curve', 'N1', C1; 'curve', 'N2', C2})`. Use the curve **PlotInfo** to control how this initial display is performed.

- **CurveLoad** lets you load datasets.

`iicom('CurveLoad FileName')` loads curves stored in *FileName*.

`iicom('CurveLoad')` opens a dialog box to choose the file containing curves to load. If the file contains multiple curves, one can select the curves to be loaded in a cell array given as a second argument. For example,

```
ci=iicom('CurveLoad', 'gartid.mat')
```

loads the **gartid** data in an **iiplot** figure. Command option **-append** (`iicom(ci, 'CurveLoad -append MyFile')`) lets you append loaded curves to existing curves in the stack (by default existing curves are replaced). Command option **-hdf** (`iicom(ci, 'CurveLoad -hdf MyFile')`) lets you load curves under the **sdt** hdf format. Only pointers to the data stacked in **iiplot** are thus loaded. Visualizations and data transformation can be performed afterwards. Command option **-back** does not generate any visualization in **iiplot**. This can be useful in combination to **-hdf**, as the user can then fully control the data loaded in RAM.

- **CurveSave** lets you save **iiplot** stack data.

`iicom('CurveSave FileName', CurveNames)` saves the curves **CurveNames** in the **.mat** file given by *FileName*. If *FileName* is omitted a GUI is opened. To save more than 2 GB of data, or to save in the new MATLAB file formats (**-v7.3**), use the SDT **V6Flag**: `setpref('SDT', 'V6Flag', '-v7.3')`.

```
fname=fullfile(sdtdef('tempdir'), 'IicomSaveTestmat')
iicom(['CurveSave' fname], {'IIxi'; 'IdMain'})
```

- **CurveNewId CurveName** opens new **iiplot** figure for identification of the curve *CurveName* of the **ci** stack with **idcom**.

`iicom('CurveLoadId', FileName)` loads from *FileName* into for identification.

- **CurveRemove** removes the curves from the stack of the **iiplot** figure.

```
iicom('CurveRemove', CurveNames);
```

- `CurveReset` defines an empty curve stack to renew your work.
- `CurveJoin` combines datasets that have comparable dimensions. In particular first dimension (time, frequencies ...) must be the same. For example it is useful to combine dataset from parameter studies (same dimension).
`iicom('CurveJoin',CurveNames);`
Curves targeted by `CurveNames` (or selected curves in `iiplot`) are joined and replace the first curve in the `iiplot` stack.
- `CurveCat` concatenates dataset that have the same dimensions. For example it is useful to combine dataset from successive time simulation. Syntax is the same as for `iicom CurveJoin` command. One can use following command options:

`-follow` to remove last value of first abscissa before concatenate.

`-shift` to shift abscissa of second dataset of the last value of first dataset abscissa.

`ga i`

Get handle to a particular axis. This is used to easily modify handle graphics properties of `iiplot` axes accessed by their number. For example, you could use `set(iicom('ga1:2'),'xgrid','on')` to modify the grid property of `iiplot` axes 1 and 2.

If you use more than one `feplot` or `iiplot` figure, you will prefer the calling format `cf=iiplot; set(cf.ga(1:2),'xgrid','on')`.

`head [Main,Text,Clear]`

Note : the preferred approach is now to define fixed displays using `comgui objSet` commands stored in the curve `PlotInfo ua.axProp` entry. For example

```
C1=fe_curve('testSine f 5',linspace(0,10,100e3));
r1={'@title',{'String','Main Title','FontSize',16}};
C1=sdsetprop(C1,'PlotInfo.ua.axProp',r1{:});
iicom('curveinit','SineWithFixedTitle',C1);
```

For backward compatibility, header axes are still supported (the change is to `objSet` allows better tab switching). Header axes are common to all plot functions and span the full figure area (normalized position `[0 0 1 1]`). You can get a pointer to this axis with `cf.head` and add any relevant object there.

```
ci=iicom('curveload','gartid') % Load a test case
h=text(0,0,'Main Title', ...
    'parent',ci.head,'unit','normalized','position',[.05 .95], ...
    'fontsize',20,'fontname','Times', ...
    'tag','iimain')
iimouse('textmenu',h); % Allow Editing

iicom('HeadClear') deletes all objects from the header axis of the current figure.
```

Data set selection `IIx:name [On,Off,Only]`, `cIIx ...`

Curve set selection for display in the current axis.

`IIxf:TestOnly` displays the `ci.Stack{'Test'}` data set only in all axes (`on` and `off` turn the display on or off respectively). By adding a `c` in front of the command (`cIIx:Test` for example), the choice is only applied to the current axis. You can also toggle which of the data sets are shown using the `Variables` menu (applies to all axes) or axis context menu applies to (current axis).

The alternate calling format `iicom('iix',{'Test','IdFrfr'})` can be used to specify multiple sets to display. `iicom('iixOnly',{'Test','IdFrfr'})` will display those two sets only.

`IIxf`, `IIxe`, `IIxh`, `IIxi [On,Off]` are still supported for backward compatibility.

Polar

Polar plots are used for cases where the abscissa is not the standard value. Accepted values (use a command of the form `Polar val`) are

- `-1` abscissa is the channel before the one displayed. In a curve with channels `[X Y]` display `Y`, channel 2, and use `X,channel 1`, as abscissa.
- `xi` uses i^{th} column of `def.data` when displaying FEM time signals. This is typically used when this second column is an other form of abscissa (angle for rotating machines, ...)
- `i` with `i0` uses the specified channel as abscissa.
- `Off` or `0` turns off polar plots.

`PoleLine [,c] [,3], IIpo, ...`

Pole line display. are dotted vertical lines placed at relevant abscissa values. These lines can come from

- standard curves with an `curve.ID` field, see `ii_plp` Call from `iiplot`.
- frequencies of poles in `ci.Stack{'IdMain'}` in black and `ci.Stack{'IdAlt'}` in red.

By itself, `PoleLine` toggles the state of pole line display. The `c` option applies the command to the current axis only. `PoleLine3` places the lines on the pole norm rather than imaginary part used by default (this corresponds to the `ii_plp` formats `2` and `3`).

The state of the current axis (if it is an `iiplot` axis) can also be changed using the `IIPlot:PoleLine` menu (`PoleLineTog` command).

Low level commands `IIpo` and `IIpo1` are low level commands force/disable display of pole lines in the main identified model

`ci.Stack{'IdMain'}.po` or the alternate set `ci.Stack{'IdAlt'}.po`. With `cIIpo` the choice is only applied to the current axis. These options are usually accessed through menus.

`ImWrite, ...` 

`comgui ImWrite` is the generic command used to generate a clean printout of figures. It supports many basic capabilities, filename generation, cropping, ..

When using `iiplot` and `feplot`, it may often be interesting to generate multiple images by scanning through a selected range of channels. A command of the form `iicom(cf,'ImWrite',RO)` is then used with `RO` a structure containing generic image capture fields (see `comgui ImWrite`) and fields specific to multi-image capture

- `.ShowFcn` the callback that is executed for each image to be generated. The default is `fecom(cf,sprintf('ch %i',ch));` for `feplot`.
- `.ch` a vector of channel indices that will give an index for each image. With the string `all`, all the channels are used.
- `.ImWrite` is the command used to call `comgui` with the default `'imwrite -ftitle'`.

- `.FileName` if present replaces any other file name generation mechanism. Your `ShowFcn` callback can thus implement your own file name generation mechanism.
- `.HtmWidth` can specify an HTML view width which differs from the image width.
- `.RestoreFig=1` can be used to restore the figure and display after image generation.
- `.RelPath` optional integer giving the level of relative path to be retained (1 keeps just the file name, 2 the directory containing the images, ...). This is useful to create HTML report files that can be moved.

To automate figure generation, it is typically desirable to store image capture information in the set of deformations or the curve. A `curve.ImWrite` field in `iipLOT` can be used to predefine the option structure, for user defined dynamic change of settings, defining a `ua.PostFcn` callback (see `iipLOT PlotInfo`) is typically the appropriate approach. For `feplot`, `def.ImWrite` is used for multi-image capture but more evolved file name generation is found using `comgui def.Legend`.

```
% Example of 4 views in feplot
cf=demosdt('DemoGartFEplot')
cf.def=sdsetprop(cf.def,'Legend', ...
    'string',{'Garteur FE';'$Title'}, ... % Define a two line title
    'ImWrite', ... % Name generation, see sdtweb('comgui#def.Legend')
    {sdtdef('tempdir'),'Root',1,2,'$v.png'});
RO=comgui('imfeplot4view'); % Predefined strategy to generate 4 views
cf.def.ImWrite=RO; %
fecom(cf,'ImWrite')

% Example of two channels in iipLOT, with finish on same view
ci=iicom('curveload GartId');iicom('ch20')
ci.Stack{'Test'}.ImWrite=struct('ch',1:2, ...
    'FileName',fullfile(sdtdef('tempdir'),'Test$ch1.png'), ...
    'RestoreFig',1);
iicom(ci,'ImWrite')
```

`comgui('ImFeplot')` returns a list of standard calls to options for image generation.








Pro

`iicom('ProFig')` shows or hides the properties figure.
`iicom(ci,'ProRefreshIfVisible')` refreshes the property figure when it is visible.
`iicom(ci,'ProInit')` reinites the property figure.

Show plot type

Specify the current axis type. The `iipplot` plot functions support a number of plot types which can be selected using the **Show** menu/command. From command line, you can specify the target axis with a `-cax i` option.

The main plot types are

- 2D ($f(x)$) plots are associated with the following buttons  **Abs** (absolute value),  **Pha** phase,  **Phu** unwrapped phase,  **Rea** real part,  **Ima** imaginary part,  **R&I** real and imaginary,  **Nyq** Nyquist.
- 3D ($f(x, y)$) plots are **image**, **mesh**, **contour** and **surface**. **Show3D** gives time-frequency representation of the log of the abs of the signal displayed as and image. The `ua.yFcn` callback operates on the variable called **r3** and can be used for transformations (absolute value, phase, ...). Note that you may then want to define a colorbar see `iipplot PlotInfo`.

```
R1=d_signal('Resp2d'); % load 2d map
R1.PlotInfo= ii_plp('plotinfoTimeFreq -yfcn="r3=r3" type "contour"');
ci=iicom('curveinit','2DMap',R1);

% or
R1.PlotInfo={}; ci=iicom('curveinit','2DMap',R1);
ci=iicom('curveinit','2DMap',R1);
iicom('show3D -yfcn="r3=log10(abs(r3))" type "contour"')
```

- `idcom` specialized plots see `iipplot TypeIDcom`: **mmi** MMIF of **Test**, **fmi** forces of MMIF of **Test**, **ami** alternate mode indicator of **Test**, **SUM** of **Test**, **CMIF** of **Test**, **sumi** sum imaginary part of **Test**, **pol** poles in **IdMain**, **fre** freq. vs. damping in **IdMain**, **rre** real residue in **IdMain**, **cre** complex residue of **IdMain**, **lny** local Nyquist of **Test** (superposition around current pole), **err** Nyquist **Error** for current pole, **Quality** for all poles
- `feplot` plots.

SubSave, SubSet

`SubSave i` saves the current configuration of the interface in a stack entry `TabInfo`. This configuration can then be recalled with `SubSet i`. The `TabInfo` entry is also augmented when new curves are shown so that you can come back to earlier displays. `SubSetI i` selects an index in the `TabInfo` stack.

SubToFig

`SubToFig` copies the `iiplot` figure visualization to a standard matlab figure, thus allowing an easier handling to produce customized snapshots (see also `iicom ImWrite`). Reformatting is then typically performed with `comgui objSet`.

Command option `-cf i` forces the visualization output to figure `i`.

Command option `leg i` allows `iiplot` legend handling in the visualization. `leg0` removes the legend, `leg1` keeps it as in `iiplot`, `leg2` transforms the `iiplot` legend in a standard matlab legend. The legend is removed by default.

Sub *plot init*

This command is the entry point to generate multiple drawing axes within the same figure.

`iicom Sub` by itself checks all current axes and fixes anything that is not correctly defined.

Accepted command options are

- `MagPha` gives a standard subdivision showing a large amplitude plot and a small wrapped phase plot below.
- `Iso` gives a standard 2 by 2 subdivision showing four standard 2-D projections of a 3-D structure (this is really used by `fepplot`).
- `i j k` divides the figure in the same manner as the MATLAB `subplot` command. If `k` is set to zero all the `i` times `j` axes of the subplot division are created. Thus the default call to the `Sub` command is `Sub 2 1` which creates two axes in the current figure. If `k` is non zero only one of these axes is created as when calling `subplot(i, j, k)`.

As the `subplot` function, the `Sub` command deletes any axis overlapping with the new axis. You can prevent this with command option `nd`.

Standard subdivisions are accessible by the `IIplot:Sub commands` menu.

Note that `set(cf.ga(i), 'position', Rect)` will modify the position of `iplot` axis `i`. This axis will remain in the new position for subsequent refreshing with `iplot`.

- `step` increments the deformation shown in each subplot. This is generally used to show various modeshapes in the same figure.
- `Reset` forces a reinit of all properties. For example `SubMappha Reset`.

`TitOpt [,c]i, title and label options`

Automated title/label generation options. `TitOpt i` sets title options for all axes to the value `i`. `i` is a 5 digit number with

- units corresponding to `title`. For modes `[None,ModeNumber,Name]`.
- decades to `xlabel` 0 none, 1 label and units, 2 label.
- hundreds to `ylabel` 0 none, 1 label and units, 2 label.
- thousands to `zlabel` 0 none, 1 label and units, 2 label.
- `1e4` to legend/title switching.

The actual meaning of options depends on the plot function (see `iplot` for details). By adding a `c` after the command (`titoptc 111` for example), the choice is only applied to the current axis.

When checking the axes data (using `iicom Sub` command), one rebuilds the list of labels for each dataset using `iicom('chlab')`. This cell array of labels, stored in `ci.ua.chlab`, gives title strings for each channel (in rows) of datasets (in columns) with names given in `ci.ua.sList`. The label should start with a space and end with a comma. The dataset name is added if multiple datasets are shown.

Modifying the `ci.IDopt.unit` value changes the unit assumed for identification but not the dataset units.

Titles and labels are not regenerated when using `ch` commands. If something is not up to date, use `iicom Sub` which rechecks everything.


Scale : `xlin,xlog ...`


Default values for `xscale` and `yscale`. `xlin`, `xlog`, `ylin`, `ylog`, set values. `xy+1`, `xy+2` are used to toggle the `xscale` and `yscale` respectively (you can also use the **I**plot:`xlin` and **I**plot:`ylin` menus). Other commands are `xy1` for x-lin/y-lin, `xy2` for x-log/y-lin, `xy3` for x-lin/y-log, `xy4` for x-log/y-log.

You can all use the `all` option to change all axes: `iicom('xlog all')`.

`ytight[on,off,]` can be used to obtain tight scales on the `y` axis. The `x` axis is typically always tight. Automated `ztight` is not yet supported.

Limits : `wmin, xlim, xmin, xmax, wmo, w0, ...`

Min/max abscissa selection is handled using the fixed zoom (graphically use  button). Accepted commands are

- `xlim min max` (or the legacy `wmin f1 f2`).
- `xmin min` (or the legacy `wmin f1`)
- `xmax max` (or the legacy `wmax f1`)
- `wmo` allows a mouse selection of the minimum and maximum value (same as  button).
- `w0` resets values (same as double click after hitting the button)

When performing identification with `idcom` the fixed zoom corresponds to the working frequency range and corresponds to indices in `ci.IDopt(4:5)` (see `IDopt`, turn off with `idcom('Off')`). The index of the frequency closest to the specified min/max is used. When viewing general responses, the information for the abscissa limits is stored in the axis and is thus lost if that axis is cleared.

See also [iipplot](#), section 2.1, [idcom](#)

iimouse

Purpose Mouse related callbacks for GUI figures.

Syntax

```
iimouse  
iimouse('ModeName')  
iimouse('ModeName',Handle)
```

Description `iimouse` is the general function used by `feplot` and `iiplot` to handle graphical inputs. While it is designed for *SDT* generated figures, `iimouse` can be used with any figure (make the figure active and type `iimouse`).

The main mouse mode is linked supports zooming and axis/object selection (see `zoom`). Context menus are associated to many objects and allow typical modifications of each object. When an axis is selected (when you pressed a button while your mouse was over it), `iimouse` reacts to a number of keys (see `key`). An active cursor mode (see `Cursor`) has replaced the information part of previous versions of `iimouse`. 3-D orientation is handled by `view` commands.

On,Off

`iimouse` with no argument (which is the same as `iimouse('on')`) turn `zoom`, `key` and context `menu` on.

In detail, the `figure` is made `Interruptible`, `WindowButtonDownFcn` is set to `iimouse('zoom')` and `KeyPressFcn` to `iimouse('key')`.

Plot functions (`iiplot`, `feplot`) start `iimouse` automatically.

`iimouse off` turns all `iimouse` callbacks off.

clip [Start,Undo]

This command is used to eliminate faces not contained within the area that the user selects with a dragging box. `ClipUndo` clears the current axis and calls `feplot` to reinitialize the plot.

zoom

This is basic mode of `iimouse`, it supports

- click and drag zoom into an area for both 2-D and 3-D plots (even when using perspective).
- zoom out to initial limits is obtained with a double click or the **i** key (on some systems the double click can be hard to control).
- active axis selection. **iimouse** makes the axis on which you clicked or the closest to where you clicked active (it becomes the current axis for **feplot** and **iiplot** figures).
- **colorbar** and **triax** axes automatically enter the **move** mode when made active
- **legend** axes are left alone but kept on top of other axes.

Context menus are described in section 2.1.1 and section 4.1.1.

Cursor

When you start the cursor mode (using the context menu opened with the right mouse button or by typing the **c** key), you obtain a red pointer that follows your mouse while displaying information about the object that you are pointing at. You can stop the cursor mode by clicking in the figure with your right mouse button or the **c** key. The object information area can be hidden by clicking on it with the right mouse button.

For **feplot** figures, additional information about the elements linked to the current point can be obtained in the MATLAB command window by clicking in the figure with the left button. By default, the cursor follows nodes of the first object in the **feplot** drawing axis. If you click on another object, the cursor starts pointing at it. In the wire-frame representation, particularly when using OpenGL rendering, it may be difficult to change object, the **n** key thus forces the cursor to point to the next object.

For **iiplot** axes, the cursor is a vertical line with circles on each data set and the information shows the associated data sets and values currently pointed at.

For **ii_mac** axes the current value of the MAC is shown.

key

Keyboard short-cuts. Some commands are directly available by pressing specific keys when a plot axis is active (to make it active just click on it with your mouse). The short cuts are activated by setting the **KeyPressFcn** to **iimouse('key')** (this is done by **iimouse on**). Short cuts are:

<code>a,A</code>	all axis shrink/expand	<code>u,U</code>	10° horizontal rotation
<code>c</code>	start <code>iimouse</code> cursor	<code>v,V</code>	10° vertical rotation
<code>i</code>	return to initial axis limits	<code>w,W</code>	10° line of sight 10° rotation
<code>l,L</code>	smaller/larger <code>fecom</code> <code>scaleddef</code>	<code>x,X</code>	x/horizontal translation
<code>n</code>	cursor on next <code>fecom</code> object	<code>y,Y</code>	y/vertical translation
		<code>z,Z</code>	z/line of sight translation
<code>-,_</code>	previous (<code>iicom ch-</code>)	<code>+,=</code>	next (<code>iicom ch+</code>)
<code>1,2,3,4</code>	see <code>view</code> commands	<code>?</code>	list keyboard shortcuts

The list of the associated call is accessible using the call `sdt_table_generation('KeyPressTable.feplot')` or `sdt_table_generation('KeyPressTable.iipplot')`.

For `feplot` axes the translations are based on camera movements and correspond to the horizontal, vertical and line of sight for the current view. Translating along the line of sight has no effect without perspective and is similar to zooming with it. For other axes, the `xyz` keys translate along the data `xyz` directions.

move

The object that you decided to move (`axes` and `text` objects) follows your mouse until you click on a final desired position. This mode is used for `triax` (created by `feplot`) and `colorbar` axes, as well as text objects when you start `move` using the context menu (right button click to open this menu).

The `moveaxis` used for `legend` as a slightly different behavior. It typically moves the axis while you keep the button pressed.

You can call move yourself with `iimouse('move',Handle)` where `Handle` must be a valid `axes` or `text` object handle.

text

This series of commands supports the creation of a context menu for `text` objects which allows modification of font properties (it calls `uisetfont`), editing of the text string (it calls `edtext`), mouse change of the position (it calls `iimouse`), and deletion of the text object.

You can make your own text objects active by using `iimouse('textmenu',Handle)` where `Handle` must contain valid `text` object handle(s).

view, cv

`iimouse` supports interactive changes in the 3-D perspective of axes. Object views are controlled using azimuth and elevation (which control the orientation vector linking the `CameraTarget` and the `CameraPosition`) and self rotation (which control the `CameraUpVector`). You can directly modify the view of the current axis using the MATLAB `view` and `cameramenu` functions but additional capabilities and automated orientation of triax axes are supported by `iimouse`.

1	first standard view. Default <code>n+y</code> . Changed using the <code>View default</code> context menu.
2	standard <code>xy</code> view (<code>n+z</code>). Similar to MATLAB <code>view(2)</code> with resetting of <code>CameraUpVector</code> . Changed using the <code>View default</code> context menu.
3	standard view. Default to MATLAB <code>view(3)</code> .
4	standard view. Default <code>n+x</code> .
<code>n[+,-][x,y,z]</code>	2-D views defined by the direction of the camera from target.
<code>n[+,-][+,-][+,-]</code>	3-D views defined by the signs projection of line of sight vector along the <code>xyz</code> axes.
<code>dn ...</code>	<code>dn</code> commands allow setting of default <code>1234</code> views. Thus <code>viewdn-x</code> will set the 4 view to a normal along negative <code>x</code>
<code>az el sr</code>	specify azimuth, elevation and rotation around line of sight
<code>g rz ry rz</code>	specify rotations around global <code>xyz</code> axes
<code>[x,y,z][+,-] ang</code>	rotation increments around global <code>xyz</code> axes
<code>[h,v,s][+,-] ang</code>	current horizontal, vertical and line of sight axes

All angles should be specified in degrees.

`iimouse key` supports rotations by +/- 10 degrees around the current horizontal, vertical and line of sight axes when any of the `u, U, v, V, w, W` keys are pressed (same as `fecom('viewh-10')` ...). `1, 2, 3, 4` return to basic 2-D and 3-D views.

`iimouse('cv')` returns current view information you can then set other axes with `iimouse('view', AxesHandles, cv)`.

See also

`iicom`, `fecom`, `iiplot`

iipplot

Purpose Refresh all the drawing **axes** of the **iipplot** interface.

Syntax `iipplot`

Description `iipplot` used as a command with no arguments is the user entry point to refresh all the drawing axes. Section 2.1 gives an introduction to the use of `iipplot` and the companion function `iicom`.

Each `iipplot` axis can display some or all data sets in their stack. The selection of what is displayed is obtained using the `iicom IIX` commands or the **Variables** menu.

You can create a new `iipplot` axis using a curve data structure **Curve** (generated by `fe_curve` for example), simply calling `iipplot(Curve)`. One can also add a stack of curve with the same mechanism by giving a curve stack in **Curve**.

Plot types supported by `iipplot` are described below. The plot type can be selected using the **PlotType** menu of the toolbar or through `iicom Show` commands.

Selected channels (columns of the data sets) are shown for all plots. The `iicom` commands `+`, `-`, `ch` and the associated keys and toolbar buttons can be used to change selected channels.

Pole lines for the indication of pole frequencies are available for many plots. The `iicom PoleLine` commands and the `IIPplot:PoleLine` menu can be used to change how these lines appear. `ci.Stack{'IdMain'}` pole lines are shown in white/black. `ci.Stack{'IdAlt'}` pole lines in red.

`ci` : `handle`

`ci=iipplot` returns a *SDT handle* to the current `iipplot` figure (2nd optionnal output argument is **XF**, a pointer to the curve stack, see section 2.1.2). You can create more than one `iipplot` figure with `ci=iipplot(FigHandle)`.

PlotInfo

Curves to be display can contain a `C1.PlotInfo` cell array where the first column gives the type as detailed below and the second the associated data.








- `LineProp` specifies properties to be used as properties for lines. For example `C1=sdsetprop(C1,'PlotInfo','LineProp',{ 'LineWidth',2})`. This is checked at each display.
- `sub`, `show`, `scale` commands to be executed when initializing a display tab with `iicom Sub`.
- `ua.PostFcn` commands executed at the end of a refresh. This gives the user a chance to introduce modifications to the result of `iipplot`.
- `ua.TickFcn` commands executed whenever a mouse zoom is done, see `TickFcn`.
- `ua.axProp` is a cell array containing properties to be applied with an `comgui objSet` command.
- `ColorBar` is a cell array containing properties to be used to generate a colorbar. See `fecom ColorBar` for more details. For example

```
C1=d_signal('Resp2D');
C1=sdsetprop(C1,'PlotInfo','ColorBar',{ 'YAxisLocation','left'});
iicom('curveinit','2D',C1);
```

- `LDimPos` specifies the dimension used to generate the label on the response axis (y for $f(x)$, z for $f(x,y)$).

The `ii_plp('PlotInfo',C1)` command provides default values for classical configurations.

Type

- 2D ($f(x)$) plots are associated with the following buttons and `iicom Show` commands:  `Abs` (absolute value),  `Pha` phase,  `Phu` unwrapped phase,  `Rea` real part,  `Ima` imaginary part,  `R&I` real and imaginary,  `Nyq` Nyquist.
- 3D ($f(x,y)$) plots are `image`, `mesh`, `contour` and `surface`. For this plots `ua.XDimPos` should give the positions of dimensions associated with the x and y variations. Proper `.PlotInfo` can be generated with `ii_plp('PlotInfo2D'-type "contour",C1)`.

DimPos and channel

When displaying multi-dimensional curves as 2D plots $f(x)$, the abscissa x is taken to be the first dimension declared in the `C1.DimPos` field (with a default at 1).

When displaying as 3D ($f(x, y)$) plots, the x, y are taken to be the first two dimensions declared in the `C1.DimPos` field (with a default at 1,2). You can then flip the positions in the plot axis by setting `ci.ua.XDimPos=[2 1]`.

Channels are indices for remaining dimensions.

The y (z for 3D) axis label is built using the `C1.DimPos(2)` dimension unless the curve contains a `LDimPos` entry.

TypeIDcom

Specialized plots for `idcom` are

- *Local Nyquist plots* (initialized by `show lny`) show a comparison of `Test` (measured FRFs) and `IdFrf` (identified model) in a reduced frequency band
$$\left[\begin{array}{cc} \omega_j(1 - \zeta_j) & \omega_j(1 + \zeta_j) \end{array} \right]$$
 near the currently selected pole (the current pole is selected by clicking on a pole line in another plot axis). Local Nyquist plots allow a local evaluation of the quality of the fit. The `error` and `quality` plots give a summary of the same information for all the frequency response functions and all poles.
- *Multivariate Mode Indicator Function* (initialized by `show mmi`), *forces associated to the MMIF* (initialized by `show fmi`), *Alternate Mode Indicator Function* (`show ami`), and *Channel Sum* (`show sum`) are four ways to combine all the FRFs or a set to get an indication of where its poles are located. These indicators are discussed in the `ii_mmif Reference` section. They are automatically computed by `iiplot` based on data in the 'Test' set.
- *Pole locations in the complex plane* (initialized by `show pol`).
- *Poles shown as damping vs. frequency* are initialized by `show fre`.
- *Position of residues in the complex plane* are initialized by `show cre`. This plot can be used to visualize the phase scatter of identified residues.
- *Value of real residue for each measured channel* are initialized by `show rre`.

- **Error** *Local Nyquist error* (initialized by `show err`). For the current pole, error plots select frequency points in the range $[\omega_j(1 - \zeta_j) \ \omega_j(1 + \zeta_j)]$. For each channel (FRF column), the normalized error (RMS response of `ci.Stack{'Test'}.xf - ci.Stack{'IdMain'}.xf` divided by RMS response of `ci.Stack{'Test'}`) is shown as a dashed line with `+` markers and a normalized response level (RMS response of `ci.Stack{'Test'}`) as a dashed line with `x` markers.

Normalized errors should be below 0.1 unless the response is small. You can display the error using the nominal sensor sort with `ci.Stack{'IdError'}.sort=0` and with increasing error using `sort=1`.

- **Quality** *Mode quality plot* (initialized by `show qua`), gives a mean of the local Nyquist plot. The dashed lines with `+` and `x` markers give a standard and amplitude weighted mean of the normalized error. The dotted line gives an indication of the mean response level (to see if a mode is well excited in the FRFs). Normalized errors should be below 0.1 unless the response is small.

See also [iicom](#), [iipplot](#), [setlines](#), [xfopt](#)

ii_cost

Purpose Compute the quadratic and log-least-squares cost functions comparing two sets of frequency response functions.

Syntax `[cst] = ii_cost(xf,xe)`

Description For two sets of FRFs H and \hat{H} , the quadratic cost function is given by

$$J_{ij}(\Omega) = \sum_{ij \text{ measured}, k \in \Omega} |\hat{H}_{ij}(s_k) - H_{ij}(s_k)|^2$$

and the log-least-square cost function by

$$J_{ij}(\Omega) = \sum_{ij \text{ measured}, k \in \Omega} \left| \log \left| \frac{\hat{H}_{ij}(s_k)}{H_{ij}(s_k)} \right| \right|^2$$

For sets `xf` and `xe` stored using the `xf` format (see page 167), `ii_cost` computes both those costs which are used in identification and model update algorithms (see section 3.2.3).

See also `id_rc`, `up_ixf`

ii_mac

Purpose

User interface function for MAC and other vector correlation criteria.

Syntax

```
ii_mac(cpa,cpb)
VC      = ii_mac(cpa,cpb,'PropertyName',PropertyValue, ...)
[VC,ReS] = ii_mac('PropertyName',PropertyValue, ... , 'Command')
ii_mac(Fig,'PropertyName',PropertyValue, ... , 'Command')
Result = ii_mac(Fig, 'Command')
VC.PropertyName = PropertyValue
```

Description

The `ii_mac` function gives access to vector correlation tools provided by the *SDT* starting with the Modal Assurance Criterion (MAC) but including many others. A summary of typical applications is given in section 3.2 and examples in the `gartco` demo.

Vector correlations are *SDT* objects which contain deformations, see `va`, typically given at test sensors. For criteria using model mass or stiffness matrices see `m`. Other details about possible fields of `VC` objects are given after the listing of supported commands below.

GUI

If you use `ii_mac` without requesting graphical output, the vector correlation object is deleted upon exit from `ii_mac`. In other cases, the object is saved in the figure so that you can reuse it.

You can add data to other fields or call new commands from the command line by starting the `ii_mac` call with a pointer to the figure where the vector correlation is stored (`ii_mac(fig,'Command'), ...`). An alternate calling form is to set a field of the vector correlation object.

The following commands

```
[cf,def_fem,res_test]=demosdt('demo gartte cor plot');
[m,k,mdof] = fe_mknl(cf.mdl);
Sens=fe_case(cf.mdl,'sens');
figure(1); subplot(221); % Make figure(1) current so that ii_mac uses it
VC=ii_mac(res_test,def_fem,'labela','Test','labelb','FEM', ...
          'sens',Sens,'Mac Pair Plot');
subplot(212);ii_mac(1,'comac'); % set new axis and display other criterion
VC.m = m; VC.kd = ofact(k+1e1*m);
subplot(222); VC.MacMPairPlot;
```

illustrate a fairly complex case where one shows the MAC in `subplot(221)`, all three COMAC indicators in `subplot(212)`, then provide mass and a mass-shifted stiffness to allow computation of the mass condensed on sensors and finally show the reduced mass weighted MAC in `subplot(222)`.

The `II_MAC` menu lets you choose from commands that can be computed based on the data that you have already provided. The `context` menu associated with plots generated by `ii_mac` lets you start the cursor, display tabular output, ...

You can link deformations shown in a `feplot` figure to a MAC plot using

```
[model,sens,ID,FEM]=demosdt('demopairmac');
cf=feplot(model);
cf.def(1)=ID; % display test as first def set
cf.def(2)=FEM; % display FEM as second def set
% overlay & show interactive MAC in fig 1:
figure(1);clf;fecom('show2def-mac1')
ii_mac(1,'mac table');
```

Main commands

`Options ... [Plot,Table,TeX,Thtml]`

By default, the commands plot the result in a figure. Options valid for all commands are

- `plot` generates figure in the current axis. You can use `figure` and `subplot` to set the current axis.
- `Table` generates a text output
- `TeX` generates a format suitable for direct inclusion in L^AT_EX
- `Thtml` creates and open an HTML file in the MATLAB browser.

Data fields

Data fields are defined using name, value pairs.

- `'cpa',dataAsCols` sets shapes . But calls with data structures are preferable, see `va`.
- `'sens',sens` sets sensor observation matrix, see `sens`.

- 'labela', 'name' sets the name of data set A. Typical values are `Test`, `FEM`, ...
- 'inda', `ind` selects vectors given by `ind` when computing a criterion. For example, rigid body modes are often not included in correlation. Thus 'indb', `7:20` would skip the first 6 modes.
- 'SubDofInd', `ind` allows the selection a subset of sensors when computing correlation criteria.

`MAC [,M] [,PairA,PairB,AutoA, ...] ...`

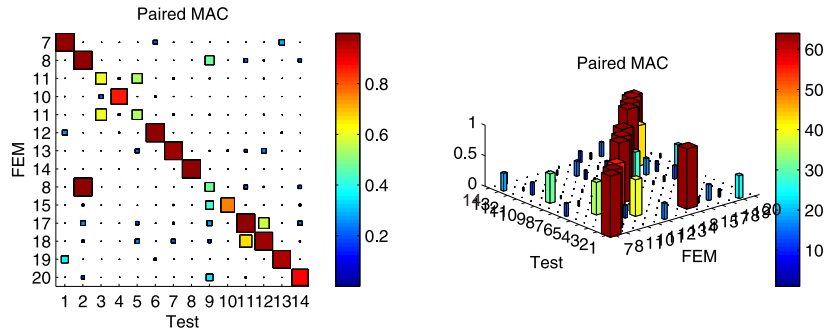
The Modal Assurance Criterion (MAC) [4] is the most widely used criterion for vector correlation (mainly because of its simplicity).

The MAC is the correlation coefficient of vector pairs in two vector sets `cpa` and `cpb` defined at the same DOFs (see `ii_mac va` for more details). In general `cpa` corresponds to measured modeshapes at a number of sensors $\{c\phi_{idj}\}$ while `cpb` corresponds to the observation of analytical modeshapes $[c]\{\phi_k\}$. The MAC is given by

$$\text{MAC}_{jk} = \frac{|\{c\phi_{idj}\}^H \{c\phi_k\}|^2}{|\{c\phi_{idj}\}^H \{c\phi_{idj}\}| |\{c\phi_k\}^H \{c\phi_k\}|} \quad (9.7)$$

For two vectors that are proportional the MAC equals 1 (perfect correlation). Values above 0.9 are generally considered as well correlated. Values below 0.6 should be considered with much caution (they may or may not indicate correlation).

The commands and figure below shows the standard 2-D (obtained using the context menu or `view(2)`) and 3-D (obtained using the context menu or `view(-130,20)`) representations of the MAC supported by `ii_mac`. The color and dimensions of the patches associated to each vector pair are proportional to the MAC value.



```
[model,sens,ID,FEM]=demosdt('demopairmac');
```

```
figure(1);clf;
ii_mac(ID,FEM,'sens',sens,'mac paira plot')
ii_mac(1,'mac paira table');
```

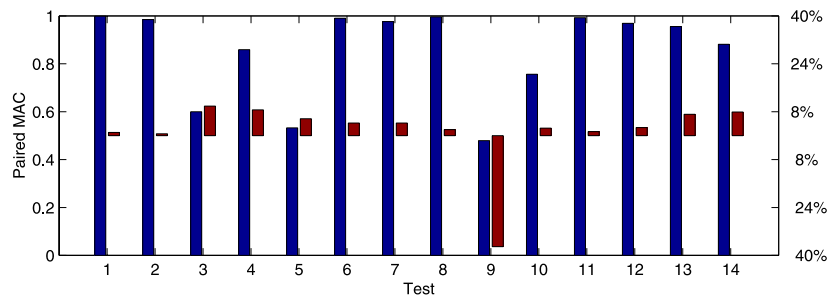
The basic MAC shows vector pairs for all vectors in the two sets. Specific command options are

- **MacM** should be used when a mass is provided, see **MacM**
- **MacPairA** command seeks for each vector in **cpa** (**cpb** with **PairB**) the vector in **cpb** (**cpa**) that is best correlated. **MacPairB** pairs against **cpb** vectors.
- **MacAutoA** Since the objective of the MAC is to estimate the correlation between various vectors, it is poor practice to have vectors known to be different be strongly correlated.

Strong correlation of physically different vectors is an indication of poor test design or poor choice of weighting. **MacAutoA (B)** compute the correlation of **cpa** (**cpb**) with itself. Off diagonal terms should be small (smaller than 0.1 is generally accepted as good practice).

- **-combineval** allows orthogonal linear combinations of vectors whose frequencies are closer than **val** relatively. This is meant for cases with very closely spaced modes where subspaces rather than individual vectors should be compared.
- **Error** computes the MAC (or MAC-M), does pairing and plots a summary combining the MAC value found for paired modes and the associated error on frequencies $((fb-fa)./fa)$. Typical calls can be found in **gartco** example.

By default this command displays a plot similar to the one shown below where the diagonal of the paired MAC and the corresponding relative error on frequencies are shown. For text output see general command options.



This is an example of how to use of the MACError command

```
[model,sens,ID,FEM]=demosdt('demopairmac');
ii_mac(ID,FEM,'sens',sens,'macerror plot')
ii_mac(ID,FEM,'sens',sens,'macerror table')
```

A few things you should know ...

- The MAC measures the shape correlation without any reference to scaling of each vector (because of the denominator in (9.7)). This makes the MAC easy to use but also limits its applicability (since the modeshape scaling governs the influence of a given mode on the overall system response, a proper scaling is necessary when comparing the relative influence of different modes). In other terms, the MAC is not a norm (two vectors can be very correlated and yet different), so care must be taken in interpreting results.
- As the MAC is insensitive to mode scaling, it can be used with identified normal mode residues. You do not have to determine modal masses (see [id_rm](#)) to compute a MAC.
- The main weakness of the MAC is linked to scaling of individual components in the correlation. A change in sensor calibration can significantly modify the MAC. If the natures of various sensors are different (velocity, acceleration, deformation, different calibration...) this can induce significant problems in interpretation.

- The reference weighting in mechanics is energy. For vectors defined at all DOFs, the MAC is a poor criterion and you should really use its mass weighted counter part. For incomplete measurements, kinetic energy can be approximated using a static condensation of the mass on the chosen sensors which can be computed using the `MacM` command.
- In certain systems where the density of sensors is low on certain parts, cross-correlation levels with the mass weighted MAC can be much lower than for the non weighted MAC. In such cases, you should really prefer the mass weighted MAC for correlation.

`MACCo [,M] [,ns]`

The `MACCo` criterion is a *what if* analysis. It takes modes in `cpa`, `cpb` and computes the paired MAC or MAC-M with one sensor removed. The sensor removal leading to the best mean MAC for the paired modes is a direct indication of where the poorest correlation is found. The algorithm removes this first sensor then iteratively proceeds to remove `ns` other sensors (the default is 3). The `MACCo` command used with command option `text` prints an output of the form

Test	1	2	3	4	5	6	7	8
FEM	7	8	11	10	11	12	13	14
Sensor Mean								
All	87	100	99	60	86	53	100	98
1112z	88	100	99	59	90	62	100	98
1301z	89	100	99	62	90	64	100	98
1303z	90	100	98	66	90	66	100	98

where the indices for the vectors used in the pairing are shown first, then followed by the initial mean MAC and MAC associated to each pair. The following lines show the evolution of these quantities when sensors are removed. Here sensor 1112z has a strong negative impact on the MAC of test mode 5.

The sensor labels are replaced by sensor numbers if the sensor configuration `sens` is not declared.

By default the `MACCO` command outputs a structure in which field `.data` contains in its first column the sensor or indice removed and the resulting MAC evolution of paired modes in the following columns. The field `.xlabel` contains the sensor labels or indices.

Command option `plot` will plot in the `ii_mac` figure the MAC evolutions as function of the sensors removed. Command option `text` will output the result as text.

This is an example of how to use of the `MACCO` command

```
% To see the result
[model,sens,ID,FEM]=demosdt('demopairmac');
ii_mac(ID,FEM,'sens',sens, ...
'inda',[1:8], ... % Select test modes to pair
'macco text')
% To get sensor indices
data=ii_mac(ID,FEM,'sens',sens, ...
'inda',[1:8], ... % Select test modes to pair
'macco');
i1=data.data(2:end,1) % indice of the sensors removed during the MACCO
```

MacM ...

When `cpa` and `cpb` are defined at finite element DOFs, it is much more appropriate to use a mass weighted form of the MAC defined as

$$\text{MAC-M}_{jk} = \frac{|\{\phi_{jA}\}^T [M] \{\phi_{kB}\}|^2}{|\{\phi_{jA}\}^T [M] \{\phi_{jA}\}| |\{\phi_{kB}\}^T [M] \{\phi_{kB}\}|} \quad (9.8)$$

called with `ii_mac(... 'm',m,'MacM Plot')`. If vectors are defined as sensors, the problem is to define what the mass should be. The standard approach is to use the static condensation of the full order model mass on the sensor set. When importing an external reduced mass matrix, just define the mass as shown above, when using *SdT*, see the `ii_mac mc` section below.

If `cpa` is defined at sensors and `cpb` at DOFs, `ii_mac` uses the sensor configuration `sens` to observe the motion of `cpb` at sensors. If `cpa` is defined at DOFs and `cpb` at sensors, `ii_mac` calls `fe_exp` to expand `cpb` on all DOFs.

The MAC-M can be seen as a scale insensitive version of the Pseudo-Orthogonality check (also called Cross Generalized Mass criterion) described below.

COMAC [,M][,A,B][,N][,S][,E] [,sort]

The `COMAC` command supports three correlation criteria (`N` nominal, `S` scaled and `E` enhanced) whose objective is to detect sensors that lead to poor correlation. You can compute all or some of these criteria using the `n`, `s`, or `e` options (with no option

the command computes all three). Sensors are given in the nominal order or sorted by decreasing COMAC value (`sort` command option).

These criteria assume the availability of paired sets of sensors. The `COMAC` commands thus start by using `MacPair` (`MacMPair` with the `M` command option) to pair vectors in `cpb` to vectors in `cpa`. The `B` command option can be used to force pairing against vectors in set B (rather than A which is the default value).

The **nominal** Coordinate Modal Assurance Criterion (COMAC) measures the correlation of two sets of similarly scaled modeshapes at the same sensors. The definition used for the *SDT* is

$$\text{COMAC}_l = 1 - \frac{\left\{ \sum_j^{NM} |c_l \phi_{jA} c_l \phi_{jB}| \right\}^2}{\sum_j^{NM} |c_l \phi_{jA}|^2 \sum_j^{NM} |c_l \phi_{jB}|^2} \quad (9.9)$$

which is 1 minus the definition found in [54] in order to have good correlation correspond to low COMAC values.

The assumption that modes a similarly scaled is sometimes difficult to ensure, so that the **scaled** COMAC is computed with shapes in set B scaled using the Modal Scale Factor (MSF)

$$\{ \widehat{c\phi_{jB}} \} = \{ c\phi_{jB} \} \text{MSF}_j = \{ c\phi_{jB} \} \frac{\{ c\phi_{jB} \}^T \{ c\phi_{jA} \}}{\{ c\phi_{jB} \}^T \{ c\phi_{jB} \}} \quad (9.10)$$

which sets the scaling of vectors in set B to minimize the quadratic norm of the difference between $\{ c\phi_{jA} \}$ and $\{ \widehat{c\phi_{jB}} \}$.

The **enhanced** COMAC (eCOMAC), introduced in [55], is given by

$$\text{eCOMAC}_l = \frac{\sum_j^{NM} \left\| \{ \widetilde{c_l \phi_{jA}} \} - \{ \widehat{c\phi_{jB}} \} \right\|}{2NM} \quad (9.11)$$

where the comparison is done using modeshapes that are vector normalized to 1

$$\{ \widetilde{c_l \phi_{jA}} \} = \{ c\phi_{jA} \} / \|c\phi_{jA}\|$$

This is an example of how to use of the `COMAC` command

```
[model,sens,ID,FEM]=demosdt('demopairmac');
```

```
figure(1);clf;
```

```
ii_mac(ID,FEM,'sens',sens,'comac plot')
ii_mac(1,'comac table');
```

POC [,Pair[A,B]] ...

The orthogonality conditions (6.85) lead to a number of standard vector correlation criteria. The **pseudo-orthogonality check** (POC) (also called **Cross Generalized Mass** (CGM)) and the less commonly used cross generalized stiffness (CGK) are computed using

$$\mu_{jk} = \{\phi_{jA}\}^T [M] \{\phi_{kB}\} \quad \kappa_{jk} = \{\phi_{jA}\}^T [K] \{\phi_{kB}\} \quad (9.12)$$

where for mass normalized test and analysis modes one expects to have $\mu_{jk} \approx \delta_{jk}$ and $\kappa_{jk} \approx \omega_j^2 \delta_{jk}$.

For matched modes, POC values differing significantly from 1 indicate either poor scaling or poor correlation. To distinguish between the two effects, you can use a MAC-M which corresponds to the square of a POC where each vector would be normalized first (see the **MacM** command).

Between unmatched modes, POC values should be close to zero. In some industries, off-diagonal cross POC values below 0.1 are required for the test verification of a model.

The **PairA**, **PairB**, **Plot**, **Table** options are available for **POC** just as for the **MAC**.

Rel [,scaled][,m]

For scaled matched modeshapes, the **relative error**

$$e_j = \frac{\|\{\phi_{jA}\} - \{\phi_{jB}\}\|}{\|\{\phi_{jA}\}\| + \|\{\phi_{jB}\}\|} \quad (9.13)$$

is one of the most accurate criteria. In particular, it is only zero if the modeshapes are exactly identical and values below 0.1 denote very good agreement.

The **rel** command calls **MacPair** to obtain shape pairs and plots the result of (9.13).

For uncalled matched modeshapes, you may want to seek for each vector in set B a scaling coefficient that will minimize the relative error norm. This coefficient is known as the **modal scale factor** and defined by

$$\text{MSF}_j = \frac{\{\phi_{jA}\}^T \{\phi_{jB}\}}{\{\phi_{jB}\}^T \{\phi_{jB}\}} \quad (9.14)$$

The `RelScale` command calls `MacPair` to obtain shape pairs, multiplies shapes in set B by the modal scale factor and plots the result of (9.13).

With the `M` option, the `MacPairM` is used to obtain shape pairs, kinetic energy norms are used in equations (9.13)-(9.14).

This is an example of how to use the `Rel` command

```
[model,sens,ID,FEM]=demosdt('demopairmac');
ii_mac(ID,FEM,'sens',sens,'rel');
```

VC

The following sections describe standard fields of `VC` vector correlation objects and how they can be set.

<code>VC.va</code>	vector set A detailed below
<code>VC.vb</code>	vector set B detailed below.
<code>VC.sens</code>	sensor description array describing the relation between the DOFs of <code>cpb</code> and the sensors on which <code>cpa</code> is defined.
<code>VC.m</code>	full order mass matrix
<code>VC.mc</code>	reduced mass matrix defined at sensors (see definition below)
<code>VC.qi</code>	sensor confidence weighting
<code>VC.k</code>	full order stiffness matrix
<code>VC.kd</code>	factored stiffness or mass shifted stiffness matrix
<code>VC.T</code>	reduced basis used for dynamic expansion

va,vb,sens

`ii_mac` uses two data sets referenced in `VC.va` and `VC.vb`. All standard input formats for shape definition are accepted

- `FEM result` with `.def` and `.DOF` fields, see section 7.8.
- `Shapes at DOFs` or pole residue with `.res` and `.po` fields (see section 5.6)
- `Response data` with `.w` and `.xf` fields (see section 5.8)

At a low level, if you have shapes as columns, you can define them with

```
FigHandle=figure(1);
ii_mac(FigHandle,'cpa',shapes_as_col,'labela','Test', ...
       'cpb',shape2, ... % Define vb
       'mac'); % define command
```

The `cpa` and `cpb` names stand for $[c]\{\phi_a\}$ and $[c]\{\phi_b\}$ since these vectors typically represent the observation of modeshapes at test sensors, see section 5.1).

It is usual to have `cpa` defined at sensors and `cpb` at DOFs, `ii_mac` uses the sensor configuration `sens` to observe the motion of `cpb` at sensors. If `cpa` is defined at DOFs and `cpb` at sensors, `ii_mac` calls `fe_exp` to expand `cpb` on all DOFs.

The generation of sensor configurations is detailed in section 4.3. You can provide an observation matrix obtained from an `fe_case('Sens')` command to allow observation of FEM results which are always assumed to be placed in the `.vb` field. One thus compares `VC.va.def` and `VC.sens.cta*VC.vb.def`.

```
[model,sens,ID,FEM]=demosdt('demopairmac-open')
figure(1);ii_mac(ID,FEM,'sens',sens, ...
                 'indb',7:20,'mac plot');
```

`m,k,kd`

For criteria that use vectors defined at DOFs, you may need to declare the mass and stiffness matrices. For large models, the factorization of the stiffness matrix is particularly time consuming. If you have already factored the matrix (when calling `fe_eig` for example), you should retain the result and declare it in the `kd` field.

The default value for this field is `kd=ofact(k,'de')` which is not appropriate for structures with rigid body modes. You should then use a mass-shift (`kd = ofact(k + alpha*m,'de')`), see section 6.2.4).

`mc`

The *SDT* supports an original method for reducing the mass on the sensor set. Since general test setups can be represented by an observation equation (4.1), the principle of reciprocity tells that $[c]^T$ corresponds to a set of loads at the location and in the direction of the considered sensors. To obtain a static reduction of the model on the sensors, one projects the mass (computes T^TMT) on the subspace

$$[T] = [\tilde{T}] [c\tilde{T}]^{-1} \quad \text{with} \quad [K] [\tilde{T}] = [c]^T \quad (9.15)$$

In cases where the model is fixed $[K]$ is non-singular and this definition is strictly equivalent to static/Guyan condensation of the mass [16]. When the structure is free, $[K]$ should be replaced by a mass shifted $[K]$ as discussed under the `kd` field above.

T

Reduced basis expansion methods were introduced in [16]. Static expansion can be obtained by using `T` defined by (9.15).

To work with dynamic or minimum residual expansion methods, `T` should combine static shapes, low frequency modes of the model, the associated modeshape sensitivities when performing model updating.

Modeshape expansion is used by `ii_mac` when `cpa` is full order and `cpb` is reduced. This capability is not currently finalized and will require user setting of options. Look at the HTML or PDF help for the latest information.

See also `ii_comac`, `fe_exp`, the `gartco` demonstration, section 3.2


ii_mmif

Purpose Mode indicator functions and signal processing.

Syntax

```
OUT = ii_mmif('command',IN,'waitbar')
ci=iipplot; ii_mmif('command',ci,'CurveName')
```

Description This function supports all standard transformations of response datasets in particular mode indicator functions and signal processing.

With data stored in a `iipplot` figure, from the GUI, open the Stack tab of the property figure (accessible through `iicom('CurtabStack')` or by clicking on ) then select **Compute ...** in the context menu to transform a given dataset. This has the advantage of allowing interactive changes to signal processing results, see section 2.1.7.

From the command line, use `ii_mmif('command',ci,Curve)` (where `ci` is a handle referring to `iipplot` figure). `Curve` can be a string defining a curve name or a regular expression (beginning by `#`) defining a set of curves. One can also give some curve names as strings in a cell array. Without output argument, computed `mmif` is stored in the stack with name `mmif(CurveName)`. Use command option `-reset` to compute a `mmif` which has already been computed before (otherwise old result is returned).

```
ci=iicom('curveload','gartid'); % load curve gartid example
ii_mmif('mmif',ci,'Test');      % compute mmif of set named Test
iicom('iixonly',{'mmif(Test)'});% display result
```

When used with `idcom`, the **Show ...** context menu supports the automated computation of a number of transformations of `ci.Stack{'Test'}`. These mode indicator functions combine data from several input/output pairs of a MIMO transfer function in a single response that gives the user a visual indication of pole locations. You can then use the `idcom e` command to get a pole estimate.

With data structures not in `iipplot` use `mmif=ii_mmif(command,Curve)`. Use command option `-struct` to obtain output as curve data structure.

```
ci=iicom('curveload','gartid'); % load curve gartid example
R1=ci.Stack{'Test'};           % get Test dataset in variable R1
R2=ii_mmif('mmif-struct',R1);  % compute mmif
```

MMIF

The Multivariate Mode Indicator Function (MMIF) (can also be called using `icom Show mmi`) was introduced in [56]. Its introduction is motivated by the fact that, for a single mode mechanical model, the phase at resonance is close to -90° . For a set of transfer functions such that $\{y(s)\} = [H(s)] \{u(s)\}$, one thus considers the ratio of real part of the response to total response

$$q(s, \{u\}) = \frac{\{u\}^T [\operatorname{Re}(H)^T \operatorname{Re}(H)] \{u\}}{\{u\}^T \operatorname{Re}([H^H H]) \{u\}} = \frac{\{u\}^T [B] \{u\}}{\{u\}^T [A] \{u\}} \quad (9.16)$$

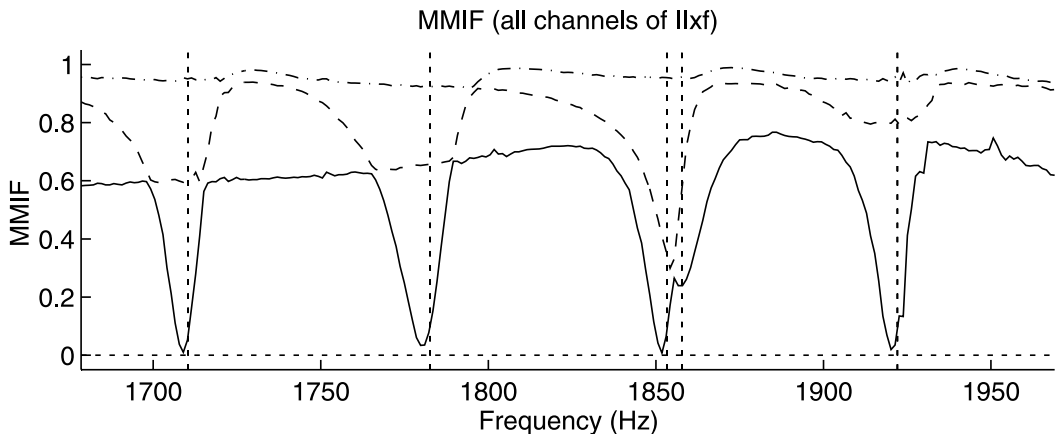
For structures that are mostly elastic (with low damping), resonances are sharp and have properties similar to those of isolated modes. The MMIF (q) thus drops to zero.

Note that the real part is considered for force to displacement or acceleration, while for force to velocity the numerator is replaced by the norm of the imaginary part in order to maintain the property that resonances are associated to minima of the MMIF. A MMIF showing maxima indicates improper setting of `idopt.DataType`.

For system with more than one input (u is a vector rather than a scalar), one uses the extreme of q for all possible real valued u which are given by the solutions of the eigenvalue problem $[A] \{u\} q + [B] \{u\} = 0$.

The figure below shows a particular set for MMIF. The system has 3 inputs, so that there are 3 indicator functions. The resonances are clearly indicated by minima that are close to zero.

The second indicator function is particularly interesting to verify pole multiplicity. It presents a minimum when the system presents two closely spaced modes that are excited differently by the two inputs (this is the case near 1850 Hz in the figure). In this particular case, the two poles are sufficiently close to allow identification with a single pole with a modeshape multiplicity of 2 (see `id_rm`) or two close modes. More details about this example are given in [8].



This particular structure is not simply elastic (the FRFs combine elastic properties and sensor/actuator dynamics linked to piezoelectric patches used for the measurement). This is clearly visible by the fact that the first MIF does not go up to 1 between resonances (which does not happen for elastic structures).

At minima, the forces associated to the MMIF (eigenvector of $[A] \{u\} q + [B] \{u\} = 0$) tend to excite a single mode and are thus good candidates for force appropriation of this mode [57]. These forces are the second optional output argument `ua`.

CMIF

The Complex Mode Indicator Function (CMIF) (can also be called using `iicom Show cmmi`, see [58] for a thorough discussion of CMIF uses), uses the fact that resonances of lightly damped systems mostly depend on a single pole. By computing, at each frequency point, the singular value decomposition of the response

$$[H(s)]_{NS \times NA} = [U]_{NS \times NS} [\Sigma]_{NS \times NA} [V^H]_{NA \times NA} \quad (9.17)$$

one can pick the resonances of Σ and use U_1, V_1 as estimates of modal observability / controllability (modeshape / participation factor). The optional `u`, `v` outputs store the left/right singular vectors associated to each frequency point.

AMIF

`ii_mmif` provides an alternate mode indicator function defined by

$$q(s) = 1 - \frac{|\operatorname{Im}(H(s))||H(s)|^T}{|H(s)||H(s)|^T} \quad (9.18)$$

which has been historically used in force appropriation studies [57]. Its properties are similar to those of the MMIF except for the fact that it is not formulated for multiple inputs.

This criterion is supported by `iipplot` (use `iicom Show amif`).

SUM, SUMI, SUMA

Those functions are based upon the sum of data from amplitude of sensors for a given input. One can specify dimensions affected by the sum using command option `-dim i` (`i` is one or more integers).

`SUM`,

$$S(s, k) = \sum_j \|H_{j,k}(s)\|^2$$

is the sum of the square of all sensor amplitude for each input.

`SUMI`,

$$S(s, k) = \sum_j \operatorname{Im}(H_{j,k}(s))^2$$

is the sum of the square of the imaginary part of all sensors for each input.

`SUMA`,

$$S(s, k) = \sum_j \|H_{j,k}(s)\|$$

is the sum of the amplitude of all sensors for each input.

Those functions are sometimes used as mode indicator functions and are thus supported by `ii_mmif` (you can also call them using `iicom Show sumi` for example).

NODEMIF

Undocumented.

Signal processing

Following commands are related to signal processing. Section section 2.1.7 illustrates the use of those functions through `iiplot`.

Integrate, DoubleInt, Vel, Acc

- `Integrate` integrates the frequency dependent signal

$$I_{j,k}(s) = \frac{H_{j,k}(0)}{s^2} + \frac{H_{j,k}(s)}{s}.$$

- `DoubleInt` integrates twice the frequency dependent signal

$$I2_{j,k}(s) = \frac{H_{j,k}(0)}{s^3} + \frac{H_{j,k}(s)}{s^2}.$$

- `Vel` computes the velocity (first derivative) of the signal. For a frequency dependent signal

$$V_{j,k}(s) = s \cdot H_{j,k}(s).$$

For a time dependent signal, finite differences are used

$$V_{j,k}(t_n) = \frac{H_{j,k}(t_{n+1}) - H_{j,k}(t_n)}{t_{n+1} - t_n}.$$

$V_{j,k}(t_{end})$ is linearly interpolated in order to obtain a signal of the same length.

- `Acc` computes the acceleration (second derivative) of the signal. For a frequency dependent signal

$$A_{j,k}(s) = s^2 \cdot H_{j,k}(s).$$

For a time dependent signal, finite differences are used

$$A_{j,k}(t_n) = \frac{h_n \cdot (H_{j,k}(t_{n+1}) - H_{j,k}(t_n)) - h_{n+1} \cdot (H_{j,k}(t_n) - H_{j,k}(t_{n-1}))}{h_{n+\frac{1}{2}} \cdot h_n \cdot h_{n+1}},$$

with $h_{n+1} = t_{n+1} - t_n$ and $h_{n+\frac{1}{2}} = \frac{h_n + h_{n+1}}{2}$.

$A_{j,k}(t_{end})$ and $A_{j,k}(t_1)$ are linearly interpolated in order to obtain a signal of the same length.

FFT, FFTShock, IFFT, IFFTShock

Computes the Discrete Fourier Transform of a time signal. `FFT` normalizes according to the sampling period whereas `FFTShock` normalizes according to the length of the signal (so that it is useful for shock signal analysis).

`IFFT` and `IFFTShock` are respectively the inverse transform.

Accepted command options are

- `-nostat` to remove static component ($f=0$) from fft response.
- `-newmark` to shift frequencies of computed time integration with a mean acceleration Newmark scheme ($\gamma = 0.5, \beta = 0.25$) in order to correct the periodicity error $\frac{\Delta T}{T} = \frac{\omega^2 h^2}{12}$. This correction is especially true for low frequencies.
- `tmin value`, `tmax value`, `fmin value`, `fmax value` to use parts of the time trace or spectrum.
- `zp value` is used to apply a factor `value` on the length of the signal and zero-pad it.
- `-window name` is used to apply a window on the time signal. Use `fe_curve('window')` to get a list of implemented windows. For windows with parameters, use double quotes. For example
`R1_FFT=ii_mmif('FFTShock -struct -window "Exponential 10 20 100"',R1)`
- `-display` force display in `iiplot` after computing

```
[model,def]=fe_time('demobar10-run');
R1=ii_mmif('FFT-struct -window "hanning" wmax 400',def);
% To allow interaction
ci=iiplot;ci.Stack{'curve','def'}=def;
ii_mmif('FFT-struct -window "hanning" fmax 400 -display',ci,'def');
iiicom('CurtabStack') % Show the property figure
```

BandPass

`ii_mmif('BandPass fmin fmin fmax fmax')` Performs a true band pass filtering (i.e. using `fft`, truncating frequencies and go back to time domain with `ifft`) between `fmin` and `fmax` frequencies.

OctGen, Octave

`filt=ii_mmif('OctGen nth',f)` computes filters to perform a $\frac{1}{nth}$ -octave analysis.

As many filters as frequencies at the $\frac{1}{nth}$ -octave of 1000 Hz in the range of **f** (vector of frequencies) are computed. Each band pass filter is associated to a frequency f_0 and a bandwidth Bw depending on f_0 . Filters are computed so that their sum is almost equal to 1. Filter computed are, for each f_0 :

$$H(f, f_0) = \frac{1}{1 + \left(\frac{1}{Bw(f_0)} \cdot \frac{f^2 - f_0^2}{f}\right)^6}$$

With command option `plot`, filters are plotted.

`ii_mmif('Octave nth',ci)` performs the $\frac{1}{nth}$ octave analysis of active curve displayed in `iiplot` figure.

The $\frac{1}{nth}$ octave analysis consists in applying each filter on the dataset. Energy in each filtered signal is computed with $10\log(S)$ (where S is the trapezium sum of the filtered signal, or of the square of the filtered signal if it contains complex or negative values) and associated to the center frequency of corresponding filter.

See also [iiplot](#), [iicom](#), [idopt](#), [fe_sens](#)

ii_plp

Purpose Pole line plots and other plot enhancement utilities.

Syntax `ii_plp(po)`
`ii_plp(po,color,Opt)`

Description `plp`

Generation of zoomable vertical lines with clickable information.

`ii_plp(po)` will plot vertical dotted lines indicating the pole frequencies of complex poles in `po` and dashed lines at the frequencies of real poles. The poles `po` can be specified in any of the 3 accepted formats (see `ii_pof`).

When you click on these lines, a text object indicating the properties of the current pole is created. You can delete this object by clicking on it. When the lines are part of `iipplot` axes, clicking on a pole line changes the current pole and updates any axis that is associated to a pole number (local Nyquist, residue and error plots, see `iipplot`).

`.ID PoleLine Call from iipplot`

When displaying a curve in `iipplot`, one can generate automatic calls to `ii_plp`. `Curve.ID` field can be used to generate automatically vertical lines in `iipplot`. It is a cell array with as many cell as line sets. Each cell is a data structure defining the line set. Following fields can be defined:

- `.po` can be a column vector defining abscissa of vertical lines. It can also be a string, possibly depending on the displayed curve `XF1` and the channel through variable `ch` to be evaluated to define the `ro.po` vector, for example `'r1.po=XF1.Y2(:,ch);'`.
- `.LineProp` is optional. One can specify some MATLAB line properties in this field as a cell array `{'prop1', value1, 'prop2', value2, ...}`, for example `{'LineStyle',':', 'color','r'}`.
- `.name` is used to generate a text info displayed when the user clicks on the line.

- `.unit`(obsolete) is used for Hz vs. rad/s unit conversion. With `tens` set to `1` (`11` or `12`) is used for poles in Hz, while those with `tens` set to `2` correspond to Rad/s. This value is typically obtained from `IDopt(3)`.
- `.format` an integer that specifies whether the imaginary part $Im(\lambda)$ (`Format=2` which is the default) or the amplitude $|\lambda|$ (using `Format=3` corresponding to format 3 of `ii_pof`) should be used as the “frequency” value for complex poles.

Legend

Dynamic multi-line legend generation used by `iiplot` and `feplot`

`ii_plp('legend',ga,prop)` with properties a cell array detailed with in `comgui def.Legend` (typical legend generation associated with FEM solutions).

`ii_plp('legend -corner .01 .01 -reset ',ga,ua,StringCell,legProp)` is an older format found in some calls.

- `cornerx y` gives the position of the legend corner with respect to the current axis.
- `-reset` option deletes any legend existing in the current axis.
- `ga` handle to the axis where the axes is to be placed, see `gca`.
- `ua` if not empty provides additional properties.
- `StringCell` cell array of strings with one per line of legend.
- `legProp` non default text properties to be used for the legend.

PlotSubMark

Generate subsampled markers.

spy

Generates a `spy` plot with color coding associated with the non-zero element values.

- `unsymm` is used to force non symmetric plots.
- `threshold` is used force small terms to zero.

TickFcn

SDT implements a general mechanism for enhancing the basic dynamic label generation of MATLAB. This allows placement of strings at proper locations on an axis. This functionality is not fully documented and you are encouraged to look-up the source code. SDT generated plots expect the following fields in the axis userdata `ua.TickInfo` for data and `ua.TickFcn` for the callback. A sample usage would be

```
C1=struct('X',{num2cell(2:4)' 2},'Xlab',{'x','y'}, ...
        'Y',(1:3))
figure(1);plot(1:3,C1.Y);ii_plp('tickXCell',C1,gca);
C1=ii_plp('tickXCell',C1); %Defines the PlotInfo
```

ColorMap

FEM oriented color maps.

See also `ii_pof`, `idopt`, `iiplot`, `iicom`

ii_poest

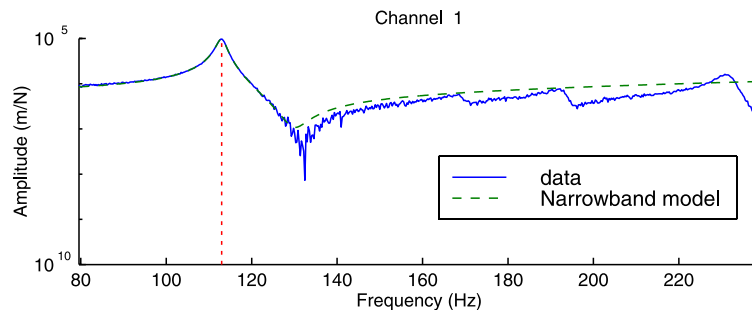
Purpose Identification of a narrow-band single pole model.

Syntax `idcom('e')`
`[res,po]= ii_poest(ci.Stack{'Test'},opt)`

Description `ii_poest` (`idcom e` command and associated button in the `idcom` GUI figure, see section 2.4) provides local curve fitting capabilities to find initial estimates of poles by simply giving an indication of their frequency.

The central frequency for the local fit is given as `opt(2)` or, if `opt(2)==0`, by clicking on a plot whose abscissas are frequencies (typically FRF of MMIF plots generated by `iipplot`).

The width of the selected frequency band can be given in number of points (`opt(1)` larger than 1) or as a fraction of the central frequency (points selected are in the interval `opt(2)*(1+[-opt(1) opt(1)])` for `opt(1)<1`). The default value is `opt(1)=0.01`.



A single pole fit of the FRFs in `xf` is determined using a polynomial fit followed by an optimization using a special version of the `id_rc` algorithm. The accuracy of the results can be judged graphically (when using the `idcom e` command, `Test` and `IdFrfr` are automatically overlaid as shown in the plot above) and based on the message passed

```
>> ci=idcom;iicom(ci,'CurveLoad','gartid');  
>> idcom('e .01 16.5');  
>> disp(ci.Stack{'IdAlt'}.po)  
1.6427e+001 1.3108e-002
```

```

LinLS: 5.337e-001, LogLS 5.480e-001, nw 18
  mean(reIE) 0.00, scatter 0.47 : acceptable
Found pole at 1.6427e+001   1.3108e-002
% manual call would be [res,po]=ii_poest(ci.Stack{'Test'},[.01 16.5]);

```

which indicates the linear and quadratic costs (see `ii_cost`) in the narrow frequency band used to find the pole, the number of points in the band, the mean relative error (norm of difference between test and model over norm of response, see `iiplot error`) which should be below 0.1, and the level of scatter (norm of real part over norm of residues, which should be small if the structure is close to having proportional damping).

If you have a good fit and the pole differs from poles already in your current model, you can add the estimated pole (add `IdAlt` to `IdMain`) using the `idcom ea` command.

The choice of the bandwidth can have a significant influence on the quality of the identification. As a rule the bandwidth of your narrow-band identification should be larger than the pole damping ratio (`opt(1)=0.01` for a damping of 1% is usually efficient). If, given the frequency resolution and the damping of the considered pole, the default does not correspond to a frequency band close to $2\zeta_j\omega_j$, you should change the selected bandwidth (for example impose the use of a larger band with `opt(1)=.02` which you can obtain simply using `idcom ('e.02')`).

This routine should be used to obtain an initial estimate of the pole set, but the quality of its results should not lead you to skip the pole tuning phase (`idcom eup` or `eopt` commands) which is essential particularly if you have closely spaced modes.

See also [idcom](#), [id_rc](#), [iiplot](#)

ii_pof

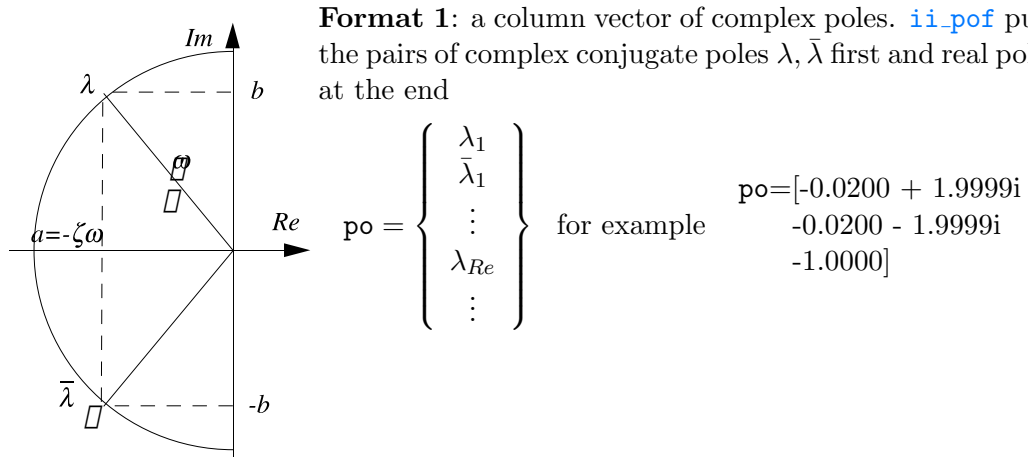
Purpose Transformations between the three accepted pole formats.

Syntax `[pob] = ii_pof(poa,DesiredFormatNumber)`
`[pob] = ii_pof(poa,DesiredFormatNumber,SortFlag)`

Description The *Structural Dynamics Toolbox* deals with real models so that poles are either real or come in conjugate pairs

$$\{\lambda, \bar{\lambda}\} = \{a \pm ib\} = \{-\zeta\omega \pm \omega\sqrt{1 - \zeta^2}\}$$

Poles can be stored in three accepted formats which are automatically recognized by `ii_pof`(see warnings below for exceptions).



Because non-real poles come in conjugate pairs with conjugate eigenvectors, it is generally easier to only view the positive-imaginary and real poles, as done in the two other formats.

Format 2: real and imaginary part

$$\text{po} = \begin{bmatrix} a & b \\ \vdots & \vdots \end{bmatrix} \quad \text{for example} \quad \text{po} = [-0.0200 & 1.9999 \\ -1.0000 & 0.0000]$$

Format 3: frequency ω and damping ratio ζ

$$\mathbf{po} = \begin{bmatrix} \omega_1 & \zeta_1 \\ \vdots & \vdots \end{bmatrix} \quad \text{for example} \quad \mathbf{po} = \begin{bmatrix} 2.0000 & 0.0100 \\ -1.0000 & 1.0000 \end{bmatrix}$$

To **sort** the poles while changing format use an arbitrary third argument [SortFlag](#).

Warnings

The input format is recognized automatically. An **error** is however found for poles in input format 2 (real and imaginary) with all imaginary below 1 and all real parts positive (unstable poles). In this rare case you should change your frequency unit so that some of the imaginary parts are above 1.

Real poles are always put at the end. If you create your own residue matrices, make sure that there is no mismatch between the pole and residue order (the format for storing residues is described in section 5.6).

See also

[idcom](#), [id_rc](#), [ii_plp](#)

nasread

Purpose Read results from outputs of the MSC/NASTRAN finite element code. This function is part of FEMLink.

Syntax `out = nasread('FileName','Command')`

Description `nasread` reads bulk data deck (NASTRAN input), direct reading of model and result information in OUTPUT2 and OUTPUT4 files generated using NASTRAN `PARAM,POST,-i` cards. This is the most efficient and accurate method to import NASTRAN results for post-processing (visualization with `feplot`, normal model handling with `nor2ss`, ...) or parameterized model handling with `upcom`. Results in the `.f06` text file (no longer supported).

Available commands are

Bulk file

`model=nasread('FileName','bulk')` reads NASTRAN bulk files for nodes (grid points), element description matrix, material and element properties, and coordinate transformations, MPC, SPC, DMIG, SETS, ...

Use `'BulkNo'` for a file with no `BEGIN BULK` card. Unsupported cards are displayed to let you know what was not read. You can omit the `'bulk'` command when the file name has the `.dat` or `.bdf` extension.

Each row of the `bas.bas` output argument contains the description of a coordinate system.

The following table gives a *partial conversion list*. For an up to date table use `nas2up('convlist')`

NASTRAN	<i>SDT</i>
CELAS1, CELAS2, RBAR	<i>celas</i>
RBE2	<i>rigid</i>
RBE3	<i>rbe3</i> in <i>Case</i>
CONROD	<i>bar1</i>
CBAR, CBEAM, CROD	<i>beam1</i>
CBUSH	<i>cbush</i>
CSHEAR	<i>quad4</i>
CONM1, CONM2	<i>mass2</i>
CHEXA	<i>hexa8, hexa20</i>
CPENTA	<i>penta6, penta15</i>
CTETRA	<i>tetra4, tetra10</i>
CTRIA3, CTRIAR	<i>tria3</i>
CTRIA6	<i>tria6</i>
CQUAD4, CQUADR	<i>quad4</i>
CQUAD8	<i>quadb</i>

Details on properties are given under [naswrite WritePLIL](#). NASTRAN Scalar points are treated as standard SDT nodes with the scalar DOF being set to DOF *.01* (this has been tested for nodes, DMIG and MPC).

OUTPUT2 binary

`model=nasread('FileName','output2')` reads *output2 binary output format* for tables, matrices and labels. You can omit the *output2* command if the file names end with *2*. The output *model* is a model data structure described in section 7.6. If deformations are present in the binary file, they are saved *OUTG(i)* entries in the stack (see section 7.8). With no output argument, the result is shown in *feplot*.

Warning: do not use the `FORM = FORMATTED` in the eventual `ASSIGN OUTPUT2` statement.

The optional *out* argument is a cell array with fields the following fields

<code>.name</code>	Header data block name (table, matrix) or label (label)
<code>.dname</code>	Data block name (table, matrix) or NASTRAN header (label)
<code>.data</code>	cell array with logical records (tables), matrix (matrix), empty (label)
<code>.trl</code>	Trailer (7 integers) followed by record 3 data if any (for table and matrix), date (for label)

Translation is provided for the following tables

<code>GEOM1</code>	nodes with support for local coordinates and output of nodes in global coordinates
<code>GEOM2</code>	elements with translation to SDT model description matrix (see <code>bulk</code> command).
<code>GEOM4</code>	translates constraints (<code>MPC</code> , <code>OMIT</code> , <code>SPC</code>) and rigid links (<code>RBAR</code> , <code>RBE1</code> , <code>RBE2</code> , <code>RBE3</code> , <code>RROD</code> , ...) to SDT model description matrix
<code>GPDT</code>	with use of <code>GPL</code> and <code>CSTM</code> to obtain nodes in global coordinates
<code>KDICT</code>	reading of element mass (<code>MDICT</code> , <code>MELM</code>) and stiffness (<code>KDICT</code> , <code>KELM</code>) matrix dictionaries and transformation of a type 3 superelement handled by <code>upcom</code> . This is typically obtained from NASTRAN with <code>PARAM,POST,-4</code> . To choose the file name use <code>Up.file='FileName';Up=nasread(Up,'Output2.op2');</code>
<code>MPT</code>	material property tables
<code>OUG</code>	transformation of shapes (modes, time response, static response, ...) as <code>curve</code> entries in the stack (possibly multiple if various outputs are requested). Note : by default deformations are in the SDT global coordinate system (basic in NASTRAN terminology). You may switch to output in the local (global in NASTRAN terminology) using <code>PARAM,OUGCORDD,GLOBAL</code> . To avoid <i>Out of Memory</i> errors when reading deformations, you can set use a smaller buffer <code>sdtdef('OutOfCoreBufferSize',10)</code> (in MB). When too large, <code>def.def</code> is left in the file and read as a <code>v_handle</code> object that lets you access deformations with standard indexing commands. Use <code>def.def=def.def(:,:)</code> to load all. To get the deformation in the stack use calls of the form <code>def=stack_get(model,'curve','OUG(1)','getdata')</code>
<code>OEE</code>	tables of element energy
<code>OES</code>	tables of element stresses or strains.

This translation allows direct reading/translation of output generated with NASTRAN `PARAM,POST` commands simply using `out=nasread('FileName.op2')`. For model and modeshapes, use `PARAM,POST,-1`. For model and element matrices use

PARAM,POST,-4 or PARAM,POST,-5 (see BuildUp command below).

BuildUp,BuildOrLoad

A standard use of FEMLink is to import a model including element matrices to be used later with `upcom`. You must first run NASTRAN SOL103 with `PARAM,POST,-4` to generate the appropriate `.op2` file (note that you must include the geometry in the file, that is not use `PARAM,OGEOM,NO`). Assuming that you have saved the bulk file and the `.op2` result in the same directory with the same name (different extension), then

```
Up=nasread('FileName.blk','buildup')
```

reads the bulk and `.op2` file to generate a superelement saved in `FileName.mat`.

It is necessary to read the bulk because linear constraints are not saved in the `.op2` file during the NASTRAN run. If you have no such constraints, you can read the `.op2` only with `Up=upcom('load FileName');Up=nasread(Up,'FileName.op2')`.

The `BuildOrLoad` command is used to generate the `upcom` file on the first run and simply load it if it already exists.

```
nasread('FileName.blk','BuildOrLoad') % result in global variable Up
```

OUTPUT4 binary

`out=nasread('FileName','output4')` reads `output4` *binary output format* for matrices. The result `out` is a cell array containing matrix names and values stored as MATLAB sparse matrices.

All double precision matrix types are now supported. If you encounter any problem, ask for a patch which will be provided promptly.

OUTPUT4 text

```
out=nasread('FileName','output4')
```

`Output4` text files are also supported with less performance and no support for non sequential access to data with the SDT `v_handle` object.

Supported options

`-full` : assumes that the matrix to be read should be stored as full (default sparse).

`-transpose` : transpose data while reading.

`-hdf` : save data in a hdf file. Reading is performed using buffer (`setpref('SDT','OutOfCoreBu`
for a 100MB buffer). It is useful to overcome the 2GB limit on 32 bit Matlab:
see `sdt hdf` for details about how to build `v_handle` on hdf file.

`.f06` output (obsolete)

ASCII reading in `.f06` files is slow and often generates round-off errors. You should thus consider reading binary OUTPUT2 and OUTPUT4 files, which is now the only supported format. You may try reading matrices with `nasread('FileName','matprt')`, tables with `nasread('F','tabpt')` and real modes with

```
[vector,mdof]=nasread('filename','vectortype')
```

Supported vectors are displacement (`displacement`), applied load vector (`oload`) and grid point stress (`gpstress`).

See also

`naswrite`, `FEMLink`

naswrite

Purpose	Formatted ASCII output to MSC/NASTRAN bulk data deck. This function is part of FEMLink.
Syntax	<pre>naswrite('FileName',node,elt,pl,il) naswrite('FileName','command', ...) naswrite('-newFileName','command', ...) naswrite(fid,'command', ...)</pre>
Description	<code>naswrite</code> appends its output to the file <code>FileName</code> or creates it, if it does not exist. Use option <code>-newFileName</code> to force deletion of an existing file. You can also provide a handle <code>fid</code> to a file that you opened with <code>fopen</code> . <code>fid=1</code> can be used to have a screen output.

EditBulk

Supports bulk file editing. Calls take the form

`nas2up('EditBulk',InFile,edits,Outfile)`, where `InFile` and `OutFile` are file names and `edits` is a cell array with four columns giving `command`, `BeginTag`, `EndTag`, and `data`. Accepted commands are

<code>Before</code>	inserts data before the <code>BeginTag</code> .
<code>Insert</code>	inserts data after the <code>EndTag</code> .
<code>Remove</code>	removes a given card. Warning this does not yet handle multiple line cards.
<code>Set</code>	used to set parameter and assign values. For example

```
edits={'Set','PARAM','POST','-2'};
rootname='my_job';
f0={'OUTPUT4',sprintf('%s_mkekvr.op4',rootname),'NEW',40,'
      'OUTPUT4',sprintf('%s_TR.op4',rootname),'NEW',41,'DELE
edits(end+1,1:4)={'set','ASSIGN','','f0}
```

When writing automated solutions, the edits should be stored in a stack entry `info,EditBulk`.

model

`naswrite('FileName',model)` the nominal call, it writes everything possible : nodes, elements, material properties, case information (boundary conditions, loads, etc.). For example `naswrite(1,femesh('testquad4'))`.

The following information present in model stack is supported

- curves as `TABLED1` cards if some curves are declared in the `model.Stack` see `fe_curve` for the format).
- Fixed DOFs as `SPC1` cards if the model case contains `FixDof` and/or `KeepDof` entries. `FixDof,AutoSPC` is ignored if it exists.
- Multiple point constraints as `MPC` cards if the model case contains `MPC` entries.
- coordinate systems as `CORDi` cards if `model.bas` is defined (see `basis` for the format).

The obsolete call `naswrite('FileName',node,elt,pl,il)` is still supported.

node,elt

You can also write nodes and elements using the low level calls but this will not allow fixes in material/element property numbers or writing of case information.

```
femesh('reset');
femesh('testquad4')
fid=1 % fid=fopen('FileName');
naswrite(fid,'node',FENode)
naswrite(fid,'node',FENode)
%fclose(fid)
```

Note that `node(:,4)` which is a group identifier in SDT, is written as the SEID in NASTRAN. This may cause problems when writing models from translated from other FEM codes. Just use `model.Node(:,4)=0` in such cases.

dmig

DMIG writing is supported through calls of the form `naswrite(fid,'dmigwrite NAME',mat,mdof)`. For example

```
naswrite(1,'dmigwrite KAAT',rand(3),[1:3]'+.01)
```

A `nastran,dmig` entry in `model.Stack`, where the data is a cell array where each row gives name, DOF and matrix, will also be written. You can then add these matrices to your model by adding cards of the form `K2GG=KAAT` to your NASTRAN case.

job

NASTRAN job handling on a remote server from the MATLAB command line is partially supported. You are expected to have `ssh` and `scp` installed on your computer. On windows, it is assumed that you have access to these commands using CYGWIN. You first need to define your preferences

```
setpref('FEMLink','CopyFcn','scp');
setpref('FEMLink','RunNastran','nastran');
setpref('FEMLink','RemoteShell','ssh');
setpref('FEMLink','RemoteDir','/tmp2/nastran');
setpref('FEMLink','RemoteUserHost','user@myhost.com');
setpref('FEMLink','DmapDir',fullfile(fileparts(which('nasread'))),'dmap');
```

You can define a job handler customized to your needs and still use the `nas2up` calls for portability by defining `setpref('FEMLink','NASTRANJobHandler','FunctionName')`

You can then run a job using `nas2up('joball','BulkFileName.dat')`. Additional arguments can be passed to the `RunNastran` command by simply adding them to the `joball` command. For example

```
nas2up('joball','BulkFileName.dat',struct('RunOptions','memory=1GB')).
```

It is possible provide specific options to your job handler by storing them as a `info,NasJobOpt` entry in your `model.Stack`. `nas2up('JobOptReset')` resets the default. The calling format in various functions that use the job handling facility is then

```
model=stack_set('info','NasJobOpt',nas2up('jobopt'));
nas2up('joball','step12.dat',model);
```

`RunOpt.RunOptions` stores text options to be added to the `nastran` command. `RunOpt.BackWd` can be used to specify an additional relative directory for the `JobCpFrom` command. `RunOpt.RemoteRelDir` can be used to specify the associated input for the `JobCpTo` command.

`nas2up('JobCpTo','LocalFileName','RemoteRelDir')` puts (copies) files to the remote directory or to `fullfile(RemoteDir,RemoteRelativeDir)` if specified.

`nas2up('JobCpFrom','RemoteFileName')` fetches files. The full remote file name is given by `fullfile(RemoteDir,RemoteFileName)`. Any relative directory is ignored for the local directory.

Here is a simple script that generates a model, runs NASTRAN and reads the result

```
wd=sdtdef('tempdir');
```

```
model=demosdt('demoubeam-2mat'); cf=feplot;
model=fe_case(model,'dofload','Input', ...
    struct('DOF',[349.01;360.01;241.01;365.03],'def',[1;-1;1;1],'ID',100));
model=nas2up('JobOpt',model);
model=stack_set(model,'info','Freq',[20:2:150]);

% write bulk but do not include eigenvalue options
naswrite(['-new' fullfile(wd,'ubeam.bdf')],stack_rm(model,'info','EigOpt'))

% generate a job by editing the reference mode.dat file
fname='ubeam.dat';
edits={'Set','PARAM','POST','-2';
    'replace','include ''model.bdf''','','include ''ubeam.bdf''';
    'replace','EIGRL','',nas2up('writecard',-1,[1 0 0 30],'ijji','EIGRL')};
nas2up('editbulk','mode.dat',edits,fullfile(wd,fname));
cd(wd);type(fname)
nas2up('joball',fname,model)
cg=feplot(4);mo1=nasread('ubeam.op2');
```

Wop4

Matrix writing to `OUTPUT4` format. You provide a cell array with one matrix per row, names in first column and matrix in second column. The optional byte swapping argument can be used to write matrices for use on a computer with another binary format.

```
kv=speye(20);
ByteSwap=0; % No Byte Swapping needed
nas2up('wop4','File.op4',{'kv',kv},ByteSwap);
```

For `ByteSwap` you can also specify `ieee-le` for little endian (Intel PC) or `ieee-be` depending on the architecture NASTRAN will be running on. You can omit specifying `ByteSwap` at every run by setting

```
setpref('FEMLink','OutputBinaryType','ieee-le')
```

WriteFreqLoad

`edits=naswrite('Target.bdf','WriteFreqLoad',model)` (or the equivalent `nas2up` call when the file is already open as show below) writes loads defined in `model` (and generated with `Load=fe_load(model)`) as a series of cards. `FREQ1` for load frequencies, `TABLED1` for the associated curve, `RLOAD1` to define the loaded DOFs and `DAREA` for the spatial information about the load. The return `edits` argument is the entry that can be used to insert the associated subcase information in a nominal bulk.

The identifiers for the loads are supposed to be defined as `Case.Stack{j1,end}.ID` fields.

```
% Generate a model with sets of point loads
model=demosdt('Demo ubeam dofload noplot')
% Define the desired frequencies for output
model=stack_set(model,'info','Freq', ...
    struct('ID',101,'data',linspace(0,10,12)));
fid=1 % fid=fopen('FileName');
edits=nas2up('writefreqload',fid,model);
fprintf('%s\n',edits{end}{:}); % Main bulk to be modified with EditBulk
fclose(fid)
```

Write[Curve,Set,SetC,Uset]

Write commands are used to `WriteCurve` lets you easily generate NASTRAN curve tables.

`WriteSet` lets you easily generate NASTRAN node and elements sets associated with node and element selection commands.

`WriteSetC` formats the sets for use in the case control section rather than the bulk.

`WriteUset` generates DOFs sets.

```
model=demosdt('demogartfe');
fid=1; % display on screen (otherwise use FOPEN to open file)
nas2up('WriteSet',fid,3000,model,'findnode x>.8');
selections={'zone_1','group 1';'zone_2','group 2:3'};
nas2up('WriteSet',fid,2000,model,selections);
st=nas2up('WriteSet',-1,2000,model,selections);

curves={'curve','Sine',fe_curve('testsin -id1',linspace(0,pi,10)) ; ...
    'curve','Exp.',fe_curve('testexp -id100',linspace(0,1,30))};
nas2up('WriteCurve',fid,curves)
DOF=feutil('getdof',model);
nas2up('WriteUset U4',fid,DOF(1:20))
```

WritePLIL

The `WritePLIL` is used to resolve identifier issues in `MatId` and `ProId` (elements in SDT have both a `MatId` and an `ProID` while in NASTRAN they only have a `ProId` with the element property information pointing to material entries). While this command is typically used indirectly while writing a full model, you may want to access it directly. For example

```
model=demosdt('demogartfe');  
nas2up('Writeplil',1,model);
```

- `p_solid` properties are implemented somewhat differently in NASTRAN and SDT, thus for a `il` row giving `[ProID type Coordm In Stress Isop Fctn]`

In NASTRAN `In` is either a string or an integer. If it is an integer, this property is the same in `il`. If it is a string equal to resp. `TWO` or `THREE`, this property is equal to resp. 2 or 3 in `il`.

In NASTRAN `Stress` is either a string or an integer. If it is an integer, this property is the same in `il`. If it is a string equal `GAUSS`, this property is equal to 1 in `il`.

In NASTRAN, `Isop` is either a string or an integer. If it is an integer, this property is the same in `il`. If it is a string equal `FULL`, this property is equal to 1 in `il`.

If `Fctn` is equal to `FLUID` in the NASTRAN Bulk file, it is equal to 1 in `il` and elements are read as `flui*` elements.

- `MAT9` and `m_elastic 3` differ by the order of shear stresses yz, zx, Gxy in SDT and xy, yz, zx in NASTRAN. The order of constitutive values is thus different, which is properly corrected in SDT 6.5.

See also

`nasread`, `ufread`, `ufwrite`

nor2res, nor2ss, nor2xf

Purpose Transformations from normal mode models to other model formats.

Syntax

```
[res,po,psib,cpsi] = nor2res( ... )      % sdtweb('res') for format
RES = nor2res( ... )
[a,b,c,d] = nor2ss ( ... )             % sdtweb('ss') for format
SYS = nor2ss ( ... )
xf = nor2xf ( ... )                   % sdtweb('xf') for format
... = nor2.. (DEF,MODEL, ... )        % high level input
... = nor2.. (DEF,ga,MODEL, ... )
... = nor2.. (ga,om,pb,cp, ... )      % low level input
... = nor2ss ( ... , ind,fc,OutputCmd) % frequency,truncation...
... = nor2xf ( ... , w,ind,fc,OutputCmd)
```

Description These functions provide detailed access, for simple high level calls see [fe2ss](#). Normal mode models are second order models detailed in the [Theory](#) section below. [nor2res](#), [nor2ss](#), and [nor2xf](#) provide a number of transformations from the normal mode form to residue, state-space, and transfer function formats.

The normal mode model is specified using either high level structure arguments [DEF,MODEL](#) (where the model is assumed to contain load and sensor case entries) or low level numeric arguments [om,ga,pb,cp](#). Additional arguments [w,ind,fc,OutputCmd](#) can or must be specified depending on the desired output. These arguments are listed below.

DEF,MODEL

The normal mode shapes are given in a [DEF](#) structure with fields [.def](#), [.DOF](#), [.data](#) (see section 7.8).

These mode shapes are assumed mass normalized and the first column of the [.data](#) field is assumed to give modal frequencies **in Hz**. They can be computed with [fe_eig](#) or imported from an external FEM code (see [FEMLink](#)). See also [fe2ss](#).

Damping can be declared in different ways

- modal damping ratio can be given in [DEF.data\(:,2\)](#). When this column exists other damping input is ignored.

- `damp` a vector of modal damping ratio can be given as the second argument `nor2ss(DEF,damp,MODEL)`, or as an `info,DefaultZeta` entry as shown in the example below.
- a modal damping matrix `ga` can be given as the second argument. Note that this modal damping matrix is assumed to use frequency units consistent with the specified frequencies. Thus a physical viscous damping matrix will need to be divided by 2π (see `demo_fe`).
- hysteretic modal damping is not systematically supported since it leads to complex valued state-space models. You can compute FRFs with an hysteretic modal damping model using

```
def.data=sqrt(real(def.data.^2)).*sqrt(1+i*damp*2);
IIxh=nor2xf(def,[],model,w,'hz');
```

as illustrated in section 5.3.2.

Inputs and outputs are described by a model containing a `Case` (see section 4.2). Giving the model is needed when inputs correspond to distributed loads (`FVol` or `FSurf` case entries detailed under `fe_load`). `SensDof` are the only output entries currently supported (see `fe_case`).

Note that `DofSet` entries are handled as acceleration inputs. The basis described by `DEF` must allow a correct representation of these inputs. This can be achieved by using a basis containing static corrections for unit displacements or loads on the interface (see `fe2ss` `CraigBampton` or `Free` commands). A proper basis can also be generated using acceleration inputs at single nodes where a large seismic mass is added to the model. This solution is easier to implement when dealing with external FEM codes.

Here is a sample call that compares responses for two damping levels

```
[model,def]=demosdt('demogartfe');
InDof=[4.03;55.03;2.03]; OutDof=[4 55 30]'+.03;
freq=linspace(5,70,500)';
model=fe_case(model, ...
              'DofLoad','Force',InDof, ...
              'SensDof','Sensors',OutDof);
model=stack_set(model,'info','Freq',freq, ...
                'info','DefaultZeta',.01); % Ignored when def.data(:,2) exists
nor2xf(def,model,'acc iplot "Test" -po -reset');
```

```

% Another calling format not using the model
nor2xf(def,.05,InDof,OutDof,freq,'acc iplot "HighDamping"');
ci=iiplot;ci.Stack

```

When using distributed loads (pressure, etc.), the model elements are needed to define the load so that the `model` rather than a `Case` must be given as in the following example

```

model = demosdt('demo ubeam');
def=fe_eig(model,[106 20 10000 11 1e-5]);

%Pressure load
data=struct('sel','x==-.5', ...
           'eltsel','withnode {z>1.25}','def',1,'DOF',.19);
model=fe_case(model,'Fsurf','Surface load',data)
%Sensors
model=fe_case(model,'sensdof','Sensors',[50:54]'+.03);

fe_case(model,'info')
model=stack_set(model,'info','Freq',linspace(10,240,460));

nor2xf(def,0.01,model,'iplot "Test" -po -reset');

```

Example of transmissibility prediction using the large mass method where one defines a rigid base and a large mass such that one has 6 rigid body modes and fixed interface modes

```

model = demosdt('demo ubeam');

% define rigid base
i1=feutil('findnode z==0',model);
model = fe_case(model,'reset', ...
               'rigid append','Base',[i1(1);123456;i1(2:end)]);
% Add large mass on the base
model.Elt(end+[1:2],1:7)=[Inf abs('mass1') 0;
                          i1(1) [1 1 1 1 1 1]*1e6];

def=fe_eig(model,[5 20 1e3]); % This can be computed elsewhere

% Transmissibility for unit acceleration along x
model=fe_case(model,'DofSet','IN', ...

```

nor2res, nor2ss, nor2xf

```
    struct('def',[1;0;0;0;0;0],'DOF',i1(1)+[1:6]'/100), ...
        'SensDof','OUT',[1.01;314.01]);
f=linspace(50,500,1024)';
nor2xf(def,.01,model,f,'acc iipplot "Trans-Large" -reset');

% Clean approach without the large mass
mo2=stack_set(model,'info','EigOpt',[5 14 1e3]);
mo2=fe_case(mo2,'DofSet','IN',i1(1));
SE=fe_reduc('CraigBampton -se',model); % craig-bampton reduction
% Free modes of Craig-Bampton basis
TR=fe_eig({SE.K{:} SE.DOF});TR.DOF=SE.TR.DOF;TR.def=SE.TR.def*TR.def;

nor2xf(TR,.01,model,f,'acc iipplot "Trans-Craig"');
iicom('ch2');
```

om,ga,pb,cp

Standard low level arguments **om** (modal stiffness matrix), **ga** (modal viscous damping matrix), **pb** (modal controllability) and **cp** (modal observability) used to describe *normal mode models* are detailed in section section 5.2. A typical call using this format would be

```
[model,def]=demosdt('demogartfe');
b = fe_c(def.DOF,[4.03;55.03])'; c = fe_c(def.DOF,[1 30 40]'+.03);
IIw=linspace(5,70,500)';
nor2xf(def.data,0.01,def.def'*b,c*def.def,IIw*2*pi, ...
    'Hz iipplot "Simul" -po -reset');
```

w,ind,fc,OutputCmd

Other arguments are

w frequencies (**in rad/s** unless **Hz** is specified in **OutputCmd**) where the FRF should be computed (for **nor2xf**). Can also be given as a **model.Stack{'info','Freq'}** entry.

ind (optional) gives the indices of modes to be retained

fc (optional) roll-off frequency or correction mode poles for static correction modes (for load input only)

`OutputCmd` (optional) is a string that can contain. `'Hz'` to specify that `w` and `wj` are given in Hz. Non diagonal `om` or `ga` are always given in rad/s. `'dis'`, `'vel'`, or `'acc'` are used to obtain displacement (default), velocity or acceleration output. `'struct'` is used to obtain a curve structure. `'iipplot "StackName" -po -reset'` can be used to display results in `iipplot`(see section 2.1.2). The optional `-po` is used to save poles in `ci.Stack'IdMain'` so that they can be displayed. `-reset` reinitializes the curve stack. `-zoh Ts` or `-foh Ts` can be used to obtain a discrete state-space model based on zero or first order hold approximations with the specified time step.

res

`nor2res` returns a complex mode model in the residue form

$$[\alpha(s)] = \sum_{j=1}^{2N} \frac{\{c\psi_j\} \{\psi_j^T b\}}{s - \lambda_j} = \sum_{j=1}^{2N} \frac{[R_j]}{s - \lambda_j}$$

This routine is particularly useful to recreate results in the identified residue form `res` for comparison with direct identification results from `id_rc`.

Pole residue models are always assumed to correspond to force to displacement transfer functions. Acceleration input or velocity, acceleration output specifications are thus ignored.

ss

`nor2ss` returns state-space models (see the theory section below).

When no roll-off frequency is specified, `nor2ss` introduces a correction, **for displacement only**, in the state-space models through the use of a non-zero `d` term. If a roll-off frequency `fc` is given, the static correction is introduced in the state-space model through the use of additional high frequency modes. Unlike the non-zero `D` term which it replaces, this correction also allows to correct for velocity contributions of truncated modes.

You can also specify `fc` as a series of poles (as many as inputs) given in the frequency/damping format (see `ii_pof`).

xf

nor2xf computes FRF (from u to y) associated to the normal mode model. When used with modal frequencies **freq** and a subset of the modes (specified by a non empty **ind**), **nor2xf** introduces static corrections for the truncated modes.

Theory

The basic normal mode form associated with load inputs $[b] \{u\}$ is (see section 5.2)

$$\begin{aligned} [[I] s^2 + [\Gamma] s + [\Omega^2]]_{NP \times NP} \{s\} &= [\phi^T b]_{NP \times NA} \{u(s)\}_{NA \times 1} \\ \{y(s)\} &= [c\phi]_{NS \times NP} \{p(s)\}_{NP \times 1} \end{aligned}$$

where the coordinates p are such that the mass is the identity matrix and the stiffness is the diagonal matrix of frequencies squared.

The associated state-space model has the form

$$\begin{aligned} \begin{Bmatrix} \dot{p}(t) \\ \ddot{p}(t) \end{Bmatrix} &= \begin{bmatrix} [0] & [I] \\ -[\Omega^2] & -[\Gamma] \end{bmatrix} \begin{Bmatrix} p(t) \\ \dot{p}(t) \end{Bmatrix} + \begin{bmatrix} 0 \\ \phi^T b \end{bmatrix} \{u(t)\} \\ \{y\} &= [c\phi \ 0] \begin{Bmatrix} p(t) \\ \dot{p}(t) \end{Bmatrix} + [0] \{u(t)\} \end{aligned}$$

When used with modal frequencies **wj** and a subset of the modes (specified by **ind**), **nor2ss** introduces static corrections for the truncated modes. When requesting velocity or acceleration output, static correction can only be included by using additional modes.

In cases with displacement output only, the static corrections are ranked by decreasing contribution (using a SVD of the **d** term). You can thus look at the input shape matrix **b** to see whether all corrections are needed.

nor2ss (and **nor2xf** by calling **nor2ss**) supports the creation of state-space models of transmissibilities (transfer functions from acceleration input to displacement, velocity or acceleration. For such models, one builds a transformation such that the inputs u_a associated with imposed accelerations correspond to states

$$\begin{Bmatrix} u_a \\ q_c \end{Bmatrix} = [T_I \ T_C] \{p\}$$

and solves the fixed interface eigenvalue problem

$$[T_C^T \Omega T_C - \omega_{jC}^2 T_C^T I T_C] \{\phi_{jC}\} = \{0\}$$

leading to basis $[T_I \hat{T}_C] = [T_I \ T_C [\phi_{jC}]]$ which is used to build the state space model

$$\begin{aligned} \begin{Bmatrix} \dot{u} \\ \dot{q}_C \\ \ddot{u} \\ \ddot{q}_C \end{Bmatrix} &= \begin{bmatrix} [0] \\ 0 \\ -\hat{T}_C^T \Omega [T_I \ \hat{T}_C] \\ [I] \\ 0 \\ -\hat{T}_C^T \Gamma [T_I \ \hat{T}_C] \end{bmatrix} \begin{Bmatrix} u \\ q_C \\ \dot{u} \\ \dot{q}_C \end{Bmatrix} + \\ &\begin{bmatrix} 0 & 0 \\ 0 & 0 \\ 0 & I \\ \hat{T}_C^T b & \hat{T}_C^T T_I \end{bmatrix} \begin{Bmatrix} u_F \\ \ddot{u}_a \end{Bmatrix} \\ \{y\} &= [c_{T_I} \ c_{\hat{T}_C} \ 0 \ 0] \begin{Bmatrix} u_a \\ q_C \\ \dot{u}_a \\ \dot{q}_C \end{Bmatrix} + [0] \begin{Bmatrix} u_F \\ \ddot{u}_a \end{Bmatrix} \end{aligned}$$

Simple adjustments lead to velocity and acceleration outputs.

When using acceleration input, care must be taken that the initial shapes of the normal mode model form an appropriate basis. This can be achieved by using a basis containing static corrections for unit displacements or loads on the interface (see [fe2ss](#) [CraigBampton](#) or [Free](#) commands) or a seismic mass technique.

See also [res2nor](#), [id_nor](#), [fe_c](#), [psi2nor](#)
[demo_fe](#)

of2vtk

Purpose Export model and deformations to **VTK** format for visualization purposes.

Syntax `opfem2VTK(FileName,model)`
`opfem2VTK(FileName,model,val1,...,valn)`

Description Simple function to write the mesh corresponding to the structure model and associated data currently in the “Legacy VTK file format” for visualization.

To visualize the mesh using VTK files you may use **ParaView** which is freely available at <http://www.paraview.org> or any other visualization software supporting **VTK** file formats.

```
try;tname=nam2up('tempname.vtk');catch;tname=[tempname '.vtk'];end
model=femesh('testquad4');
```

```
NodeData1.Name='NodeData1';NodeData1.Data=[1 ; 2 ; 3 ; 4];
NodeData2.Name='NodeData2';NodeData2.Data=[0 0 1;0 0 2;0 0 3;0 0 4];
of2vtk('fic1',model,NodeData1,NodeData2);
```

```
EltData1.Name = 'EltData1' ;EltData1.Data = [ 1 ];
EltData2.Name = 'EltData2' ;EltData2.Data = [ 1 2 3];
of2vtk('fic2',model,EltData1,EltData2);
```

```
def.def = [0 0 1 0 0 0 0 2 0 0 0 0 0 3 0 0 0 0 0 4 0 0 0 ]'*[1 2];
def.DOF=reshape(repmat((1:4),6,1)+repmat((1:6)'/100,1,4), [],1)
def.lab={'NodeData3', 'NodeData4'};
of2vtk('fic3',model,def);
```

```
EltData3.EltId=[1];EltData3.data=[1];EltData3.lab={'EltData3'};
EltData4.EltId=[2];EltData4.data=[2];EltData4.lab={'EltData4'};
of2vtk('fic4',model,EltData3,EltData4);
```

The default extension `.vtk` is added if no extension is given.

Input arguments are the following:

FileName

file name for the VTK output, no extension must be given in FileName, “FileName.vtk” is automatically created.

model

a structure defining the model. It must contain at least fields `.Node` and `.Elt`.
FileName and model fields are mandatory.

vali

To create a VTK file defining the mesh and some data at nodes/elements (scalars, vectors) you want to visualize, you must specify as many inputs *vali* as needed. *vali* is a structure defining the data: *vali = struct('Name', ValueName, 'Data', Values)*. Values can be either a table of scalars ($Nnode \times 1$ or $Nelt \times 1$) or vectors ($Nnode \times 3$ or $Nelt \times 3$) at nodes/elements. Note that a deformed model can be visualized by providing nodal displacements as data (e.g. in ParaView using the “warp” function).

ofact

Purpose Factored matrix object.

Syntax

```
ofact
ofact('method MethodName');
kd=ofact(k); q = kd\b; ofact('clear',kd);
kd=ofact(k,'MethodName')
```

Description The factored matrix object `ofact` is designed to let users write code that is independent of the library used to solve static problems of the form $[K] \{q\} = \{F\}$. For FEM applications, choosing the appropriate library for that purpose is crucial. Depending on the case you may want to use full, skyline, or sparse solvers. Then within each library you may want to specify options (direct, iterative, in-core, out-of-core, parallel, ...).

Using the `ofact` object in your code, lets you specify method at run time rather than when writing the code. Typical steps are

```
ofact('method spfmex'); % choose method
kd = ofact(k);           % create object and factor
static = kd\b           % solve
ofact('clear',kd)      % clear factor when done
```

For single solves `static=ofact(k,b)` performs the three steps (factor, solve clear) in a single pass.

The first step of method selection provides an open architecture that lets users introduce new solvers with no need to rewrite functions that use `ofact` objects. Currently available methods are listed simply by typing

```
>> ofact
```

```
Available factorization methods for OFACT object
```

```
-> spfmex : SDT sparse LDLt solver
    sp_util : SDT skyline solver
        lu : MATLAB sparse LU solver
        mtaucs : TAUCS sparse solver
    pardiso : PARDISO sparse solver
        chol : MATLAB sparse Cholesky solver
    *psldlt : SGI sparse solver (NOT AVAILABLE ON THIS MACHINE)
```

and the method used can be selected with `ofact('method MethodName')`. SDtools maintains pointers to pre-compiled solvers at http://www.sdtools.com/faq/FE_ofact.html.

The factorization `kd = ofact(k)`; and resolution steps `static = kd\b` can be separated to allow multiple solves with a single factor. Multiple solves are essential for eigenvalue and quasi-newton solvers. `static = ofact(k)\b` is of course also correct.

The clearing step is needed when the factors are not stored as MATLAB variables. They can be stored in another memory pile, in an out-of-core file, or on another computer/processor. Since for large problems, factors require a lot of memory. Clearing them is an important step.

Historically the object was called `skyline`. For backward compatibility reasons, a `skyline` function is provided.

umfpack

To use UMFPACK as a `ofact` solver you need to install it on your machine. This code is available at www.cise.ufl.edu/research/sparse/umfpack.

pardiso

Based on the Intel MKL (Math Kernel Library), you should use version 8 and after.

By default the pardiso call used in the `ofact` object is set for symmetric matrices. For non-symmetric matrices, you have to complement the `ofact` standard command for factorization with the character string `'nonsym'`. Moreover, when you pass a matrix from Matlab to PARDISO, you **must transpose** it in order to respect the PARDISO sparse matrix format.

Assuming that k is a real non-symmetric matrix and b a real vector, the solution q of the system $k.q = b$ is computed by the following sequence of commands:

```
ofact pardiso                % select PARDISO solver
kd = ofact('fact nonsym',k'); % factorization
q=kd\b;                      % solve
ofact('clear',kd);          % clear ofact object
```

The factorization is composed of two steps: symbolic and numerical factorization. For the first step the solver needs only the sparse matrix structure (i.e. non-zeros location), whereas the actual data stored in the matrix are required in the second step only. Consequently, for a problem with a unique factorization, you can group

the steps. This is done with the standard command `ofact('fact',...)`.

In case of multiple factorizations with a set of matrices having the same sparse structure, only the second step should be executed for each factorization, the first one is called just for the first factorization. This is possible using the commands `'sybmfact'` and `'numfact'` instead of `'fact'` as follows:

```
kd = ofact('sybmfact',k); % just one call at the beginning
...
kd = ofact('numfact',k,kd); % at each factorization
q=kd\b; %
...
ofact('clear',kd); % just one call at the end
```

You can extend this to **non-symmetric systems** as described above.

spfmex

`spfmex` is a sparse multi-frontal solver based on `spooles` a compiled version is provided with SDT distributions.

sp_util

The skyline matrix storage is a traditional form to store the sparse symmetric matrices corresponding to FE models. For a full symmetric matrix `kfull`

```
kfull=[1  2
        10 5  8 14
          6  0  1
            9  7
          sym.      11 19
                           20]
```

The non-zero elements of each column that are above the diagonal are stored sequentially in the data field `k.data` from the diagonal up (this is known as the reverse Jenning's representation) and the indices of the elements of `k` corresponding to diagonal elements of the full matrix are stored in an index field `k.ind`. Here

```
k.data = [1; 10; 2; 6; 5; 9; 0; 8; 11; 7; 1; 14; 20; 19; 0]
k.ind  = [1; 2; 4; 6; 9; 13; 15];
```

For easier manipulations and as shown above, it is assumed that the index field `k.ind` has one more element than the number of columns of `kfull` whose value is the index of a zero which is added at the end of the data field `k.data`.

If you have imported the `ind` and `data` fields from an external code, `ks = ofact(data, ind)` will create the `ofact` object. You can then go back to the MATLAB sparse format using `sparse(ks)` (but this needs to be done before the matrix is factored when solving a static problem).

Your solver

To add your own solver, simply add a file called `MySolver_utils.m` in the `@ofact` directory. This function must accept the commands detailed below.

Your object can use the fields `.ty` used to monitor what is stored in the object (0 unfactored `ofact`, 1 factored `ofact`, 2 LU, 3 Cholesky, 5 other), `.ind`, `.data` used to store the matrix or factor in true `ofact` format, `.dinv` inverse of diagonal (currently unused), `.l` L factor in `lu` decomposition or transpose of Cholesky factor, `.u` U factor in `lu` decomposition or Cholesky factor, `.method` other free format information used by the object method.

method

Is used to define defaults for what the solver does.

fact

This is the callback that is evaluated when `ofact` initializes a new matrix.

solve

This is the callback that is evaluated when `ofact` overloads the matrix left division (`\`)

clear

`clear` is used to provide a clean up method when factor information is not stored within the `ofact` object itself. For example, in persistent memory, in another process or on another computer on the network.

See also `fe_eig`, `fe_reduc`

perm2sdt

Purpose Read results from outputs of the PERMAS (V7.0) finite element code.

Syntax

```
out = perm2sdt('Read Model_FileName')
out = perm2sdt('Read Result_FileName')
out = perm2sdt('merge',model)
out = perm2sdt('binary.mtl Matrix_FileName')
out = perm2sdt('ascii.mtl Matrix_FileName')
```

Description The `perm2sdt` function reads PERMAS model, result and matrices files. Binary and ASCII files are supported.

Model files

To read a FE model, use the following syntax: `model = perm2sdt('Read FileName')`

To deal with sub-components, you may use the `merge` command.

The current element equivalence table is

<i>SDT</i>	<i>PERMAS</i>
<code>mass2</code>	MASS3, MASS6, X1GEN6
<code>bar1</code>	FLA2
<code>beam1</code>	PLOTL2, BECOC, BECOS, BECOP, BETOP, BETAC, FD-PIPE2, X2GEN6
<code>celas</code>	SPRING3, SPRING6, SPRING1, X2STIFF3
<code>t3p</code>	TRIM3
<code>tria3</code>	TRIA3, TRIA3K, TRIA3S, FSINTA3
<code>quad4</code>	QUAD4, FSINTA4, QUAD4S, PLOTA4, SHELL4
<code>flui4</code>	FLTET4
<code>tetra4</code>	TET4
<code>tetra10</code>	TET10
<code>penta6</code>	PENTA6, FLPENT6
<code>hexa8</code>	HEXE8, FLHEX8
<code>pyra5</code>	PYRA5, FLPYR5

Merging model

The `merge` command integrates subcomponents into the main model.

Result files

The syntax is

```
perm2sdt('read result_file')
```

Matrix files

`perm2sdt` reads binary and ASCII `.mtl` file format. The syntax is

```
perm2sdt('binary.mtl File.mtl') for binary files and perm2sdt('ascii.mtl File.mtl') for ASCII files.
```

See also

[FEMLink](#)

psi2nor

Purpose Estimation of normal modes from a set of scaled complex modes.

Syntax `[wj,ga,cps,pbs] = psi2nor(po,cp)`
`[wj,ga,cps,pbs] = psi2nor(po,cp,ncol,NoCommentFlag)`

Description `psi2nor` should generally be used through `id_nor`. For cases with as many and more sensors than modes, `psi2nor` gives, as proposed in Ref. [12], a proper approximation of the complex mode outputs `cp = [c] [ψ]` (obtained using `id_rm`), and uses the then exact transformation from complex to normal modes to define the normal mode properties (modal frequencies `wj`, non-proportional damping matrix `ga`, input `pbs = [φ]T [b]` and output `cps = [c] [φ]` matrices).

The argument `ncol` allows the user to specify the numbers of a restricted set of outputs taken to have a collocated input (`pbs=cps(ncol,:)`).

If used with less than four arguments (not using the `NoCommentFlag` input argument), `psi2nor` will display two indications of the amount of approximation introduced by using the proper complex modes. For the complex mode matrix ψ_T (of dimensions NT by $2NT$ because of complex conjugate modes), the properness condition is given by $\psi_T \psi_T^T = 0$. In general, identified modes do not verify this condition and the norm $\|\psi_T \psi_T^T\|$ is displayed

`The norm of psi*psi' is 3.416e-03 instead of 0`

and for well identified modes this norm should be small (10^{-3} for example). The algorithm in `psi2nor` computes a modification $\Delta\psi$ so that $\tilde{\psi}_T = \psi_T + \Delta\psi$ verifies the properness condition $\tilde{\psi}_T \tilde{\psi}_T^T = 0$. The mean and maximal values of `abs(dpsi./psi)` are displayed as an indication of how large a modification was introduced

`The changes implied by the use of proper cplx modes are 0.502 maximum and 0.1`

The modified modes do not necessarily correspond to a positive-definite mass matrix. If such is not the case, the modal damping matrix cannot be determined and this results in an error. Quite often, a non-positive-definite mass matrix corresponds to a scaling error in the complex modeshapes and one should verify that the identification process (identification of the complex mode residues with `id_rc` and determination of scaled complex mode outputs with `id_rm`) has been accurately done.

Warnings The complex modal input is assumed to be properly scaled with reciprocity constraints (see `id_rm`). After the transformation the normal mode input/output ma-

trices verify the same reciprocity constraints. This guarantees in particular that they correspond to mass-normalized analytical normal modes.

For lightly damped structures, the average phase of this complex modal output should be close to the -45° line (a warning is given if this is not true). In particular a sign change between collocated inputs and outputs leads to complex modal outputs on the $+45^\circ$ line.

Collocated force to displacement transfer functions have phase between 0 and -180° , if this is not verified in the data, one cannot expect the scaling of [id_rm](#) to be appropriate and should not use [psi2nor](#).

See also [id_rm](#), [id_nor](#), [id_rc](#), [res2nor](#), [nor2xf](#), [nor2ss](#), the [demo_id](#) demonstration

qbode

Purpose Frequency response functions (in the `xf` format) for linear systems.

Syntax

```
xf = qbode(a,b,c,d,w)
xf = qbode(ss,w)
xf = qbode(num,den,w)
XF = qbode( ... , 'struct')
      qbode( ... , 'iplot ...')
```

Description For state-space models described by matrices `a`, `b`, `c`, `d`, or the LTI state-space object `sys` (see *Control System Toolbox*), `qbode` uses an eigenvalue decomposition of `a` to compute, in a minimum amount of time, all the FRF `xf` at the frequency points `w`

$$\mathbf{xf} = [C] (s [I] - [A])^{-1} [B] + [D]$$

The result is stored in the `xf` format (see details page 167). `'iplot "Test" -po -reset'` can be used to display results in `iplot` (see section 2.1.2). The option `-po` is used to save poles in `ci.Stack{'IdMain'}` so that they can be displayed. `-reset` reinitializes the curve stack.

qbode will not work if your model is not diagonalizable. A specific algorithm was developed to deal with systems with rigid-body modes (double pole at zero associated to non-diagonalizable matrices). This algorithm will not, however, indicate the presence of incoherent `b` and `c` matrices. In other cases, you should then use the direct routines `res2xf`, `nor2xf`, etc. or the `bode` function of the *Control System Toolbox*.

For the polynomial models `num`, `den` (see details page 166), `qbode` computes the FRF at the frequency points `w`

$$\mathbf{xf} = \frac{\text{num}(j\omega)}{\text{den}(j\omega)}$$

Warnings

- All the SISO FRF of the system are computed simultaneously and the complex values of the FRF returned. This approach is good for speed but not always well numerically conditioned when using state space models not generated by the *SDT*.
- As for all functions that do not have access to options (`IDopt` for identification and `Up.copt` for FE model update) frequencies are assumed to be given

in the mathematical default (rad/s). If your frequencies `w` are given in Hz, use `qbode(sys,w*2*pi)`.

- Numerical conditioning problems may appear for systems with several poles at zero.

See also [demo_fe](#), [res2xf](#), [nor2xf](#), and [bode](#) of the *Control System Toolbox*

res2nor

Purpose Approximate transformation from complex residues to normal mode residue or proportionally damped normal mode forms.

Syntax

```
[Rres,po,Ridopt] = res2nor(Cres,po,Cidopt)
[wj,ga,cp,pb] = res2nor(Cres,po,Cidopt)
```

Description The contributions of a pair of conjugate complex modes (complex conjugate poles λ and residues R) can be combined as follows

$$\frac{[R]}{s - \lambda} + \frac{[\bar{R}]}{s - \bar{\lambda}} = 2 \frac{(s\text{Re}(R)) + (\zeta\omega\text{Re}(R) - \omega\sqrt{1 - \zeta^2}\text{Im}(R))}{s^2 + 2\zeta\omega s + \omega^2}$$

Under the assumption of proportional damping, the term $s\text{Re}(R)$ should be zero. `res2nor`, assuming that this is approximately true, sets to zero the contribution in s and outputs the normal mode residues `Rres` and the options `Ridopt` with `Ridopt.Fit = 'Normal'`.

When the four arguments of a normal mode model (see `nor` page 154) are used as output arguments, the function `id_rm` is used to extract the input `pbs` and output `cps` shape matrices from the normal mode residues while the frequencies `wj` and damping matrix `ga` are deduced from the poles.

Warning This function assumes that a proportionally damped model will allow an accurate representation of the response. For more accurate results use the function `id_nor` or identify using real residues (`id_rc` with `idopt.Fit='Normal'`).

See also `id_rm`, `id_rc`, `id_nor`, `res2ss`, `res2xf`

res2ss, ss2res

Purpose Transformations between the residue `res` and state-space `ss` forms.

Syntax

```
SYS          = res2ss(RES)
SYS          = res2ss(RES, 'AllIO')
[a,b,c,d]    = res2ss(res,po,idopt)
RES          = ss2res(SYS)
[res,po,idopt] = ss2res(a,b,c,d)
```

Description The functions `res2ss` and `ss2res` provide transformations between the complex / normal mode residue forms `res` (see section 5.6) and the state space forms (see section 5.4). You can use either high level calls with data structures or low level calls providing each argument

```
ci=demosdt('demo gartid est')
SYS = res2ss(ci.Stack{'IdMain'});
RES = ss2res(SYS);
ID=ci.Stack{'IdMain'};
[a,b,c,d] = res2ss(ID.res,ID.po,ID.idopt);
```

Important properties and limitations of these transformations are

res2ss

- The residue model should be minimal (a problem for MIMO systems). The function `id_rm` is used within `res2ss` to obtain a minimal model (see section 2.4.1). To obtain models with multiple poles use `id_rm` to generate `new_res` and `new_po` matrices.
- `idopt.Reciprocity='1 FRF'` or MIMO `id_rm` then also constrains the system to be reciprocal, this may lead to differences between the residue and state-space models.
- The constructed state-space model corresponds to a displacement output.
- Low frequency corrections are incorporated in the state-space model by adding a number (minimum of `ns` and `na`) of poles at 0.

Asymptotic corrections (see `idopt.ResidualTerms`) other than the constant and s^{-2} are not included.

- See below for the expression of the transformation.

- The 'AllIo' input can be used to return all input/output pairs when assuming reciprocity.

ss2res

- Contributions of rigid-body modes are put as a correction (so that the pole at zero does not appear). A real pole at 0 is not added to account for contributions in $1/s$.
- To the exception of contributions of rigid body modes, the state-space model must be diagonalizable (a property verified by state-space representations of structural systems).

Theory

For control design or simulation based on identification results, the minimal model resulting from `id_rm` is usually sufficient (there is no need to refer to the normal modes). The state-space form is then the reference model form.

As shown in section 2.4.1, the residue matrix can be decomposed into a dyad formed of a column vector (the modal output), and a row vector (the modal input). From these two matrices, one derives the $[B]$ and $[C]$ matrices of a real parameter state-space description of the system with a bloc diagonal $[A]$ matrix

$$\begin{cases} \dot{x}_1 \\ \dot{x}_2 \end{cases} = \begin{bmatrix} [0] & [I] \\ -[\omega_j^2] & [2\zeta_j\omega_j] \end{bmatrix} \begin{cases} x_1 \\ x_2 \end{cases} + \begin{cases} B_1 \\ B_2 \end{cases} \{u(t)\}$$

$$\{y(t)\} = [C_1 \ C_2] \begin{cases} x_1 \\ x_2 \end{cases}$$

where the blocks of matrices B_1 , B_2 , C_1 , C_2 are given by

$$\begin{cases} C_{1j} \\ C_{2j} \end{cases} = \frac{1}{\omega_j \sqrt{1-\zeta_j^2}} \begin{bmatrix} \omega_j \sqrt{1-\zeta_j^2} & \zeta_j \omega_j \\ 0 & 1 \end{bmatrix} \begin{bmatrix} \text{Re}(c\psi_j) \\ \text{Im}(c\psi_j) \end{bmatrix}$$

$$\begin{cases} B_{j1} \\ B_{j2} \end{cases} = 2 \begin{bmatrix} \text{Re}(\psi_j^T b) & \text{Im}(\psi_j^T b) \end{bmatrix} \begin{bmatrix} 1 & 0 \\ \zeta_j \omega_j & -\omega_j \sqrt{1-\zeta_j^2} \end{bmatrix}$$

Form the state space model thus obtained, FRFs in the `xf` format can be readily obtained using `qbode`. If the state space model is not needed, it is faster to use `res2xf` to generate these FRFs.

See also

[demo_fe](#), [res2xf](#), [res2nor](#), [qbode](#), [id_rm](#), [id_rc](#)

res2tf, res2xf

Purpose Create the polynomial representation associated to a residue model.
Compute the FRF corresponding to a residue model.

Syntax

```
[num,den] = res2tf(res,po,idopt)
xf        = res2xf(res,po,w,idopt)
xf        = res2xf(res,po,w,idopt,RetInd)
```

Description For a set of residues `res` and poles `po` (see `res` page 165), `res2tf` generates the corresponding polynomial transfer function representation (see `tf` page 166)).

For a set of residues `res` and poles `po`, `res2xf` generates the corresponding FRFs evaluated at the frequency points `w`. `res2xf` uses the options `idopt.Residual`, `.DataType`, `AbcissaUnits`, `PoleUnits`, `FittingModel`. (see `idopt` for details).

The FRF generated correspond to the FRF used for identification with `id_rc` except for the complex residue model with positive imaginary poles only `idopt.Fit='Posit'` where the contributions of the complex conjugate poles are added.

For MIMO systems, `res2tf` and `res2xf` do not restrict the pole multiplicity. These functions and the `res2ss`, `qbode` sequence are thus not perfectly equivalent. A unit multiplicity residue model for which the two approaches are equivalent can be obtained using the matrices `new_res` and `new_po` generated by `id_rm`

```
[psib,cpsi,new_res,new_po]=id_rm(IIres,IIpo,idopt,[1 1 1 1]);
IIxh = res2xf(new_res,new_po,IIw,idopt);
```

The use of `id_rm` is demonstrated in `demo_id`.

See also `res2ss`, `res2nor`, `qbode`, `id_rm`, `id_rc`

rms

Purpose Computes the RMS response of the given frequency response function **xf** or auto-spectra **a** to a unity white noise input over the frequency range **w**.

Syntax

```
rm = feval(id_rc('@rms'),t,w)
rm = feval(id_rc('@rms'),a,w,1)
```

Description The presence of a third input argument indicates that an auto-spectrum **a** is used (instead of frequency response function **xf**).

A trapezoidal integration is used to estimate the root mean squared response

$$\mathbf{rms} = \sqrt{\frac{1}{2\pi} \int_{\omega_1}^{\omega_2} |t(\omega)|^2 d\omega} = \sqrt{\frac{1}{2\pi} \int_{\omega_1}^{\omega_2} a(\omega) d\omega}$$

If **xf** is a matrix containing several column FRF, the output is a row with the RMS response for each column.

Warning If only positive frequencies are used in **w**, the results are multiplied by 2 to account for negative frequencies.

See also [ii_cost](#)

samcef

Purpose Interface function with SAMCEF FEM code.

Syntax

```
Up=samcef('read model.u18')
Up=samcef('read model.u18','buildup')
Up=samcef('read model.bdf','buildup')
a=samcef('lectmat','FileRoot')
samcef('write FileName',model)
```

Description `read`

By itself the `read` commands imports the model (not the properties since those are not stored explicitly in the `.u18` file. With the `'buildup'` argument, the `.u11` and `.u12` files are also read to import element matrices into a superelement. Additional DOFs linked to reduced shear formulations are properly condensed.

Since the properties are not read, there are some difficulties knowing DOFs actually used in the model. You should then start by declaring those properties in `format` before calling the `read` command.

```
model=samcef('read test_dy.u18'); % read model
% define properties
model.pl=m_elastic('dbval 1 steel','dbval 5 steel');
model.il=p_shell('dbval 100 kirchhoff .1 -f5','dbval 1 kirchhoff .1 -f5')
% check that properties are valid
[Case,model.DOF]=fe_mkn1('initnocon',model);
% Now read the element matrices (from .u11 and .u12 files)
model=samcef('read test_dy.u18',model,'buildup')
```

When reading `.u18` files, modeshapes are stored in the model stack entry `curve,record()`. Other imported results are also stored in the stack.

`write`

Basic writing is supported with `samcef('write FileName',model)`. Please send requests to extend these capabilities.

conv

This command lists conversion tables for elements, topologies, facetologies. You can redefine (enhance) these tables by setting preferences of the form `setpref('FEMLink', 'samcef.list', value)`, but please also request enhancements so that the quality of our translators is improved.

See also [FEMLink](#)

setlines

Purpose Line color and style sequencing utility.

Syntax

```
setlines
setlines(ColorMap,LineSequence)
setlines(ColorMapName,LineSequence,MarkerSequence)
```

Description The M-by-3 `ColorMap` or `ColorMapName` (standard color maps such as `jet`, `hsv`, etc.) is used as color order in place or the `ColorMap` given in the `ColorOrder` axis property (which is used as a default).

The optional `LineSequence` is a matrix giving the `linestyle` ordering whose default is `['- ' ; '--' ; '-.' ; ':']`.

The optional `MarkerSequence` is a matrix giving the `marker` ordering. Its default is empty (marker property is not set).

For all the axes in the current figure, `setlines` finds solid lines and modifies the `Color`, `LineStyle` and `Marker` properties according the arguments given or the defaults. Special care is taken to remain compatible with plots generated by `feplot` and `iipplot`.

`setlines` is typically used to modify line styles before printing. Examples would be

```
setlines k
setlines([], '-','ox*s')
setlines(get(gca,'colororder'),' : ','o+^>')
```

sdtdef

Purpose Internal function used to handle default definitions.

Syntax

```
sdtdef('info')  
sdtdef('ConstantName',Value)  
sdtdef('ConstantName')
```

Description For an exact list of current defaults use `sdtdef('info')`. To reset values to factory defaults use `sdtdef('factory')`.

Values that you are likely to need changing are

avi cell array of default AVI properties, see the MATLAB `avifile` command.

DefaultFeplot cell array of default `feplot` figure properties. For MATLAB versions earlier than 6.5, the OpenGL driver is buggy so you will typically want to set the value with

```
sdtdef('DefaultFeplot',{ 'Renderer' 'zbuffer' ...  
'doublebuffer' 'on'})
```

eps1 tolerance on node coincidence used by `femesh`, `feutil`. Defaults to 1e-6 which is generally OK except for MEMS applications, ...

The following MATLAB preferences can also be used to customize SDT behavior for your particular needs

SDT **DefaultZeta** Default value for the viscous damping ratio. The nominal value is `1e-2`. The value can also be specified in a model stack and is then handled by `fe_def defzeta` and `fe_def defeta` commands.

SDT **KikeMemSize** Memory in megabytes used to switch to an out-of-core saving of element matrix dictionaries.

<i>SDT</i>	<code>tempdir</code>	<code>tempdir</code> can be used to specify a directory different than the <code>tempdir</code> returned by MATLAB. This is typically used to specify a faster local disk.
<i>SDT</i>	<code>OutOfCoreBufferSize</code>	Memory in bytes used to decide switching to an out-of-core procedure. This is currently used by <code>nasread</code> when reading large <code>OUTPUT2</code> files.
<i>FEMLink</i>	<code>CopyFcn</code>	command used to copy file to remote locations. See <code>naswrite job</code> commands.
<i>FEMLink</i>	<code>DmapDir</code>	directory where <code>FEMLink</code> is supposed to look for NASTRAN DMAP and standard files.
<i>FEMLink</i>	<code>NASTRAN</code>	NASTRAN version. This is used to implement version dependent writing of NASTRAN files.
<i>FEMLink</i>	<code>RemoteDir</code>	location of remote directory where files can be copied. See <code>naswrite job</code> commands.
<i>FEMLink</i>	<code>SoftwareDocRoot</code>	defines the path or URL for a given software. You can use <code>sdtweb('\$Software/file.html')</code> commands to access the proper documentation. For example <pre>setpref('FEMLink','SdtDocRoot', ... 'http://www.sdtools.com/help/'); sdtweb('\$sdt/sdt.html');</pre>
<i>FEMLink</i>	<code>TextUnix</code>	set to 1 if text needs to be converted to UNIX (rather than DOS) mode before any transfer to another machine.

Note that these definitions are available for the current session. If you want to use permanent preferences, you should use the `getpref/setpref` Matlab functions and define the `SDT` preferences.

sdth

Purpose Class constructor for *SDT* handle objects.

Description The *Structural Dynamics Toolbox* now supports *SDT handles* ([sdth](#) objects). Currently implemented types for [sdth](#) objects are

[SDTRoot](#) global context information used by the toolbox
[IDopt](#) identification options (see [idopt](#))
[FeplotFig](#) [feplot](#) figure handle
[IiplotFig](#) [iiplot](#) figure handle
[VectCor](#) Vector correlation handle (see [ii_mac](#))
[XF](#) stack pointer (see [xfopt](#))

[SDT handles](#) are wrapper objects used to give easier access to user interface functions. Thus [idopt](#) displays a detailed information of current identification options rather than the numeric values really used.

Only advanced programmers should really need access to the internal structure of [SDT handles](#). The fixed fields of the object are [opt](#), [type](#), [data](#), [GHandle](#) (if the [sdth](#) object is stored in a graphical object), and [vfields](#).

Most of the information is stored in the variable field storage field [vfields](#) and a field of [vfields](#) is accessible using [GetData](#). To get the model of a [cf FeplotFig](#), you may use the syntax [cf.mdl.GetData](#).

See also [feplot](#), [idopt](#), [iiplot](#), [ii_mac](#), [xfopt](#)

v_handle, sdthdf

Purpose Class constructor for variable handle objects.

Description The *Structural Dynamics Toolbox* supports variable handle objects, which act as **pointers** to variables that are actually stored as global variables, user data of graphical objects, or in files. This latter application may become very useful when handling very large models. `sdthdf` indeed allows RAM unloading by keeping data on drive while using a pointer to it. A trade-off between data access performance (limited to your drive I/O performance) and amount of free memory will occur.

Some supported file formats are MATLAB 6 `.mat` files, MATLAB 7.3 HDF based `.mat` files, NASTRAN `.op2`, ABAQUS `.fil` ...

`v_handle` objects are used to

- allow context dependent reference to a single MATLAB variable
- provides a graphic callback when modifying the object in a function or the command line.

`v_handle` objects essentially behave like global variables with the notable exception that a `clear` command only deletes the handle and not the pointed data.

Only advanced programmers should really need access to the internal structure of `v_handle`.

hdf Commands The new `hdf5` file format, supported by MATLAB since version 7.3, allows very efficient data access from files. Partial loading is possible, as well as data location by pointers. `sdthdf` allows the user to unload RAM by saving specific data to dedicated files, and to optimize file loading using pointers. To be able to use these functionalities, the file must have been saved in `hdf5` format, which is activated in MATLAB using the `-v7.3` option of the `save` function.

The following commands are supported.

`hdfreadref`

This command handles partial data loading, depending on the level specified by the user.

For unloaded data, a `v_handle` pointer respecting the data structure and names is generated, so that the access is preserved. Further `hdfreadref` application to this specific data can be done later.

By default, the full file is loaded. Command option `-level` allows specifying the desired loading level. For structured data, layers are organized in which substructures are leveled. This command allows data loading until a given layer. Most common levels used are given in the following list

- `-level0` Load only the data structure using pointers.
- `-level1` Load the data structure and fully load fields not contained in substructures.
- `-level2` Load the data structure, and fully load fields including the ones contained in the main data substructures
- `-level100` Load the data structure, and fully load all fields (Until level 100, which is generally sufficient).

It takes in argument either a file, or a data structure containing `hdf5 v_handle` pointers. In the case where a file is specified, the user can precise the data to be loaded, by giving its name preceded by a slash `/`, substructure names can also be specified giving the name path to the variable to be loaded with a succession of slashes.

```
% To load an hdf5 file
r1=sdthdf('hdfreadref','my_file.mat');
% To load it using \vhandle pointers
r1=sdthdf('hdfreadref-level0',my_file.mat');
% To load a specified variable
r2=sdthdf('hdfreadref-level0','my_file.mat','/var2');
% To load a specified sub data
r3=sdthdf('hdfreadref-level1','my_file.mat','/var2/subvar1');
% To load a subdata from a previously loaded pointer
r4=sdthdf('hdfreadref',r2.subvar1);
```

`hdfdbsave`

This command handles partial data saving to a temporary file. It is designed to unload large numerical data, such as sparse matrices, or deformation fields. Command option `-struct` however allows to save more complex data structures.

The function takes in argument the data to save and a structure with a field `Dbfile` containing the temporary file path (string). The function outputs the `v_handle` to the saved data. The `v_handle` has the same data structure than the original. The `v_handle` data can be recovered by `hdfreadref`.

```
opt.Dbfile=nam2up('tempname_DB.mat');
r1=sdthdf('hdfdbsave',r1,opt);
r2=sdthdf('hdfdbsave-struct',r2,opt);
```

`hdfmodelsave`

This command handles similar saving strategy than `hdfdbsave` but is designed to integrate `feplot` models in `hdf5` format. The file linked to the model is not supposed to be temporary, and data names are linked to an SDT model data structure, which are typically in the model stack. The variable data names, must be of format `field_name` to store `model.field` in `hdf5` format.

For model stack entries, the name must be of the type `Stack_type_name` to store `cf.Stack'type', 'name'`.

The function takes in argument the data base file, the `feplot` handle and the data name, which will be interpreted to be found in the `feplot` model. The data will be replaced by `v_handle` pointers in the `feplot` model. Data can be reloaded with command `hdfmodel`

```
sdthdf('hdfmodelsave','my_file.mat',cf,'Stack_type_name');
```

`hdfmodel`

This command loads `v_handle` data pointers in the `feplot` model at locations where `hdf5` data have been saved. This command works from the `hdf` file side, and loads all the data contained with standard names in the `feplot` model. See `hdfmodelsave` for more information on the standard data names. Commando option `-check` only loads the data contained in the `hdf` file that is already instanced in the `feplot` model.

```
sdthdf('hdfmodel','my_file.mat',cf);
```

`hdfclose`

Handling `hdf5` files in data structures can become very complex when multiple handles are generated in multiple data. This command thus aims to force a file to be closed.

v_handle, sdthdf

```
sdthdf('hdfclose','my_file.mat');
```

A lower level closing call allows clearing the `hdf5` libraries, when needed,

```
sdthdf('hdfH5close')
```

Example

Here is an example of offload to HDF5 based mat files, and how to access the data afterwards.

```
fname=fullfile(sdtdef('tempdir'),'ubeam_Stack_SE.mat');
fname2=fullfile(sdtdef('tempdir'),'ubeam_model.mat');
model=demosdt('demoubeam');cf=feplot;
cf.mdl=fe_case(cf.mdl,'assemble -matdes 2 1 NoT -SE');
cf.Stack{'curve','defR'}=fe_eig(cf.mdl,[5 50 1e3]);

% save(off-load) some stack entries to a file
sdthdf('hdfmodelsave',fname,cf,'Stack_curve_defR')
% save model but not the off-loaded entries
fecom('save',fname2);

cf=fecom('load',fname2); % reload the model
sdthdf('hdfmodel',fname,cf); % reload pointers to the entries
cf.Stack{'defR'}
```

For MATLAB ≥ 7.3 HDF based `.mat` files, you can open a `v_handle` pointer to a variable in the file using

```
fname=fullfile(sdtdef('tempdir'),'ubeam_Stack_SE.mat');
var=sdthdf('hdfreadref -level0',fname,'Stack_curve_defR')
```

See also

SDT handle

sdtweb

Purpose SDT file navigation function.

Description This function allows opening the SDT documentation, opening classical file types outside Matlab, and source code navigation.

`_taglist`

Opens the `TagList` navigator. This navigator opens a tree providing links to source code functions following the SDT code formatting.

- Command option `-sortABC` will display the navigation tree alphabetically sorted.
- Command option `-levelval` in combination with `sortABC` perform the alphabetical sorting up to level `val`.

One can open the TagList of the current opened file in the editor, typing

```
sdtweb _taglist
```

or specify a function for `_taglist` to open, typing

```
sdtweb _taglist feutil
```

The code typing convention are

```
function out=my_fun(varargin)
...
%% #Command -----
if comstr(Cam,'command'); [CAM,Cam]=comstr(Cam,8);
%% #CommandSub - - - - -
if comstr(Cam,'sub');
...
end
end
...
end % function
%% #SubFunc -----
%% #my_sub_fun - - - - -
function out=my_sub_fun(input)
```

```
end
%% #my_sub_fun_2 - - -----
function out=my_sub_fun_2(input)
end
```

[function_name]

Opens a function documentation.

E.g. to open the `feutil` documentation, one can type

```
sdtweb _taglist feutil
```

sp_util

Purpose Sparse matrix utilities.

Description This function should be used as a `mex` file. The `.m` file version does not support all functionalities, is significantly slower and requires more memory.

The `mex` code is **not** MATLAB **clean**, in the sense that it often modifies input arguments. You are thus not encouraged to call `sp_util` yourself.

The following comments are only provided, so that you can understand the purpose of various calls to `sp_util`.

- `sp_util` with no argument returns its version number.
- `sp_util('ismex')` true if `sp_util` is a `mex` file on your platform/path.
- `ind=sp_util('profile',k)` returns the profile of a sparse matrix (assumed to be symmetric). This is useful to have an idea of the memory required to store a Cholesky factor of this matrix.
- `ks=sp_util('sp2sky',sparse(k))` returns the structure array used by the `ofact` object.
- `ks = sp_util('sky_dec',ks)` computes the LDL' factor of a `ofact` object and replaces the object data by the factor. The `sky_inv` command is used for forward/backward substitution (take a look at the `@ofact\mldivide.m` function). `sky_mul` provides matrix multiplication for unfactored `ofact` matrices.
- `k = sp_util('nas2sp',K,RowStart,InColumn,opt)` is used by `nasread` for fast transformation between NASTRAN binary format and MATLAB sparse matrix storage.
- `k = sp_util('spind',k,ind)` renumbering and/or block extraction of a matrix. The input and output arguments `k` MUST be the same. This is not typically acceptable behavior for MATLAB functions but the speed-up compared with `k=k(ind,ind)` can be significant.
- `k = sp_util('xkx',x,k)` coordinate change for `x` a 3 by 3 matrix and DOFs of `k` stacked by groups of 3 for which the coordinate change must be applied.

- `ener = sp_util('ener',ki,ke,length(Up.DOF),mind,T)` is used by `upcom` to compute energy distributions in a list of elements. Note that this function does not handle numerical round-off problems in the same way as previous calls.
- `k = sp_util('mind',ki,ke,N,mind)` returns the square sparse matrix `k` associated to the vector of full matrix indices `ki` (column-wise position from `1` to `N^2`) and associated values `ke`. This is used for finite element model assembly by `fe_mk` and `upcom`. In the later case, the optional argument `mind` is used to multiply the blocks of `ke` by appropriate coefficients. `mindsym` has the same objective but assumes that `ki,ke` only store the upper half of a symmetric matrix.
- `sparse = sp_util('sp2st',k)` returns a structure array with fields corresponding to the MATLAB sparse matrix object. This is a debugging tool.
- `sp_util('setinput',mat,vect,start)` places vector `vect` in matrix `mat` starting at C position `start`. Be careful to note that `start` is modified to contain the end position.

stack_get,stack_set,stack_rm

Purpose Stack handling functions.

Syntax

```
[StackRows,index]=stack_get(model,typ);  
[StackRows,index]=stack_get(model,typ,name);  
Up=stack_set(model,typ,name,val)  
Up=stack_rm(model,typ,name);  
Up=stack_rm(model,typ);  
Up=stack_rm(model,'',name);
```

Description The `.Stack` field is used to store a variety of information, in a N by 3 cell array with each row of the form `{'type','name',val}` (see section 7.6 or section 7.7 for example). The purpose of this cell array is to deal with an unordered set of data entries which can be classified by type and name.

Since sorting can be done by name only, names should all be distinct. If the types are different, this is not an obligation, just good practice. In get and remove calls, `typ` and `name` can start by `#` to use a regular expression based on matching (use `doc regexp` to access detailed documentation on regular expressions).

Syntax

```
Case.Stack={'DofSet','Point accel',[4.03;55.03];  
           'DofLoad','Force',[2.03];  
           'SensDof','Sensors',[4 55 30]'+.03};  
% Replace first entry  
Case=stack_set(Case,'DofSet','Point accel',[4.03;55.03;2.03]);  
Case.Stack  
% Add new entry  
Case=stack_set(Case,'DofSet','P2',[4.03]);  
Case.Stack  
% Remove entry  
Case=stack_rm(Case,'','Sensors');Case.Stack  
% Get DofSet entries and access  
[Val,ind]=stack_get(Case,'DofSet')  
Case.Stack{ind(1),3} % same as Val{1,3}  
% Regular expression match of entries starting with a P  
stack_get(Case,'','P*')
```

SDT provides simplified access to stacks in `feplot` (see section 4.1.2) and `iipplot`

stack_get,stack_set,stack_rm

figures (see section 2.1.2). `cf.Stack{'Name'}` can be used for direct access to the stack, and `cf.CStack{'Name'}` for access to FEM model case stacks.

ufread

Purpose Read from Universal Files.

Syntax

```
ufread
ufread('FileNameOrList')
UFS = ufrac('FileName')
UFS = ufrac('FileList*.uff')
```

Description The Universal File Format is a set of ASCII file formats widely used to exchange analysis and test data. As detailed below `ufread` supports test related UFF (15 grid point, `UFF55` analysis data at node, `UFF58` response data at DOF) and with the FEMLink extension FEM related datasets.

`ufread` with no arguments opens a GUI to let you select a file and displays the result using `feplot` and/or `iiplot`. `ufread('FileName')` opens an `feplot` or `iiplot` figure with the contents. `UFS=ufread('FileName')` returns either a FEM model (if only model information is given) or a curve stack `UFS` pointing to the universal files present in `FileName` grouped by blocks of files read as a single dataset in the *SDT* (all FRFs of a given test, all trace lines of a given structure, etc.). You can specify a file list using the `*` character in the file name.

You get a summary of the file contents by displaying `UFS`

```
>> UFS
```

```
UFS = UFF curve stack for file 'example.uff'
```

```
{1} [.Node (local) 107x7, .Elt (local) 7x156] : model
 2 [.w (UFF) 512x1, .xf (UFF) 512x3] : response data
 3 [.po (local) 11x2, .res (local) 11x318] : shape data
```

which indicates the content of each dataset in the stack, the current data set between braces { }, the type and size of the main data fields. For response data (UFF type 58), the data is only imported when you refer to it (`UFS(i)` call) but it is imported every time you do so unless you force loading into memory using `UFS(i)=UFS(i)`.

The `UFS` object gives you direct access to the data in each field. In the example above, you can display the modeshapes using

```
cf = feplot;
```

```
cf.model = UFS(1);
cf.def   = UFS(3);
```

When loading response data, you may want to transfer all options from the universal file to an `iiplot` stack entry using calls of the form `ci.Stack{'curve', 'Test'}=UFS(3)`. If you need to extract partial sets of DOF, consider `fe_def SubDof`.

15 Grid point

Grid points stored in a node matrix (see node page 227) in a `UFS(i).Node` field.

The format is a (4I10,1P3E13.5) record for each node with fields

```
[NodeID PID DID GID x y z]
```

where `NodeID` are node numbers (positive integers with no constraint on order or continuity), `PID` and `DID` are coordinate system numbers for position and displacement respectively (this option is not currently used), `GID` is a node group number (zero or any positive integer), and `x y z` are the coordinates.

55 Analysis data at node

UFF55 *Analysis data at nodes* are characterized by poles `.po` and residues `.res` (corresponding to DOFs `.dof`) and correspond to shape at DOF datasets (see more info under the `xfopt` help).

The information below gives a short description of the universal file format. You are encouraged to look at comments in the `ufread` and `ufwrite` source codes if you want more details.

<code>Header1</code>	(80A1). The UFF header lines are stored in the <code>.header</code> field
<code>Header2</code>	(80A1)
<code>Header3</code>	(80A1) DD- <i>MMM</i> - <i>YY</i> and HH:MM:SS with format (9A1,1X,8A1)
<code>Header4</code>	(80A1)
<code>Header5</code>	(80A1)
<code>Fun</code>	(6I10) This is stored in the <code>.fun</code> field
<code>SpeInt</code>	(8I10) <code>NumberOfIntegers</code> on this line (3-N are type specific), <code>NumberOfReals</code> on the next line, <code>SpeInt</code> type specific integers (see table below for details)
<code>SpeRea</code>	Type specific real parameters
<code>NodeID</code>	(I10) Node number
<code>Data</code>	(6E13.5) Data At This Node : NDV Real Or Complex Values (real imaginary for data 1, ...) Records 9 And 10 Are Repeated For Each Node.

Type specific values depend on the **Signification** value and are stored in the **.r55** field.

0 Unknown	[1 1 ID Number] [0.0]
1 Static	[1 1 LoadCase] [0.0]
2 Normal model	[2 4 LoadCase ModeNumber] [FreqHz ModalMass DampRatioViscous DampRatioHysteretic]
3 Complex eigenvalue	[2 6 LoadCase ModeNumber] [ReLambda ImLambda ReModalA ImModalA ReModalB ImModalB]
4 Transient	[2 1 LoadCase TimeStep] [TimeSeconds]
5 Frequency response	[2 1 LoadCase FreqStepNumber] [FrequencyHz]
6 Buckling	[1 1 LoadCase] [Eigenvalue]

58 Function at nodal DOF

UFF58 *Functions at nodal DOF* (see **Response data**) are characterized by frequencies **w**, a data set **xf**, as well as other options. The information below gives a short description of the universal file format. You are encouraged to look at comments in the **ufread** and **ufwrite** source codes if you want more details. Functions at nodal DOFs are grouped by type and stored in response data sets of **UFS**.

Header1	(80A1) Function description
Header2	(80A1) Run Identification
Header3	(80A1) Time stamp DD-MMM-YY and HH:MM:SS with format (9A1,1X,8A1)
Header4	(80A1) Load Case Name
Header5	(80A1)

DOFID This is stored in `.dof` field (which also has a file number as address in column 3). Values are

- 2(I5,I10) : **FunType** (list with `xfopt('_funtype')`), stored in `.fun(1)`), **FunID** (ID in `.dof(:,5)`), **VerID** version or sequence number, **LoadCase** (0 single point)
- (1X,10A1,I10,I4) : **ResponseGroup** (**NONE** if unused, ID in `.dof(:,4)`), **ResponseNodeID**, **ResponseDofID** (1:6 correspond to *SDT* DOFs `.01` to `.06`, `-1:-6` to *SDT* DOFs `.07` to `.12`). DOF coding stored in `.dof(:,1)`).
- (1X,10A1,I10,I4) : **ReferenceGroup** (**NONE** if unused, ID in `.dof(:,4)`), **ReferenceNodeID**, **ReferenceDofID**. These are only relevant if **LoadCase** is zero. DOF coding stored in `.dof(:,2)`).

DataForm (3I10,3E13.5)

DFormat (2 : real, single precision, 4 : real, double precision, 5 : complex, single precision, 6 : complex, double precision), **NumberOfDataPoints**, **XSpacing** (0 - uneven, 1 - even (no abscissa values stored)), **XMinimum** (0.0 if uneven), **XStep** (0.0 if spacing uneven), **ZAxisValue** (0.0 if unused)

XDataForm (I10,3I5,2(1X,20A1)) **DataType** (list with `xfopt('_datatype')`), **lue** length unit exponents, **fue** force, **tue** temperature, **AxisLabel**, **AxisUnits**

Note : exponents are used to define dimensions. Thus Energy (Force * Length) has `[fue lue tue]=[1 1 0]`. This information is generally redundant with **DataType**.

YNDataForm Ordinate (or ordinate numerator) Data Form (same as **XDataForm**)

YDDataForm Ordinate Denominator Data Characteristics

ZDataForm Z-axis Data Characteristics

DataValue a series of **x** value (if uneven x spacing, always with format E13.5), real part, imaginary part (if exists) with precision (E13.5 or E20.12) depending on **DFormat**.

82, Trace Line

UFF82 *Trace Line matrix* **LDraw** where each non-empty row corresponds to a line to be traced. All trace lines, are stored as element groups of **UFS(1).Elt**.

LDraw can be used to create animated deformation plots using **feplot**.

Opt (3I10) **LineNumber**, **NumberOfNodes**, **Color**
Label (80A1) Identification for the line
Header3 (8I10) node numbers with 0 for discontinuities

(,1:2) [*NumberOfNodes GroupID*]
 (,3:82) [*LineName*] (which should correspond to the group name)
 (,83:end) [*NodeNumbers*] (*NumberOfNodes* of them, with zeros to break the line)

151, Header

Header stored as a string matrix **header** (with 7 rows).

780, 2412, Elements

These universal file formats are supported by the SDT FEMLink extension.

SDT UNV element (UNV Id)
beam1 rod (11), linear beam (21)
tria3 thin shell lin triangle (91), plane stress lin tri (41), plan strain lin tri (51), flat plate lin triangle (74)
tria6 thin shell para tri (92), plane stress para tri (42), plane strain para tri (51), flat plate para tri (62), membrane para tri (72)
quad4 thin shell lin quad (94), plane stress lin quad (44), plane strain lin quad (54), flat plate lin quad (64), membrane lin quad (71)
quadb thin shell para quad (95), plane stress para quad (54), plane strain para quad(55), flat plate para quad (65), membrane para quad(75)
tetra4 solid lin tetra (111)
tetra10 solid para tetra (118)
penta6 solid lin wedge (112)
penta15 solid para wedge (113)
hexa8 solid lin brick (115)
hexa20 solid para brick (116)
rigid rigid element (122)
bar1 node-node trans spring (136), node-node rot spring (137)
mass2 lumped mass (161)

773, 1710 Material Database

These universal file formats are supported by the SDT FEMLink extension.

All materials properties are read, but obviously only those currently supported by the SDT are translated to the corresponding row format (see [m_elastic](#) and section 7.4).

772, 788, 789, 2437, Element Properties

These universal file formats are supported by the SDT FEMLink extension.

All element (physical) properties are read, but obviously only those currently supported by the SDT are translated to the corresponding row format (see [p_beam](#), [p_shell](#), section 7.3).

2414, Analysis data

These universal file formats are supported by the SDT FEMLink extension.

Note that the list of FEMLink supported dataset is likely to change between manual editions. Please get in touch with SDTools if a dataset you want to read is not supported.

See also [nasread](#), [ufwrite](#), [xfopt](#)

ufwrite

Purpose Write to a Universal File.

Syntax `ufwrite(FileName,UFS,i)`
`ufwrite(FileName,model)`

Description You can export to UFF using the `feplot` and `iipplot` export menus. `ufwrite(FileName,UFS,i)` appends the dataset `i` from a curve stack `UFS` to the file `FileName`. For details on curve stacks see section 2.1.2. `ufwrite(FileName,model)` can be used to export FEM models.

For datasets representing

- models, `ufwrite` writes a UFF of type 15 for the nodes and a trace line (UFF 82) for test wire frames (all `EGID` negative) or without FEMLink. With FEMLink, nodes are written in UFF 2411 format and elements in UFF 2412.
- response data, `ufwrite` writes a *response at DOF* (UFF 58) for each column of the response set.
- shape data, `ufwrite` writes a *data at nodal DOF* (UFF 55) for each row in the shape data set.

Starting from scratch, you define an curve stack `DB=xfopt('empty')`. You can then copy data sets from the stack `XF` (previously initialized by `iipplot` or `xfopt`) using `DB(i)=XF(j)`. You can also build a new data set by giving its fields (see `xfopt` for the fields for the three supported dataset types). The following would be a typical example

```
UF=xfopt('empty')
UF(1)={'node',FEnode,'elt',FEelt};
UF(2)={'w',IIw,'xf',IIxf};
UF(3)={'po',IIres,'res',IIres,'dof',XFdof};
```

Once the curve stack built, `ufwrite('NewFile',UF,1:3)` will write the three datasets.

With `iipplot`, you can use the stack to change properties as needed then write selected datasets to a file. For example,

```
tname=nas2up('tempname .uf');
ci=iicom('CurveLoad','gartid');
```

```
ci.Stack{'Test'}.x='frequency'; % modify properties, see xfopt('_datatype')
ci.Stack{'Test'}.yn='accele';
iicom('sub'); % reinitialize plot to check
ufwrite(tname,ci,'Test');
% write a model
ci.Stack{'SE','model'}=demosdt('demo gartte');
ufwrite(tname,ci,'model');
% write a time trace
C1=fe_curve('TestRicker .6 2',linspace(0,1.2,120));
XF=iicom(ci,'curvxf');XF('Ricker')={'w',C1.X,'xf',C1.Y}
%ci.Stack{'curve','Ricker'}=struct('w',C1.X,'xf',C1.Y); % XXXeb with ci
ci.Stack{'Ricker'}.x='time'; % see xfopt('_datatype')
ci.Stack{'Ricker'}.yn='Acceleration'; % see xfopt('_datatype')
ci.Stack{'Ricker'}.fun=[1]; % see xfopt('_funtype')
ufwrite(tname,ci,'Ricker'); % XXXeb plante
UFS=ufread(tname); % reread the UFF to check result
```

Note that you can edit these properties graphically in the [iiplot properties](#) ... figure.

See also [ufread](#), [iiplot](#), [nasread](#)

upcom

Purpose User interface function for parameterized superelements.

Description The `upcom` interface supports type 3 superelements which handle parameterization by storing element matrix dictionaries and thus allowing reassembly of mass and stiffness matrices computed as weighted sums of element matrices (6.107).

By default, `upcom` uses a special purpose superelement stored in the **global variable Up**. You can however use more than one type 3 superelement by providing the appropriate variables as input/output arguments. `upcom('info')` applies to `Up` whereas `upcom(model,'info')` applies to `model`.

The `par` commands are used to dynamically relate the element matrix weights to physical parameters thus allowing fairly complex parametric studies on families of models. The main objective for `upcom` is to enable finite element model updating, but it can also be used for optimization and all problems using with families of models or hysteretic damping modeling as illustrated in section 5.3.2.

The following paragraphs detail calling formats for commands supported by `upcom` and are followed by an explanation of the signification of the fields of `Up` (see the `commode` help for hints on how to build commands and understand the variants discussed in this help).

More details on how these commands are typically sequenced are given in the *Tutorial* section 6.4 and section 6.5.

The implementation of the `upcom` interface has undergone major revisions for *SDT 5.0* so that it is not fully backward compatible. In particular the handling of parameters and the `assemble` calls have changed.

Commands

`Clear`, `Load File`, `Save File`

`upcom('clear')` clears the global variable `Up` and the local and base variables `Up` if they exist. If these local variables are not cleared then the global variable `Up` is reset to that value.

`upcom('load File')` loads the superelement fields from `File.mat` and creates the file if it does not currently exist. `upcom('save File')` makes sure that the current values of the various fields are saved in `File.mat`. Certain commands automatically save the superelement but efficiency mandates not to do it all the time. The working

directory field `Up.wd` lets you work in a directory that differs from the directory where the file is actually located.

`Assemble [,m,k] [,coef cur] [,delta i] [,NoT] [,Point]`

`[m,k] = upcom('assemble')` returns the mass and stiffness parameters associated with the parameters by the last `parcoef` command.

`Assemble Coef cur` uses the parameter values `cur` for the assembly. `Assemble CoefNone` does not use any parameter definitions (all the element matrices are used with a unit weighting coefficient). `AssembleMind` uses columns 5 and 6 of `Up.mind` for element matrix coefficients.

`Assemble Delta i` assembles the derivative of matrices with respect to parameter `i`. To assemble a derivative with non zero components on more than one parameter, use `[dm,dk]=upcom('assemble delta',dirp)` where `dirp` (with N_{par} rows) characterizes the amplitude of the derivative on each parameter for the current change. `dirp` can for example be used to describe simultaneous changes in mass and stiffness parameters.

`k=upcom('assemble k coef 2 3')` only assembles the stiffness with parameter coefficients set to 2 and 3. Similarly, `dm=upcom('assemble m delta 2')` will assemble the mass derivative with respect to parameter 2.

The `NoT` option can be used to prevent the default projection of the matrices on the master DOFs defined by the current case.

The `Point` option can be used return the `v_handle` object pointing to the non assembled matrix. This matrix can then be used in `feutilb('tkt')` and `feutilb('a*b')` out of core operations.

`ComputeMode [,full,reduced] [, eig_opt]`

`[mode,freq] = upcom('ComputeMode')` assembles the model mass and stiffness based on current model parameters (see the `parcoef` command) and computes modes. The optional `full` or `reduced` can be used to change the current default (see the `opt` command). The optional `eig_opt` can be used to call `fe_eig` with options other than the current defaults (see the `opt` command).

```
upcom('load GartUp');
def = upcom('computemode full 105 10 1e3');
```

For reduced model computations, the outputs are `[moder,freq,modefull]`.

ComputeModal [,full,reduced]

Given a parameterized model, the command `ComputeModal` computes the frequency response associated to all the inputs and outputs of the model, taken into account the damping ratio. `ComputeModal` computes the normal modes and static corrections for inputs of the full or reduced order models based on the full or reduced model. `nor2xf` is then called to build the responses (for sensor load definitions within the model, see `nor2xf`).

```
upcom('load GartUp');
Up=fe_case(Up,'SensDof','sensors',[3.03;54.03],'DofLoad','input',3.03);
upcom('compute modal full acc iiplot "updated" -po -reset');
```

You may want to compute the direct frequency response associated the inputs on all the DOFs structure. It does not compute modes and is thus faster than `ComputeModal` for a full order model and a few frequency points. The high level call uses the `fe_simul` function

```
cf=fecom('load',which('GartUp.mat'));
cf.mdl=fe_case(cf.mdl,'DofLoad','input',3.03);
cf.mdl=stack_set(cf.mdl,'info','Freq',linspace(0,15,50));
cf.def=fe_simul('DFRF',cf.mdl);fecom('ch22');
```

Ener [m, k]

`ener = upcom('ener k',def)` computes the strain energy in each element for the deformations `def`. `ener` is a data structure with fields `.IndInElt` specifying the element associated with each energy row described in the `.data` field. You can display the kinetic energy in an arbitrary element selection of a structure, using a call of the form

```
cf.sel={'group6','colordata elt',upcom('ener m','group6',mode)};
```

Fix

`upcom('fix0')` eliminates DOFs with no stiffness contribution. `upcom('fix',adof)` only retains DOFs selected by `adof`.

This command is rather inefficient and you should eliminate DOFs with `FixDOF` case entries (see `fe_case`) or assemble directly with the desired DOFs (specify `adof` in the `SetNominal` command).

Get

Information about the superelement is stored in fields of the global variable `Up`. The easiest way to access those fields is to make the variable local to your workspace (use `global Up`) and to access the fields directly. The superelement also has pseudo-fields `mi`, `me`, `ki`, `ke` which are always stored in `Up.file`. Commands of the form `load(Up.file,'ke')` are used to get them.

femesh

`upcom femesh` copies `Up.Elt` to `FEelt` and `Up.Node` to `FEnode` so that `femesh` commands can be applied to the model.

IndInElt

`upcom('IndInElt')` returns a vector giving the row position in `Up.Elt` of each row in `Up.mind`. This is in particular used for color coded energy plots which should now take the form

```
feplot('ColorDataElt',upcom('eners',res),upcom('indinelt'));
```

Although it is typically easier to use high level calls of the form

```
upcom('load GartUp');upcom('plotelt');  
cf=feplot;cf.def=fe_eig(Up,[5 10 1e3]);fecom('ch7');  
cf.sel={'groupall','colordata enerk'};
```

Info [,par,elt]

`upcom('info')` prints information about the current content of `Up`: size of full and reduced model, values of parameters currently declared, types, etc.

`InfoPar` details currently defined parameters. `InfoElt` details the model.

Opt

`upcom('opt Name ' '')` sets the option *Name* to a given *Value*. Thus `upcom('opt gPrint 11')` sets the general printout level to 11 (maximum). Accepted names and values are detailed in the `Up.copt` field description below.

Par [add type values,reset]

These commands allow the creation of a parameter definition stack. Each parameter is given a type (**k** for stiffness, **m** for mass, **t** for thickness) optional current, min and max values, a name, and an element selection command.

```
Up=upcom('load GartUp'); % Load sample model
Up=fe_case(Up,'ParReset') % Reset parameters
Up=fe_case(Up,'ParAdd k 1.0 0.5 2.0','Tail','group3');
Up=fe_case(Up,'ParAdd t 1.0 0.9 1.1','Constrained Layer','group6');
Up=fe_case(Up,'parcoef',[1.2 1.3]);
upcom(Up,'info par');
```

Parameters are stored in the case stack and can be selected with

```
des=fe_case(Up,'stack_get','par')
```

des is a cell array where each row has the form {'par','name',data} with **data** containing fields

.sel	string or cell array allowing selection of elements affected by the parameter
.coef	vector of parameter coefficients (see format description under upcom ParCoef).
.pdir	Boolean vector giving the positions of affected elements in Up.mind (for upcom models)
.name	Parameter name

ParCoef

The value of each physical parameter declared using [upcom Par](#) or [fe_case par](#) commands is described by a row of coefficients following the format

```
[type cur min max vtype]
```

with

- **type 1** stiffness proportional to parameter value. This is the case for a variable Young's modulus. **2** mass proportional to parameter. This is the case for a variable mass density.
3 variable thickness ([upcom](#) only). Currently only valid for [quad4](#) and [quadb](#) elements. [tria3](#) elements can be handled with degenerate [quad4](#). Element groups with variable thickness must be declared at assembly during [upcom\('SetNom](#)

- `cur` for current value
- `min` for minimum value
- `max` for maximum value
- `vtype` deals with the type of variation 1 linear, 2 log (not fully implemented)

`upcom(Up, 'parcoef', cur)` is used to set current values (`cur` must be a vector of length the number of declared parameters), while `upcom(Up, 'parcoef', par)` also sets min, max and vtype values. You can also use `[cur, par]=upcom(Up, 'parcoef')` or `par=upcom(Up, 'parcoefpar')` to obtain current values or the parameter value matrix.

An example of parameter setting is thus

```
Up=demosdt('gartup'); % see sdtweb demosdt('gartup')
%      MatType cur min max vtype
par = [ 1      1.0 0.1 3.0  1 ; ... % Linear
        3      0.0 -1 2.0  2 ];   % Log variation
Up=upcom(Up, 'parcoef', par);
upcom(Up, 'info par');
[cur, par]=upcom(Up, 'parcoef')
```

Note that to prevent user errors, `upcom` does not allow parameter overlap for the same type of matrix (modification of the modulus and/or the thickness of the same element by two distinct parameters).

ParRed

`upcom('par red', T)` projects the current full order model with the currently declared parameters on the basis `T`. Typical reduction bases are discussed in section 6.2.7 and an example is shown in the `gartup` demo. Matrices to be projected are selected based on the currently declared variable parameters in such a way that projected reduced model is able to make predictions for new values of the parameters.

ParTable

`tt=upcom('partable')` returns a cell array of string describing the parameters currently declared. This cell array is useful to generate formatted outputs for inclusion in various reports using `comstr(tt, -17, 'excel')` for example.

PlotElt

`upcom plotelt` initializes a `feplot` figure displaying the model in `upcom`. If `Up` has deformations defined in a `.def` field, these are shown using `cf=feplot;cf.def=Up`.

Profile [,fix]

Renumbers DOFs and pseudo-fields `mi,me,ki,ke` using `symrcm` to minimize matrix bandwidth. `ProfileFix` eliminates DOFs with no stiffness on the diagonal at the same time. `upcom('ProfileFix',fdof)` profiles and eliminates DOFs in `fdof` and DOFs with no stiffness on the diagonal.

Support for case entries (see `fe_case`) makes this command obsolete.

SensMode [,reduced]

`[fsen,mdsen,mode,freq] = upcom('SensMode',dirp,indm,T)` returns frequency and modeshape sensitivities of modes with indices given in `indm` for modifications described by `dirp`.

For a model with NP parameters (declared with the `Par` commands), `dirp` is a matrix with $Npar$ rows where each column describe a case of parameter changes of the form `par = dirp(:,j)`. The default for `dirp` the identity matrix (unit change in the direction of each parameter).

The optional argument `T` can be used to give an estimate of modeshapes at the current design point. If `T` is given the modes are not computed which saves time but decreases accuracy if the modes are not exact.

`fsen` gives, for modes `indm`, the sensitivities of modal frequencies squared to all parameters (one column of `fsen` per parameter). `mdsen` stores the modeshape sensitivities sequentially (sensitivities of modes in `indm` to parameter 1, parameter 2, ...).

When modeshape sensitivities are not desired (output is `[fsen]` or `[fsen, mode, freq]`), they are not computed which takes much less computational time.

By default `SensMode` uses the full order model. The first order correction to the modal method discussed in Ref. [41] is used. You can access the reduced order model sensitivities using `SensModeReduced` but should be aware that accuracy will then strongly depend on the basis you used for model reduction (`ParRed` command).

SetNominal [, t groups]

To generate a new model, you should first clear any `Up` variable in the workspace, specify the file that where you will want the element matrices to be saved, then perform the assembly. For example

```
clear global Up
upcom(horzcat('load',nas2up('tempname.mat')));
model=demosdt('demogartfe')
upcom('setnominal',model)
% delete(fullfile(Up.wd,[Up.file,'.mat'])) % to remove the result
```

Case information (boundary conditons, ... see `fe_case`) in `model` is saved in `Up.Stack` and will be used in assembly unless the `NoT` option is included in the `Assemble` command.

If the parameter that will be declared using the `Par` commands include thickness variations of some plate/shell elements, the model will use element sub-matrices. You thus need to declare which element groups need to have a separation in element submatrices (doing this separation takes time and requires more final storage memory so that it is not performed automatically). This declaration is done with a command of the form `SetNominal T groups` which gives a list of the groups that need separation.

Obsolete calling formats `upcom('setnominal',FNode,FEelt,pl,il)` and `upcom('setnominal',FNode,FEelt,pl,il,[],adof)` (where the empty argument `[]` is used for coherence with calls to `fe_mk`) are still supported but you should switch to using FEM model structures.

Fields of Up

`Up` is a generic superelement (see description under `fe_super`) with additional fields described below. The `Up.Opt(1,4)` value specifies whether the element matrices are symmetric or not.

Up.copt

The *computational options* field contains the following information

```
(1,1:7) = [oMethod gPrint Units Wmin Wmax Model Step]
```

oMethod optimization algorithm used for FE updates
 1: `fmins` of MATLAB (default)
 2: `fminu` of the *Optimization Toolbox*
 3: `up_min`
gPrint printout level (0 none to 11 maximum)
Units for the frequency/time data vector `w` and the poles
 01: `w` in Hertz 02: `w` in rad/s 03: `w` time seconds
 10: `po` in Hertz 20: `po` in rad/s
 example: `Up.copt(1,3) = 12` gives `w` in rad/sec and `po` in Hz
Wmin index of the first frequency to be used for update
Wmax index of the last frequency to be used for update
Model flag for model selection (0 full `Up`, 1 reduced `UpR`)
Step step size for optimization algorithms (`foptions(18)`)
 (2,1:5) = [`eMethod nm Shift ePrint Thres MaxIte`]

are options used for full order eigenvalue computations (see `fe.eig` for details).

(3,1) = [`exMethod`]

exMethod expansion method (0: static, 1: dynamic, 2: reduced basis dynamic, 3: modal, 4: reduced basis minimum residual)

`Up.mind`, `Up.file`, `Up.wd`, `mi`, `me`, `ki`, `ke`

`Up` stores element submatrices in pseudo-fields `mi`, `me`, `ki`, `ke` which are loaded from `Up.file` when needed and cleared immediately afterwards to optimize memory usage. The working directory `Up.wd` field is used to keep track of the file location even if the user changes the current directory. The `upcom save` command saves all `Up` fields and pseudo-fields in the file which allows restarts using `upcom load`.

`Up.mind` is a $NElt \times 6$ matrix. The first two columns give element (sub-)matrix start and end indices for the mass matrix (positions in `mi` and `me`). Columns 3:4 give element (sub-)matrix start and end indices for the stiffness matrix (positions in `ki` and `ke`). Column 5 (6) give the coefficient associated to each element mass (stiffness) matrix. If columns 5:6 do not exist the coefficients are assumed equal to 1. The objective of these vectors is to optimize model reassembly with scalar weights on element matrices.

`Up.Node`, `Up.Elt`, `Up.pl`, `Up.il`, `Up.DOF`, `Up.Stack`

Model nodes (see section 7.1), elements (see section 7.2), material (see section 7.3) and element (see section 7.4) property matrices, full order model DOFs. These values are set during the assembly with the `setnominal` command.

upcom

`Up.Stack` contains additional information. In particular parameter information (see `upcom par` commands) are stored in a case (see section 7.7) saved in this field.

`Up.sens`

Sensor configuration array built using `fe_sens`. This is used for automatic test / analysis correlation during finite element update phases.

See also `fesuper`, `up_freq`, `up_ixf`

up_freq, up_ifreq

- Purpose** Sensitivity and iterative updates based on a comparison of modal frequencies.
- Syntax**
- ```
[coef,mode,freq]=up_freq('Method',fID,modeID,sens);
[coef,mode,freq]=up_ifreq('Method',fID,modeID,sens);
```
- Description** `up_freq` and `up_ifreq` seek the values `coef` of the currently declared `Up` parameters (see the `upcom Par` command) such that the difference between the measured `fID` and model normal mode frequencies are minimized.
- Currently `'basic'` is the only *Method* implemented. It uses the maximum MAC (see `ii_mac`) to match test and analysis modes. To allow the MAC comparison modeshapes. You are expected to provide test modeshapes `modeID` and a sensor configuration matrix (initialized with `fe_sens`).
- The cost used in both functions is given by
- $$\text{norm}(\text{new\_freq}(\text{fDes}(:,1))-\text{fDes}(:,2)) / \text{norm}(\text{fDes}(:,2))$$
- `up_freq` uses frequency sensitivities to determine large steps. As many iterations as alternate matrices are performed. This acknowledges that the problem is really non-linear and also allows a treatment of cases with active constraints on the coefficients (minimum and maximum values for the coefficients are given in the `upcom Par` command).
- `up_ifreq` uses any available optimization algorithm (see `upcom opt`) to minimize the cost. The approach is much slower (in particular it should always be used with a reduced model). Depending on the algorithm, the optimum found may or may not be within the constraints set in the range given in the `upcom Par` command.
- These algorithms are very simple and should be taken as examples rather than truly working solutions. Better solutions are currently only provided through consulting services (ask for details at [info@sdtools.com](mailto:info@sdtools.com)).
- See also** `up_ixf`, `up_ifreq`, `fe_mk`, `upcom`

# up\_ixf

---

**Purpose** Iterative FE model update based on the comparison of measured and predicted FRFs.

**Syntax** `[jump]=up_ixf('basic',b,c,IIw,IIxf,indw)`

**Description** `up_ixf` seeks the values `coef` of the currently declared `Up` parameters (see the `upcom Par` command) such that the difference Log least-squares difference (3.4) between the desired and actual FRF is minimized. Input arguments are

`method` Currently `'basic'` is the only *Method* implemented.

`range` a matrix with three columns where each row gives the minimum, maximum and initial values associated the corresponding alternate matrix coefficient

`b,c` input and output shape matrices characterizing the FRF given using the full order model DOFs. See section 5.1.

`IIw` selected frequency points given using units characterized by `Up.copt(1,3)`

`IIxf` reference transfer function at frequency points `IIw`

`indw` indices of frequency points where the comparison is made. If empty all points are retained.

Currently `'basic'` is the only *Method* implemented. It uses the maximum MAC (see `ii_mac`) to match test and analysis modes. To allow the MAC comparison modeshapes. You are expected to provide test modeshapes `modeID` and a sensor configuration matrix (initialized with `fe_sens`).

`up_ixf` uses any available optimization algorithm (see `upcom opt`) to minimize the cost. Depending on the algorithm, the optimum found may or may not be within the constraints set in the range given in the `upcom Par` command.

This algorithm is very simple and should be taken as an example rather than an truly working solution. Better solutions are currently only provided through consulting services (ask for details at [info@sdttools.com](mailto:info@sdttools.com)).

**See also** `up_freq`, `upcom`, `fe_mk`

# xfopt

---

**Purpose** User interface for curve stack pointer objects. **Stack**, see section 2.1.2, are now preferred so this function is documented mostly for compatibility.

**Syntax**

```
xfopt command
XF(1).FieldName=FieldValue
XF(1).command='value'
XF.check
r1=XF(1).GetData
curve=XF(1).GetAsCurve
XF.save='FileName'
```

**Description** SDT considers data sets in curve, **Response data** or **Shapes at DOFs** formats. Handling of datasets is described in the `iiplot` tutorial which illustrates the use of curve stacks (previously called database wrappers).

`ufread` and `ufwrite` also use curve stacks which can be stored as variables. In this case, FEM models can also be stored in the stack.

The use of a stack pointer (obtained with `XF=iicom(ci,'curvexf');`) has side advantages that further checks on user input are performed.

`XF.check` verifies the consistency of information contained in all data sets and makes corrections when needed. This is used to fill in information that may have been left blank by the user.

`disp(XF)` gives general information about the datasets. `XF(i).info` gives detailed and formatted information about the dataset in `XF(i)`. `XF(i)` only returns the actual dataset contents.

Object saving is overloaded so that data is retrieved from a `iiplot` figure if appropriate before saving the data to a mat file.

Object field setting is also overloaded (consistency checks are performed before actually setting a field) This is illustrated by the following example

```
[ci,XF]=iiplot
XF(1)
XF(1).x='time'; XF(1).x
```

where `XF(1)` is a **Response data** set (with abscissa in field `.w`, responses in field `.xf`, ...).

`XF(1).x='time'` sets the `XF(1).x` field which contains a structure describing its type. Notice how you only needed to give the `'time'` argument to fill in all the information. The list of supported axis types is given using `xfopt('_datatype')`

`XF(1).w=[1:10]` sets the `XF(1).w` field.

## `_FunType`, `_DataType`, `_FieldType`

These commands are used internally by SDT. `xfopt _FunType` returns the current list of function types (given in the format specification for Universal File 58).

`label=xfopt('_FunType',type)` and `type=xfopt('_FunType','label')` are two other accepted calls.

`xfopt _DataType` returns the current list of data types (given in the format specification for Universal File 58). `xfopt('_DataType',type)` and `xfopt('_DataType','label')` are two other accepted calls.

For example `XF.x.label='Frequency'` or `XF.x=18`.

Data types are used to characterize axes (abscissa (**x**), ordinate numerator (**yn**), ordinate denominator (**yd**) and z-axis data (**z**)). They are characterized by the fields

|                     |                                                                                                                                                                 |
|---------------------|-----------------------------------------------------------------------------------------------------------------------------------------------------------------|
| <code>.type</code>  | four integers describing the axis function type <code>fun</code> (see list with <code>xfopt('_datatype')</code> ), length, force and temperature unit exponents |
| <code>.label</code> | a string label for the axis                                                                                                                                     |
| <code>.unit</code>  | a string for the unit of the axis                                                                                                                               |

`xfopt _FieldType` returns the current list of field types.

See also

`idopt`, `id_rm`, `iipplot`, `ufread`



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